

Application for resource consent or fast-track resource consent

(Or Associated Consent Pursuant to the Resource Management Act 1991 (RMA)) (If applying for a Resource Consent pursuant to Section 87AAC or 88 of the RMA, this form can be used to satisfy the requirements of Schedule 4). Prior to, and during, completion of this application form, please refer to Resource Consent Guidance Notes and Schedule of Fees and Charges — <u>both available on the Council's web page</u>.

| 1. Pre-Lodgement Meeting | | | |
|---|--|--|--|
| Have you met with a council Resource Consent representative to discuss this application prior to lodgement? Yes No | | | |
| 2. Type of Consent being applied for | | | |
| (more than one circle can be ticked): | (more than one circle can be ticked): | | |
| C Land Use | Discharge | | |
| Fast Track Land Use* | Change of Consent Notice (s.221(3)) | | |
| Subdivision | Extension of time (s.125) | | |
| Consent under National Environmental Standard (e.g. Assessing and Managing Contaminants in Soil) | | | |
| Other (please specify) | | | |
| * The fast track is for simple land use consents and is re | estricted to consents with a controlled activity status. | | |

3. Would you like to opt out of the Fast Track Process?

Yes No

4. Consultation

| Have you consulted with lwi/Hapū? 🔵 Yes 🔵 No | | |
|--|--|--|
| If yes, which groups have you consulted with? | | |
| Who else have you consulted with? | | |

For any questions or information regarding iwi/hapū consultation, please contact Te Hono at Far North District Council <u>tehonosupport@fndc.govt.nz</u>

5. Applicant Details

| Name/s: | Oruaiti School Board of Trustees | |
|---|----------------------------------|----------|
| Email: | | |
| Phone number: | Work | Home |
| Postal address: (or alternative method of service under section 352 of the act) | | Postcode |

6. Address for Correspondence

Name and address for service and correspondence (if using an Agent write their details here)

| Name/s: | Melissa McGrath | |
|---|-----------------|--|
| Email: | | |
| Phone number: | | |
| Postal address: (or alternative method of service under section 352 of the act) | | |

* All correspondence will be sent by email in the first instance. Please advise us if you would prefer an alternative means of communication.

7. Details of Property Owner/s and Occupier/s

Name and Address of the Owner/Occupiers of the land to which this application relates (where there are multiple owners or occupiers please list on a separate sheet if required)

| Name/s: | Ministry of Education | | |
|-------------------------------|---------------------------------|------|--|
| Property Address/ ocation: | 2554 State Highway 10, Mangōnui | | |
| | Postcode | 0494 | |

8. Application Site Details

Location and/or property street address of the proposed activity:

| Name/s: Site Address/ Location: | |
|---------------------------------------|-------------|
| | Postcode |
| Legal Description: | Val Number: |
| Certificate of title: | |

Please remember to attach a copy of your Certificate of Title to the application, along with relevant consent notices and/or easements and encumbrances (search copy must be less than 6 months old)

Site visit requirements:

Is there a locked gate or security system restricting access by Council staff? **Yes No**

Is there a dog on the property? Yes No

Please provide details of any other entry restrictions that Council staff should be aware of, e.g. health and safety, caretaker's details. This is important to avoid a wasted trip and having to rearrange a second visit.

9. Description of the Proposal:

Please enter a brief description of the proposal here. Please refer to Chapter 4 of the District Plan, and Guidance Notes, for further details of information requirements.

If this is an application for a Change or Cancellation of Consent Notice conditions (s.221(3)), please quote relevant existing Resource Consents and Consent Notice identifiers and provide details of the change(s), with reasons for requesting them.

10. Would you like to request Public Notification?

Yes No

11. Other Consent required/being applied for under different legislation

(more than one circle can be ticked):

- Building Consent Enter BC ref # here (if known)
- Regional Council Consent (ref # if known) Ref # here (if known)

National Environmental Standard consent Consent here (if known)

Other (please specify) Specify 'other' here

12. National Environmental Standard for Assessing and Managing Contaminants in Soil to Protect Human Health:

The site and proposal may be subject to the above NES. In order to determine whether regard needs to be had to the NES please answer the following:

Is the piece of land currently being used or has it historically ever been used for an activity or industry on the Hazardous Industries and Activities List (HAIL) **Yes No Don't know**

Is the proposed activity an activity covered by the NES? Please tick if any of the following apply to your proposal, as the NESCS may apply as a result. **Yes No Don't know**

Subdividing land

- Changing the use of a piece of land
- Disturbing, removing or sampling soil
 Removing or replacing a fuel storage system

13. Assessment of Environmental Effects:

Every application for resource consent must be accompanied by an Assessment of Environmental Effects (AEE). This is a requirement of Schedule 4 of the Resource Management Act 1991 and an application can be rejected if an adequate AEE is not provided. The information in an AEE must be specified in sufficient detail to satisfy the purpose for which it is required. Your AEE may include additional information such as Written Approvals from adjoining property owners, or affected parties.

Your AEE is attached to this application **Yes**

13. Draft Conditions:

Do you wish to see the draft conditions prior to the release of the resource consent decision? () Yes () No

If yes, do you agree to extend the processing timeframe pursuant to Section 37 of the Resource Management Act by 5 working days? **Yes No**

14. Billing Details:

This identifies the person or entity that will be responsible for paying any invoices or receiving any refunds associated with processing this resource consent. Please also refer to Council's Fees and Charges Schedule.

| Name/s: (please write in full) | Diane | Bates, | Druaiti | School |
|---|-------|--------|---------|--------|
| Email: | | | | |
| Phone number: | | | | |
| Postal address: (or alternative method of service under section 352 of the act) | | | | |

Fees Information

An instalment fee for processing this application is payable at the time of lodgement and must accompany your application in order for it to be lodged. Please note that if the instalment fee is insufficient to cover the actual and reasonable costs of work undertaken to process the application you will be required to pay any additional costs. Invoiced amounts are payable by the 20th of the month following invoice date. You may also be required to make additional payments if your application requires notification.

Declaration concerning Payment of Fees

I/we understand that the Council may charge me/us for all costs actually and reasonably incurred in processing this application. Subject to my/our rights under Sections 357B and 358 of the RMA, to object to any costs, I/we undertake to pay all and future processing costs incurred by the Council. Without limiting the Far North District Council's legal rights if any steps (including the use of debt collection agencies) are necessary to recover unpaid processing costs I/we agree to pay all costs of recovering those processing costs. If this application is made on behalf of a trust (private or family), a society (incorporated or unincorporated) or a company in signing this application I/we are binding the trust, society or company to pay all the above costs and guaranteeing to pay all the above costs in my/our personal capacity.

Bates

Diane

Name: (please write in full)

Signature: (signature of bill payer

15. Important Information:

Note to applicant

You must include all information required by this form. The information must be specified in sufficient detail to satisfy the purpose for which it is required.

You may apply for 2 or more resource consents that are needed for the same activity on the same form. You must pay the charge payable to the consent authority for the resource consent application under the Resource Management Act 1991.

Fast-track application

Under the fast-track resource consent process, notice of the decision must be given within 10 working days after the date the application was first lodged with the authority, unless the applicant opts out of that process at the time of lodgement. A fast-track application may cease to be a fast-track application under section 87AAC(2) of the RMA.

Privacy Information:

MANDATORY

Once this application is lodged with the Council it becomes public information. Please advise Council if there is sensitive information in the proposal. The information you have provided on this form is required so that your application for consent pursuant to the Resource Management Act 1991 can be processed under that Act. The information will be stored on a public register and held by the Far North District Council. The details of your application may also be made available to the public on the Council's website, www.fndc.govt.nz. These details are collected to inform the general public and community groups about all consents which have been issued through the Far North District Council.

Date 06.05.25

15. Important information continued...

Declaration

The information I have supplied with this application is true and complete to the best of my knowledge.

| Name: (please write in full) | | |
|------------------------------|--|------|
| Signature: | | Date |
| | A signature is not required if the application is made by electronic means | |

Checklist (please tick if information is provided)

- Payment (cheques payable to Far North District Council)
- A current Certificate of Title (Search Copy not more than 6 months old)
- Details of your consultation with lwi and hapū
- Copies of any listed encumbrances, easements and/or consent notices relevant to the application
- Applicant / Agent / Property Owner / Bill Payer details provided
- Location of property and description of proposal
- Assessment of Environmental Effects
- Written Approvals / correspondence from consulted parties
- Reports from technical experts (if required)
- Copies of other relevant consents associated with this application
- Location and Site plans (land use) AND/OR
- Location and Scheme Plan (subdivision)
- Elevations / Floor plans
- Topographical / contour plans

Please refer to Chapter 4 of the District Plan for details of the information that must be provided with an application. Please also refer to the RC Checklist available on the Council's website. This contains more helpful hints as to what information needs to be shown on plans.

Oruaiti School Upgrade

Land Use Consent and Outline Plan Application 2554 State Highway 10, Mangōnui Assessment of Environmental Effects and Statutory Analysis 8 May 2025



Prepared for: Ministry of Education



B&A Reference:

WNG25080

Status:

Final Revision

Date:

8 May 2025

Prepared by:

MAR M

Melissa McGrath Senior Associate, Barker & Associates Limited

Reviewed by:

M. Dalh_

Makarena Dalton Senior Associate, Barker & Associates Limited



Contents

| 1.0 | Applicant and Property Details | 6 |
|------|---|----|
| 2.0 | Background | 8 |
| 2.1 | Pre-lodgement Consultation | 8 |
| 3.0 | Site Context | 8 |
| 3.1 | Site Description and Locality | 8 |
| 4.0 | Proposal | 10 |
| 5.0 | Reasons for Application | 12 |
| 5.1 | Operative Far North District Plan | 12 |
| 5.2 | National Environmental Standard – Contaminated Soils | 14 |
| 5.3 | National Environmental Standard - Freshwater | 14 |
| 5.4 | Land Use Activity Status | 15 |
| 6.0 | Outline Plan Assessment | 15 |
| 6.1 | Height, Shape and Bulk of Work | 15 |
| 6.2 | The location on the Site of the Public Work, Project, or Work | 15 |
| 6.3 | Likely Finished Contour of the Site | 15 |
| 6.4 | Vehicular Access, Circulation, and the Provision for Parking | 15 |
| 6.5 | Landscaping Proposed | 15 |
| 6.6 | Any Other Matters to Avoid, Remedy, or Mitigate any Adverse Effects on the Environment. | 16 |
| 7.0 | Land Use Consent Public Notification Assessment (Sections 95A, 95C and 95D) | 16 |
| 7.1 | Assessment of Steps 1 to 4 (Sections 95A) | 16 |
| 7.2 | Section 95D Statutory Matters | 17 |
| 7.3 | Land Excluded from the Assessment | 17 |
| 7.4 | Assessment of Effects on the Wider Environment | 18 |
| 7.5 | Summary of Effects | 21 |
| 7.6 | Public Notification Conclusion | 21 |
| 8.0 | Land Use Consent Limited Notification Assessment (Sections 95B, 95E to 95G) | 21 |
| 8.1 | Assessment of Steps 1 to 4 (Sections 95B) | 21 |
| 8.2 | Section 95E Statutory Matters | 22 |
| 8.3 | Assessment of Effects on Persons | 23 |
| 8.4 | Limited Notification Conclusion | 23 |
| 9.0 | Consideration of Applications (Section 104) | 24 |
| 9.1 | Statutory Matters | 24 |
| 9.2 | Weighting of Proposed Plan Changes: Proposed Far North District Plan | 24 |
| 10.0 | Effects on the Environment (Section 104(1)(A)) | 24 |
| 11.0 | District Plan and Statutory Documents (Section 104(1)(B)) | 25 |
| 11.1 | Objectives and Policies of the Far North District Plan (Operative) | 25 |
| 11.2 | Objectives and Policies of the Proposed Far North Plan | 25 |
| 11.3 | Regional Policy Statement for Northland (RPS) | 25 |
| 11.4 | Summary | 26 |



| 12.0 | Part 2 Matters | 26 |
|-----------------------------|---|-----------------------|
| 13.0 | Other Matters (Section 104(1)(C)) | 27 |
| 13.1 | Record of Title Interests | 27 |
| | | |
| 14.0 | Conclusion | 27 |
| 14.0 14.1 | Conclusion Outline Plan of Works | 27 27 |
| 14.0 14.1 14.2 | Conclusion Outline Plan of Works Land Use Consent | 27 27 27 |



Appendices

- Appendix 1: Record of Title
- Appendix 2: MoE Authority Letter
- Appendix 3: Civil Drawings
- Appendix 4: Stormwater Neutrality Report
- Appendix 5: Ecological Assessment Memorandum
- Appendix 6: Ngātikahu Ki Whangaroa Approval Letter
- Appendix 7: Correspondence with Waka Kotahi, New Zealand Transport Agency



1.0 Applicant and Property Details

| To: | Far North District Council |
|-------------------------|--|
| Site Address: | 2554 State Highway 10, Mangōnui |
| Applicant Name: | Ministry of Education (refer to Appendix 2 confirming detail) |
| Address for Service: | Barker & Associates Ltd PO Box 37, Whangārei 0140 Level 1, 136 Bank Street Whangārei 0112 Attention: Melissa McGrath |
| Legal Description: | Part Allotment 18A Parish of Mangōnui East (refer to Gazette Notice as Appendix 1) Stopped Road Survey Office Plan 51928 State Highway |
| Site Area: | 4.121ha |
| Site Owner: | Her Majesty the Queen |
| District Plan: | Operative Far North District Plan (ODP) Proposed Far North District Plan (PDP) |
| Zoning: | ODP : Rural Production Zone PDP : Rural Production Zone |
| Overlays & Controls: | ODP:NRCFloodSusceptibleandStatutoryAcknowledgement - Oruaiti River and itstributarieswithin the Ngātikahu ki Whangaroa area of interest asshown on OTS-116-19PDP:Statutory Acknowledgement, Treaty SettlementArea of Interest, River Flood Hazard Zone10- and100-Year ARI Event, Coastal Flood Zones 1, 2 and 3. |
| Designations: | ODP : Minister of Education (ME57) and New Zealand Transport Agency (State Highway 10 designation). PDP : Minister of Education (MEDU57) and New Zealand Transport Agency (NZTA -2). |
| Additional Limitations: | Nil |
| Locality Diagram: | Refer to Figure 1 |



Brief Description of Proposal:

Summary of Reasons for Consent:

Entrance upgrade, carparking, pedestrian footpaths (internal), stormwater and wastewater upgrade and associated works.

Outline Plan: Required as proposal does not comply with ME57 conditions under the ODP. Outline Plan is required pursuant section 176A of the RMA.

Works within the State Highway does not require Outline Plan approval.

Land Use Consent: Required for stormwater pursuant to rule 8.6.2.1 and for access to State Highway pursuant to rule 15.1.6C.2, the application is a Discretionary Activity.

Northland Regional Council: Resource consent is required under the Proposed Regional Plan for Northland (**PRP**) for non-compliances with various rules, this is being sought separately from Northland Regional Council, concurrently to this application to FNDC.



2.0 Background

This report has been prepared to support an application for an Outline Plan of Works (**OPW**) and land use consent by Ministry of Education (**MoE**) to provide for upgrade works to the existing Oruaiti School.

MoE is a requiring authority pursuant to Section 167 of the RMA and is specified as a requiring authority in Designations – Minister of Education – ME57 of the Operative Far North District Plan *(ODP)*. This OPW is compiled and lodged in accordance with section 176 and section 176A of the Resource Management Act 1991 (**RMA**). A letter from MoE is attached as **Appendix 2** that states Land Development Engineering (**LDE**) has the authority to act on their behalf.

Additionally, Land Use Consent is sought as proposed works will extend over land at the edge of State Highway which is owned and administered by New Zealand Transport Agency (Waka Kotahi).

The proposed school upgrades are being undertaken, to improve vehicle and pedestrian safety, as currently designed the school parking areas are gravelled and do not provide adequate safety measures for vehicles and the school children to cross safely. Further, the school's on-site wastewater system and stormwater management need upgrading.

Oruaiti School is a primary and intermediate school, catering for students from years' one to eight. The school currently has approximately >190 students and 15 staff members.

2.1 Pre-lodgement Consultation

Tangata Whenua

Ngātikahu Ki Whangaroa have a statutory acknowledgement over Oruaiti River and its tributaries within the Ngātikahu Ki Whangaroa Area of Interest.

Accordingly, engagement was undertaken and the result of that consultation with Ngātikahu Ki Whangaroa are enclosed at **Appendix 6**, confirming approval for the proposal.

Accordingly, we asked Council for the relevant hapū contact details and have sent an email outlining what is proposed and have asked for their feedback, written approval has been received from Ngātikahu Ki Whangaroa (refer to **Appendix 6**).

Waka Kotahi

With respect to Waka Kotahi, pre-lodgement engagement has been undertaken and approval indicated (refer to **Appendix 7**).

3.0 Site Context

3.1 Site Description and Locality

Oruaiti School is located at 2554 State Highway 10 and is legally described as Part Allotment 18A Parish of Mangōnui East (see **Figure 1** below), measuring 4.121ha. The site is zoned as Rural Production Zone and subject to designation ME57.





Figure 1: Locality plan, site highlighted purple. Source: Emap.

Access to the site is from an existing vehicle crossing, gravel driveway and parking area via State Highway 10. The site consists of multiple school buildings, various sheds, a swimming pool, retaining walls boundary fences, gravel driveways and parking areas. Vegetation comprises mainly grass and small-medium sized trees.

Proposed works will extend across a very small sliver of land owned and administered by Waka Kotahi, which is zoned as Rural Production Zone and located outside of the school designation.



Figure 2: Locality plan, adjacent site identified. Source: Emap.

To the north, east, south, and west the surrounding locality is also zoned as Rural Production Zone and is predominantly rural in nature, featuring single-storey dwellings, on large lots. To the south are large lots that are predominantly vacant.



4.0 Proposal

The scope of work proposed at Oruaiti School includes the following (see Civil and Structural Drawings in **Appendix 3**), an overall site layout is shown in **Figure 3** below:

- Upgraded vehicle crossing;
- A new asphalt parking area;
- Internal foot pathing;
- Traffic island;
- Internal pedestrian footpath connecting the two car parking areas;
- Retaining walls;
- An upgraded stormwater system; and
- Associated earthworks.



Figure 3: Overall site plan, source LDE.

The Applicant offers the following conditions of consent in accordance with consultation and approval from New Zealand Transport Agency:

Pre-construction Agreements

1. Prior to the commencement of any works on the vehicle crossing or within the state highway corridor, the consent holder must provide evidence of the following as approved by the New Zealand Transport Agency, or written confirmation from New Zealand Transport Agency that the approval is not required:

a. A 'Corridor Access Request' (CAR)



b. An 'Agreement as to Work on State Highway'

c. A 'Work access permit' (WAP) The application must include detailed construction drawings showing full details of the works, methodology and a Traffic Management Plan.

2. A formal licence to occupy / network licence (as advised by NZ Transport Agency) to occupy NZTA managed Crown land for the purposes of completing the proposed works shall be agreed to between the applicant and the NZ Transport Agency prior to commencement of any construction associated with this approval.

General Conditions

3. That the vehicle crossing, parking, manoeuvring and associated works be undertaken in general accordance with the plans referenced below and to the satisfaction of the New Zealand Transport Agency Network Manager:

a. 'Parking Area & Accessway Earthworks Layout" dated 1 May 2024 and referenced as 'C200', revision A, prepared by Land Development & Engineering.

b. 'Parking Area & Accessway Overall Site Plan' dated 1 May 2024 and referenced as 'C300', revision A, prepared by Land Development & Engineering.

c. 'Parking Area & Accessway Design Plan Sheet 1 of 2' dated 1 May 2024 and referenced as 'C301', revision A, prepared by Land Development & Engineering.

d. 'Parking Area & Accessway Design Plan Sheet 2 of 2' dated 1 May 2024 and referenced as 'C302', revision A, prepared by Land Development & Engineering.

e. 'Parking Area & Accessway Longitudinal Sections' dated 1 May 2024 and referenced as 'C310', revision A, prepared by Land Development & Engineering.

f. 'Parking Area & Accessway Stormwater Management Plan' dated 1 May 2024 and referenced as 'C400', revision A, prepared by Land Development & Engineering.

g. 'Parking Area & Accessway Stormwater Longitudinal Sections Sheet 1 of 2' dated 1 May 2024 and referenced as 'C410', revision A, prepared by Land Development & Engineering.

h. 'Parking Area & Accessway Stormwater Longitudinal Sections Sheet 1 of 2' dated 1 May 2024 and referenced as 'C411', revision A, prepared by Land Development & Engineering.

4. Consultation with the New Zealand Transport Agency shall be undertaken in the event changes are made from the plans initially submitted to the New Zealand Transport Agency (and referenced above). This consultation will confirm if the changes are in general accordance with the approved plans or confirm if additional approvals are required.

After construction

5. Prior to the use of the upgraded crossing and vehicle manoeuvring areas, the consent holder shall provide to Council, correspondence from New Zealand Transport Agency confirming that works to the state highway, including vehicle crossings, have been constructed to New Zealand Transport Agency standards or in general accordance with this approval.

Advice Notes:

• The maintenance of all structures and infrastructure associated with the proposal is the responsibility of the applicant. These must be maintained in good order and condition, and otherwise to the satisfaction of NZTA, and subject to any directions and conditions stipulated



by NZTA. This advice note may be superseded by a condition on a licence to occupy / network licence granted by the NZ Transport Agency.

• The applicant should provide a copy of any resource consent application associated with the proposal to NZTA once lodged for s95E approval.

5.0 Reasons for Application

5.1 Operative Far North District Plan

The site is subject to the provisions of the Operative Far District Plan Zoned Rural Production Zone and partially zoned Designation 'ME57' and State Highway 10 Designation.

Designation ME57

The purpose of designation ME57 is for 'Education Purposes'. The proposed works are clearly in accordance with the purpose of the designation.

Designation 'ME57' is subject to the following conditions all of which are complied with by the proposed upgrade:

Table 1: Summary of ME57 Conditions

| ME57 Conditions | Compliance |
|---|---|
| 1. Car Parking (all designations) Where new development increases the number of classrooms (or classroom equivalents) on a particular site, additional parking shall be provided at the rate of not less than two car parks per additional classroom or classroom equivalent, except where the Council accepts, on the basis of a specifically commissioned car parking study by an appropriately qualified engineer, that a lesser level is appropriate. | It is not proposed to construct additional classrooms or classroom equivalents. |
| 2. Traffic Management | N/A – Oruaiti School is not listed under this condition. |
| 3. Building Setbacks (all designations) | No buildings are proposed. |
| New buildings or structures shall comply with the setback from boundaries for permitted activities for the applicable underlying zone provided that the setback can be reduced where the written consent to such a reduction has been obtained from the registered proprietor of the relevant adjacent land. | |



| 4. Conditions Relating to ME73 and ME97 | N/A - Oruaiti School is ME57 |
|---|---|
| 5. Conditions Relating to ME97, ME11 and ME116 | N/A - Oruaiti School is ME57 |
| 6. Outstanding Landscapes Conditions apply to ME52, ME86, ME102 and ME117, and apply to those parts of the sites identified by the Resource Maps as being within outstanding landscape notation | N/A - Oruaiti School is ME57 and does not contain any ONL over the site. |
| 7. Coastal Hazard Condition relating to ME115 | N/A - Oruaiti School is ME57 |
| 8. Flood Susceptible Areas The following condition applies to those designations numbered ME57, and applies only to those parts of the sites which fall within flood susceptible areas as identified on the Council's Hazard Register: | Stormwater Neutrality Report has been provided by LDE and is included as Appendix 4 . |
| (a) Any outline plan of works shall be accompanied by a report or certificate from a suitably qualified and experienced professional indicating that the sub-floor design is able to withstand, and that the floor levels are above, a 50-year return period flood. | |
| 9. Conditions Relating to ME251 | N/A - Oruaiti School is ME57 |
| 10. Conditions relating to ME59 and Lot 2 DP 150885 | N/A - Oruaiti School is ME57 |
| 11. Conditions Relating to ME252 | N/A - Oruaiti School is ME57 |

The proposed upgrading to the vehicle entrance, driveway and car parking will require outline plan approval. MoE seek approval of an Outline Plan of Public Work approval pursuant to section 176A of the RMA.

Designation State Highway 10

Proposed upgrading works will extend to within the State Highway 10 designation, vehicle crossing, parking and stormwater fall within the purpose and scope of the designation. Waka Kotahi as Requiring Authority of the designation have issued written approval under Section 176A of the RMA.

Rural Production Zone



The proposed works will not result in buildings being located within the site outside the designations, however, proposed works will include parking, impervious surfaces, vehicle access etc being located within a small area of the site located outside of the designations and located within the Rural Production Zone.

• Rule 8.6.5.1.3 Stormwater- The proposed works will result in more than 15% of the gross site area covered by impervious areas and will also infringe the controlled activity limit of 20% of the gross site area covered by impervious areas under rule 8.6.2.1. Discretionary Activity under rule 8.6.5.4.

Transport Chapter

The proposed works will result in new vehicle access to State Highway 10 being located within a small area of the site located outside of the ME57 designation and zoned Rural Production Zone.

• Rule 15.1.6C.1.1 Private Accessway in All Zones – the proposed access is from State Highway 10. Discretionary Activity under rule 15.1.6C.2.

5.2 National Environmental Standard – Contaminated Soils

The Resource Management (National Environmental Standard for Assessing and Managing Contaminants in Soil to Protect Human Health) Regulations 2011 (**NES-CS**) were gazetted on 13th October 2011 and took effect on 1st January 2012. The subject site is not identified as a HAIL site as such NEC-CS does not apply.

5.3 National Environmental Standard - Freshwater

The Resource Management (National Environmental Standards for Freshwater) Regulations 2020 (**NES-F**) were gazetted on 3rd August 2020 and took effect on 3rd September 2020. The standards apply to activities that may have an effect to freshwater, particularly wetlands.

The proposal does not seek to undertake earthworks, vegetation clearance or construction of structures within or within 10m of a wetland, however, the proposal does seek to manage stormwater within the school site which is located within 100m of the natural inland wetland located 30m south of the proposed works. The Wild Ecology assessment (**Appendix 5**) confirms that the proposal will not result in any drainage of the natural wetland.

The proposal does seek to discharge stormwater via the existing system within 100m of the natural inland wetland, Wild Ecology assessment (**Appendix 5**) confirms that:

"While the stormwater diversions associated with the site development will occur within a 100m setback from the identified wetland area and may have a hydrological connection with these areas, they will not change or are unlikely to change the water level range or hydrological function of the wetland"

"While stormwater will be discharged to water and may have a hydrological connection to the wetland and will likely enter the wetland, the incremental increase in impervious surfaces and discharge (15L/s) associated with the car park development, the overall volume of water entering the aquatic features is not expected to increase to any detectable level. These discharges are not likely to change the water level range or hydrological function of the wetland areas."

As such, the proposal does not require consent under the NES-F.



5.4 Land Use Activity Status

Overall, this application is for a discretionary activity.

6.0 Outline Plan Assessment

Pursuant to Section 176A of the Resource Management Act (the Act), the Outline Plan for the proposed works must show the following:

- a) the height, shape, and bulk of the public work, project, or work; and
- b) the location on the site of the public work, project, or work; and
- c) the likely finished contour of the site; and
- d) the vehicular access, circulation, and the provision for parking
- e) the landscaping proposed; and

f) any other matters to avoid, remedy, or mitigate any adverse effects on the environment.

6.1 Height, Shape and Bulk of Work

The proposed works are largely civil site works to establish, parking, and stormwater management systems, no buildings are proposed.

6.2 The location on the Site of the Public Work, Project, or Work

The bulk of the works will occur to the north west of the existing school buildings, with earthworks and upgrading of the vehicle crossing and parking area. with pedestrian paths extending centrally to the school.

Stormwater upgrades are largely focused upon the parking areas being located underground; this will comprise a stormwater pipe extension, manholes, catchpits and a swale as illustrated in **Appendix 3**.

6.3 Likely Finished Contour of the Site

The contour of the subject site remains largely unchanged, as detailed in the earthworks cut/fill volume plans, works are limited to the parking areas and stormwater drain to establish access and flat parking areas as illustrated in **Appendix 3**.

6.4 Vehicular Access, Circulation, and the Provision for Parking

The proposed works seek to improve existing vehicle access, manoeuvring and increase onsite parking via the creation of the parking area. 35 car parking spaces will be provided onsite, which comply with the Far North District Council Environmental Engineering Standards.

6.5 Landscaping Proposed

No landscaping is proposed, all exposed areas of earthworks will be regressed.



6.6 Any Other Matters to Avoid, Remedy, or Mitigate any Adverse Effects on the Environment.

The proposed works have been designed to mitigate potential stormwater, flood risk and traffic effects as detailed further in this application.

7.0 Land Use Consent Public Notification Assessment (Sections 95A, 95C and 95D)

7.1 Assessment of Steps 1 to 4 (Sections 95A)

Section 95A specifies the steps the council is to follow to determine whether an application is to be publicly notified. These are addressed in statutory order below.

7.1.1 Step 1: Mandatory public notification is required in certain circumstances

Step 1 requires public notification where this is requested by the applicant; or the application is made jointly with an application to exchange of recreation reserved land under section 15AA of the Reserves Act 1977.

The above does not apply to the proposal.

7.1.2 Step 2: If not required by step 1, public notification precluded in certain circumstances

Step 2 describes that public notification is precluded where all applicable rules and national environmental standards preclude public notification; or where the application is for a controlled activity; or a restricted discretionary, discretionary or non-complying boundary activity.

In this case, the applicable rules do not preclude public notification, and the proposal is not a controlled activity or boundary activity. Therefore, public notification is not precluded.

7.1.3 Step 3: If not required by step 2, public notification required in certain circumstances

Step 3 describes that where public notification is not precluded by step 2, it is required if the applicable rules or national environmental standards require public notification, or if the activity is likely to have adverse effects on the environment that are more than minor.

As noted under step 2 above, public notification is not precluded, and an assessment in accordance with section 95A is required, which is set out in the sections below. As described below, it is considered that any adverse effects will be less than minor.

7.1.4 Step 4: Public notification in special circumstances

If an application is not required to be publicly notified as a result of any of the previous steps, then the council is required to determine whether special circumstances exist that warrant it being publicly notified.

Special circumstances are those that are:



- Exceptional or unusual, but something less than extraordinary; or
- Outside of the common run of applications of this nature; or
- Circumstances which make notification desirable, notwithstanding the conclusion that the adverse effects will be no more than minor.

It is considered that there is nothing noteworthy about the proposal. It is therefore considered that the application cannot be described as being out of the ordinary or giving rise to special circumstances.

7.2 Section 95D Statutory Matters

In determining whether to publicly notify an application, section 95D specifies a council must decide whether an activity will have, or is likely to have, adverse effects on the environment that are more than minor.

In determining whether adverse effects are more than minor:

• Adverse effects on persons who own or occupy the land within which the activity will occur, or any land adjacent to that land, must be disregarded.

The land to be excluded from the assessment is listed in section 7.3 below.

• Adverse effects permitted by a rule in a plan or national environmental standard (the 'permitted baseline') may be disregarded.

In this case, access from State Highway is not permitted therefore there is no relevant permitted baseline.

• Trade competition must be disregarded.

This is not considered to be a relevant matter in this case.

• The adverse effects on those persons who have provided their written approval must be disregarded.

No persons have provided their written approval for this proposal.

7.3 Land Excluded from the Assessment

In terms of the tests for public notification (but not for the purposes of limited notification or service of notice), the adjacent properties to be excluded from the assessment are shown in **Figure 4** below, and include:

- Allotment 25-26 and Part Allotment 10 Block VI Mangonui Survey District;
- 2591 State Highway 10, Mangōnui; and
- Parcel 8270033.





Figure 4: Adjacent properties (shown in blue) in relation to subject site. Source: Emaps.

7.4 Assessment of Effects on the Wider Environment

The following sections set out an assessment of wider effects of the proposal, and it is considered that effects in relation to the following matters are relevant:

- Flooding and stormwater management;
- Transportation;
- Construction activities; and
- Cultural effects.

These matters are set out and discussed below.

7.4.1 Flooding and stormwater management

The subject site is identified by NRC as subject to Priority River Flood Hazards, 10 year, 50 year and 100 year. The proposed development seeks to manage potential flood risk via the management of stormwater, as detailed in the Stormwater Neutrality Report prepared by LDE and attached as **Appendix 4**.





Figure 5: Natural Hazard Maps Source: NRC Website.

LDE concludes that the proposed stormwater management system will improve downstream flood risk. For these reasons, it is considered that the adverse effects on the wider environment are less than minor and acceptable.

7.4.2 Transportation

Traffic, access and parking matters have been considered in detailed design prepared by LDE (see **Appendix 3**). The proposal seeks to establish improved access, parking and manoeuvring to service the existing school, parking and access has been designed to comply with Far North Council's Environmental Engineering Standards. As a result of works in proximity to State Highway 10 written approval has been sought and obtained from Waka Kotahi. It is considered that, subject to implementation of suitable conditions of consent that any adverse effects with respect to transportation will be managed and acceptable.

7.4.3 Construction activities

Approximately 400m³ of net fill (20m³ cut and 460m³ fill) across an area of 2,760m² of earthworks is proposed, to establish flat access and parking areas. This is outlined on the cut and fill plan prepared by LDE (see **Appendix 3**).



Due to the area that the proposed earthworks are undertaken, resource consent under the PRP is required and an application to NRC will be made concurrently with this application. Silt and sediment control measures are proposed to be implemented in accordance with the Erosion and Sediment Control Guidelines for Land Disturbing Activities in the Auckland Region (2016) for the duration of the activity.

On the basis of the above, it is considered that any adverse effects associated with silt and sediment runoff (and resulting effects on water quality) will be less than minor.

When having regard to the nature of the construction activities it is expected that the works will comply with the District Plan construction noise limits which specifically reference New Zealand Standard NZS 6803: 1999 "Acoustics - Construction Noise". Any adverse construction noise effects would be temporary in nature.

There is sufficient space on the subject site to provide parking for construction vehicles. It is considered that traffic and parking capacity effects of the construction period will be less than minor and temporary in nature.

As with other similarly sized projects, it is expected that site works will be managed in accordance with a construction management plan that will set measures to manage potential adverse effects associated with the construction phase of the project. A condition to this effect is offered as part of the application.

Overall, it is considered that any adverse construction effects will be less than minor and acceptable.

7.4.4 Cultural Effects

There are no sites of historic heritage including sites of significance to Maori identified on FNDC's GIS system and a search of ArchSite has confirmed that there are no archaeological sites in proximity to the proposed works.

Ngātikahu Ki Whangaroa have a statutory acknowledgement over Oruaiti Stream and its tributaries within the Ngātikahu Ki Whangaroa Area of Interest, as such the Applicant has undertaken consultation with Ngātikahu Ki Whangaroa and gained written approval in support of the application.

Therefore, the proposal will have no known adverse effects on cultural values, historic heritage or the surrounding environment.

7.4.5 Ecological Effects

The application site is currently used for existing educational purposes and does not contain any protected or significant vegetation. Stormwater from the proposed upgrades is proposed to be directed to the Oruaiti stream and wetland. Wild Ecology has completed a desktop assessment (**Appendix 5**) of the proposed works on the Oruaiti Stream and wetland area concluding:

"It is considered that proposed car park redevelopment works at Oruaiti School would result in negligible potential adverse ecological effect on the identified aquatic habitats. No adverse effect on the identified natural inland wetland area or Oruaiti Stream is anticipated – the nearest natural inland wetland area is located approximately 30m south of the works footprint. Any potential adverse ecological effects on natural inland wetland features associated with the proposal can be avoided, minimised or mitigated through applying appropriate construction and sediment/erosion



controls. Provided that they are implemented successfully, adverse effects on the environment would be negligible. It is considered that the construction of the car park associated with the proposal is not likely to change the water level range or hydrological function of the identified natural inland wetland area and will not result, or is not likely to result, in the complete or partial drainage of all or part of a natural inland wetland. Therefore, it is considered that the proposal does not require a consent under NES-FW (2020)"

For this reason, it is considered that the proposal will have less than minor and acceptable ecological effects.

7.5 Summary of Effects

Overall, it is considered that any adverse effects on the environment relating to this proposal will be no more than minor.

7.6 Public Notification Conclusion

Having undertaken the section 95A public notification tests, the following conclusions are reached:

- Under step 1, public notification is not mandatory;
- Under step 2, public notification is not precluded;
- Under step 3, public notification is not required as it is considered that the activity will result in no more than minor adverse effects; and
- Under step 4, there are no special circumstances.

Therefore, based on the conclusions reached under steps 3 and 4, it is recommended that this application be processed without public notification.

8.0 Land Use Consent Limited Notification Assessment

(Sections 95B, 95E to 95G)

8.1 Assessment of Steps 1 to 4 (Sections 95B)

If the application is not publicly notified under section 95A, the council must follow the steps set out in section 95B to determine whether to limited notify the application. These steps are addressed in the statutory order below.

8.1.1 Step 1: Certain affected protected customary rights groups must be notified

Step 1 requires limited notification where there are any affected protected customary rights groups or customary marine title groups; or affected persons under a statutory acknowledgement affecting the land.

The above does not apply to this proposal.



8.1.2 Step 2: If not required by step 1, limited notification precluded in certain circumstances

Step 2 describes that limited notification is precluded where all applicable rules and national environmental standards preclude limited notification; or the application is for a controlled activity (other than the subdivision of land).

In this case, the applicable rules do not preclude limited notification and the proposal is not a controlled activity. Therefore, limited notification is not precluded.

8.1.3 Step 3: If not precluded by step 2, certain other affected persons must be notified

Step 3 requires that, where limited notification is not precluded under step 2 above, a determination must be made as to whether any of the following persons are affected persons:

- In the case of a boundary activity, an owner of an allotment with an infringed boundary;
- In the case of any other activity, a person affected in accordance with s95E.

The application is not for a boundary activity, and therefore an assessment in accordance with section 95E is required and is set out below.

Overall, it is considered that any adverse effects on persons will be less than minor, and accordingly, that no persons are adversely affected.

8.1.4 Step 4: Further notification in special circumstances

In addition to the findings of the previous steps, the council is also required to determine whether special circumstances exist in relation to the application that warrant notification of the application to any other persons not already determined as eligible for limited notification.

In this instance, having regard to the assessment in section 7.1.4 above, it is considered that special circumstances do not apply.

8.2 Section 95E Statutory Matters

If the application is not publicly notified, a council must decide if there are any affected persons and give limited notification to those persons. A person is affected if the effects of the activity on that person are minor or more than minor (but not less than minor).

In deciding who is an affected person under section 95E:

- Adverse effects permitted by a rule in a plan or national environmental standard (the 'permitted baseline') may be disregarded;
- Only those effects that relate to a matter of control or discretion can be considered (in the case of controlled or restricted discretionary activities); and
- The adverse effects on those persons who have provided their written approval must be disregarded.

These matters were addressed in section 7.2 above, and no written approvals have been obtained.

Having regard to the above provisions, an assessment is provided below.



8.3 Assessment of Effects on Persons

Wider effects, such as flood hazard and stormwater management, transportation, construction activities, cultural effects and ecology were considered in section 7.4 above, and considered to be no more than minor.

Adverse effects in relation to amenity and transportation effects on persons are considered below.

8.3.1 Allotment 25-26 and Part Allotment 10 Block VI Mangonui Survey District

This site is a larger rural site located adjacent to the school. The proposed works will be adjacent to this site but will not result in a change to the existing school activity in proximity to this site. The improved access and safety of traffic manoeuvring to and from the school which will improve the amenity of this site. Therefore, the proposal is considered to have less than minor transport and amenity effects to this property.

8.3.2 2591 State Highway 10, Mangōnui and Parcel 8270033.

These sites are larger rural sites located on the southern side of State Highway 10. The proposed works will not be directly adjacent to these sites, the works will improve the access and manoeuvring to Oruaiti School which will, in turn, improve transport safety for these sites. Therefore, the proposal is considered to have less than minor amenity and transport effects on these properties.

8.3.3 Summary of Effects

Taking the above into account, it is considered that any adverse effects on persons at the aforementioned properties will be less than minor in relation to amenity and transport effects. Wider effects were assessed in section 7.4 above and are considered to be no more than minor.

It is considered, therefore, that there are no adversely affected persons in relation to this proposal.

8.4 Limited Notification Conclusion

Having undertaken the section 95B limited notification tests, the following conclusions are reached:

- Under step 1, limited notification is not mandatory;
- Under step 2, limited notification is not precluded;
- Under step 3, limited notification is not required as it is considered that the activity will not result in any adversely affected persons; and
- Under step 4, there are no special circumstances.

Therefore, it is recommended that this application be processed without limited notification.



9.0 Consideration of Applications (Section 104)

9.1 Statutory Matters

Subject to Part 2 of the Act, when considering an application for resource consent and any submissions received, a council must, in accordance with section 104(1) of the Act have regard to:

- Any actual and potential effects on the environment of allowing the activity;
- Any relevant provisions of a national environmental standard, other regulations, national policy statement, a New Zealand coastal policy statement, a regional policy statement or proposed regional policy statement; a plan or proposed plan; and
- Any other matter a council considers relevant and reasonably necessary to determine the application.

As a discretionary activity, section 104B of the Act states that a council:

- (a) may grant or refuse the application; and
- (b) if it grants the application, may impose conditions under section 108.

9.2 Weighting of Proposed Plan Changes: Proposed Far North District Plan

On the 27th July 2022 FNDC notified their PDP. At the time of preparing this AEE, only rules identified as having immediate legal effect have been considered. This will remain the case until FNDC releases a decision on the PDP (this will occur once hearings have been completed). As such, it is considered that significantly more weight should be placed on the ODP provisions.

10.0 Effects on the Environment (Section 104(1)(A))

Having regard to the actual and potential effects on the environment of the activity resulting from the proposal, it was concluded in the assessment above that any wider adverse effects relating to the proposal will be no more than minor and that no persons would be adversely affected by the proposal.

Further, it is considered that the proposal will also result in positive effects including, improved management of stormwater within the school site and increased pedestrian and vehicular safe and efficient access to the public school site.

Overall, it is considered that when taking into account the positive effects, any actual and potential adverse effects on the environment of allowing the activity are acceptable.



11.0 District Plan and Statutory Documents (Section 104(1)(B))

11.1 Objectives and Policies of the Far North District Plan (Operative)

Chapter 8.6 Rural Production Zone

The rural production zone applies to the majority of land within the district, and seeks to enable the continuation of a wide range of activities for existing and future activities that are compatible with the productive purpose of the zone.

The objectives and policies of the zone seek to provide for a wide range of activities, while managing the effects of activities that are incompatible with the rural production zone. The proposed works associated with an existing school is not considered to be incompatible, as it does not restrict existing activities or generate reverse sensitivity effects that may limit the rural productive purpose of the zone.

In conclusion, the proposed activity is considered to be consistent with the outcomes of the zone.

Chapter 12.4 Natural Hazards

The objectives and policies of the natural hazards chapter are contained within Chapter 12.4 of the ODP and seek to reduce the threat of natural hazards to life, property and the environment.

The proposed works does not introduce new vulnerable activities and is proposed to improve management of flood risk via stormwater management.

The proposal is considered consistent with the intent of the natural hazards chapter.

Chapter 15.1 Traffic, Parking and Access

The objectives and policies of this chapter seek to ensure that parking and access in association with land uses on private property are provided in a manner which encourages sustainable transportation. Seeking to minimise the adverse effects of traffic on the natural and physical environment.

The proposed works will improve parking, access and movement within the existing school site reducing adverse traffic effects.

The proposal is considered consistent with the intent of the traffic, parking and access chapter.

11.2 Objectives and Policies of the Proposed Far North Plan

The proposal results in works being undertaken within the Rural Production Zone. The proposal also interreacts with overlays that include the River Flood Hazard Zone – Priority Rivers and Regionwide Models. The proposal will

On this basis, the proposal is not considered to be contrary to, but is not entirely consistent with the anticipated outcomes of the PDP.

11.3 Regional Policy Statement for Northland (RPS)

The Northland Regional Policy Statement (RPS) covers the management of natural and physical resources across the Northland Region. The provisions within the RPS give guidance at a higher planning level in terms of the significant regional issues. As such it does not contain specific rules



that trigger the requirement for consent but rather give guidance to consent applications and the development of Plans on a regional level.

Objectives range from integrated catchment management, improvement of overall quality of Northland's water quality, maintaining ecological flows, protecting areas of significant indigenous ecosystems and biodiversity, sustainable management of natural and physical resources in a way that is attractive for business and investment that will improve the economic wellbeing. Enabling economic wellbeing, regional form, the role of tangata whenua kaitiaki role is recognised and provided for in decision making, risks and impacts of natural hazards are minimised, outstanding natural landscapes and features and historic heritage are protected from inappropriate subdivision, use and development.

Relevant policy has been identified and summarised as follows:

- Policy 4.2.1 seeks to improve the overall quality of Northland's water resources by, establishing freshwater objectives, reducing loads of sediment, nutrients and faecal matter to water and promoting and supporting the active management, enhancement and creation of vegetated riparian margins. The proposed development will manage effect of stormwater discharge on the fresh water of the Oruaiti Wetland and Stream.
- Policy 5.1.1 seeks to provide for subdivision, use and development that is located, designed and built in a planned and coordinated matter. The proposal will improve the functionality of an existing public school servicing the local community. With specific reference to 5.1.1(d) and (h), the proposal can be adequately serviced in terms of transportation, water, wastewater, and stormwater by existing and proposed infrastructure.
- According to Policy 7.1.1 subdivision, use and development of land will be managed to minimise risks of natural hazards. The proposed works seek to improve the management of stormwater within the school site to minimise the risk of natural hazards.
- Policy 8.1.2 requires the district council to recognise and provide for the relationship of tangata whenua and their culture and traditions, have particular regard to kaitiakitanga and take into account the principles of the Treaty of Waitangi including partnership when processing resource consents. No sites of cultural significance or heritage are identified within the subject site and written approval has been obtained from Ngātikahu Ki Whangaroa.

For these reasons, it is considered that the proposal is consistent with the relevant RPS provisions.

11.4 Summary

It is considered that the proposed development is generally in accordance with the objectives and policies of the ODP, PDP and RPS.

12.0 Part 2 Matters

Section 5 of Part 2 identifies the purpose of the RMA as being the sustainable management of natural and physical resources. This means managing the use, development and protection of natural and physical resources in a way that enables people and communities to provide for their social, cultural and economic well-being and health and safety while sustaining those resources for



future generations, protecting the life supporting capacity of ecosystems, and avoiding, remedying or mitigating adverse effects on the environment.

Section 6 of the Act sets out a number of matters of national importance including (but not limited to) the protection of outstanding natural features and landscapes and historic heritage from inappropriate subdivision, use and development.

Section 7 identifies a number of "other matters" to be given particular regard by Council and includes (but is not limited to) Kaitiakitanga, the efficient use of natural and physical resources, the maintenance and enhancement of amenity values, and maintenance and enhancement of the quality of the environment.

Section 8 requires Council to take into account the principles of the Treaty of Waitangi.

Overall, as the effects of the proposal are considered to be no more than minor, and the proposal accords with the relevant ODP, PDP and RPS objectives and policies, it is considered that the proposal will not offend against the general resource management principles set out in Part 2 of the Act.

13.0 Other Matters (Section 104(1)(C))

13.1 Record of Title Interests

The Record of Title for the site is not subject to any interests.

14.0 Conclusion

14.1 Outline Plan of Works

Overall, it is considered that pursuant to Section 176A of the Resource Management Act 1991, the proposal meets all applicable conditions for the designation and is consistent with the purpose of the designation.

14.2 Land Use Consent

The proposal involves works including entrance upgrade, carparking, pedestrian footpaths (internal), stormwater and wastewater upgrade for Oruaiti School at 2554 State Highway 10, Mangōnui within in the Far North District.

Based on the above report it is considered that:

- Public notification is not required as adverse effects in relation to stormwater, construction activities, transport, cultural effects and ecological are considered to be less than minor.
- There are also positive effects including improving stormwater management and improving pedestrian and vehicular safety and access to the school;
- Limited notification is not required as no persons are considered to be adversely affected by the proposal;
- The proposal accords with the relevant ODP, PDP and RPS objectives and policies; and



• The proposal is considered to be consistent with Part 2 of the Act.

It is therefore concluded that the proposal satisfies all matters the consent authority is required to assess, and that it can be granted on a non-notified basis.

Parcel: Part Allot 18A PSH OF Mangonui East

Summary

| Appellation | Part Allot 18A PSH OF Mangonui East | | | | | |
|-------------------------|-------------------------------------|-------------|--|--|--|--|
| Parcel Status | Current | | | | | |
| Intent | DCDB | | | | | |
| Associated Feature | - | | | | | |
| Land District | North Auckland | | | | | |
| Non Surveyed Definition | - | | | | | |
| Parcel ID | 4901722 | | | | | |
| Parcel Area | - | | | | | |
| Total Area | 5.3497 Ha | | | | | |
| Survey Number | Land District | Survey Date | | | | |
| <u>SO 11108</u> | North Auckland | - | | | | |
| Statutory Actions | | | | | | |
| Statutory Action | New Zealand Gazette 1904 p 1496 | | | | | |
| Туре | Gazette Notice | | | | | |
| Recorded | 4 April 2002 | | | | | |
| Action | Create | | | | | |
| Status | Current | | | | | |
| Statute | - | | | | | |
| Purpose | School Reserve | | | | | |
| Name | - | | | | | |
| Comments | - | | | | | |

Rural Lands in Otago Land District open for Sale or Selection.

RANFURLY, Governor.

I N pursuance and exercise of the powers and authorities conferred upon me by the one-hundred-and-thirty sixth section of "The Land Act. 1892," I, Uchter John Marz. Earl of Raufurly, the Governor of the Colmy of New Zealand, having received the report of the Surveyor General in this behalf, as in the said section is provided, do hereby declare that the rural lands described in the Schedule hereto shall be open for sale or selection on and siter the twenty-fifth day of July, one thousand nine hundred and four: and also that the lands mentioned in the said behalf. The said behalf is the said behalf of the said behalf. The said section of the said behalf is the said behalf. It is the said behalf is the said behalf is the said behalf. The said behalf is the said behalf is the said behalf is the said behalf. The said behalf is the said behalf is the said behalf is the said behalf. The said behalf is the said behalf is the said behalf is the said behalf. The said behalf is the said behalf is

SCHEDULE.

OTAGO LAND DISTRICT.

| County. | District. | Section. | | | Cash Price. | | | Occupat Right of Rent, 5 p | ion with Purchase: per Cent. | Lease i= Perpetuity: Rent. 4 per Cont. | | | |
|---------|-----------|----------|--------|-------|-------------|---------------|-------|----------------------------------|------------------------------------|---|--|--|--|
| | | | Block. | Area. | Per A | cre. Total Pr | rice. | Rent per Acre per Aunum. | Half-yearly Rent. | Rent ver Acre ver Aunum, Rent. | | | |

First-class Land.

 A. R. P. E s. d. £ s. d. s. d. £ s. č. s. d. £ s. d.

 Maniototo | Upper Taieri | 53 | I. | 391 1 0 | 1 0 10 391 0 0 | 1 0 | 9 15 5 0 9 6 1 7 16 5

 Open, level, agricultural land, with soil of good quality: not well watered, but there is access to the Sowburn Stream.

 Maniototo + Upper Tuieri | 38 + VII + 249 2 14 + 1 2 6 1281 5 0 + 1 15 + 7 0 5 0 10 8 + 5 12 6

 Maniototo
 Upper Taieri
 38
 VII.
 249
 2
 14
 1
 2
 6
 281
 5
 0
 1
 15
 7
 0
 5
 0
 10.8
 5
 12
 6

 "
 "
 "
 218
 3
 0
 1
 5
 0
 1
 3
 6
 16
 11
 1
 0
 5
 9
 6

 Open, level, agricultural land, with soil of good quality; suitable for orchards, if water obtainable.
 Distant about one mile and a half from Patearoa Township.
 Aiutude, 1,300 ft.

Second-class Land.

| Maniototo | Upper Taieri | | $\frac{48}{12}$ | I. (II. (| 554 | 1 24 | 10 | 0 | 554 | 0 | 0 | 1 | 0 | 13 17 | Û | 0 | 9.6 | 11 | 1 | 7 |
|-----------|--------------|----|-----------------|---------------|-----|------|------|---|-----|---|---|---|-----|-------|----|---|-----|------|----|---|
| " | | 11 | $\frac{49}{11}$ | | 684 | 1 30 | 1 0 | 0 | 684 | 0 | 0 | 1 | 0 | 17 2 | 0, | 0 | 9.6 | 13 1 | 13 | 7 |
| | • | 1 | 50 | I. ', | 354 | 1 30 | 0 12 | 6 | 221 | 5 | 0 | 0 | 7.5 | 5 10 | 8 | 0 | 6 | 4 | 8 | 6 |

Open, level, pastoral land, soil gravelly; though not well watered, Section 50 has access to the Sowburn Stream, and the other sections have access to this stream as well as to the Taieri River. Distant about three miles from Patearoa Township, by good road. Altitude, 1,300 ft.

Maniototo | Upper Taieri | 51 | I. | 382 0 32 0 15 0 |286 10 0 | 0 9 | 7 3 3 | 0 7 2 5 14 7 Open, level, agricultural land, coil of fair quality : intersected by a water-race, and has access to the Sowburn Stream. Distant about two miles from Patearoa Township, by good road. Altitude, 1,300 ft.

Maniototo | Upper Taieri | 52 | I. | 388 2 24 0 17 6 | 340 7 6 | 0 10.5 | 8 10 2 | 0 8.4 | 6 16 2 Open, level, agricultural land, with soil of good quality; not well watered, but there is access to the Sowburn Stream.

Maniototo | Upper Taieri | 24 | VI. | 523 2 26 0 16 8 [436 13 4] 0 10 | 10 18 4 | 0 8 | 8 14 8 Open, level and undulating, pastoral land, soil of fair quality in parts; no water. Access by good road; distant about two miles from Patearoa Township. Altitude, 1,300 ft.

. o

2 -

As witness the hand of His Excellency the Governor, this sixth day of June, one thouzand nine hundred and four.

J. CARROLL, For Minister of Lands.

Lands permanently reserved.

RANFURLY, Governor.

WHEREAS by the two-hundred and thirty fifth section of "The Land Act, 1892," it is enacted that the Governor may from time to time, either by general or particular description, and whether the same has been surveyed or not, reserve from sale temporarily, notwithstanding that the same may be then held under pastoral hoense, any Crowig lands which in his opinion are required for any of the purposes in the said section mentioned:

And whereas by the two-hundred and-thirty sixth section of the said Act it is provided that land temporarily reserved under the said two-hundred and thirty fifth section may, at the expiration of one month but not later than six months after the publication in the *Gazette* of notice of such temporary reservation, be permanently reserved, and that notice of such permanent reservation shall be published in the *Gazette*:

And whereas the lands specified in the first column of the Schedule hereto were, by the Warrants the dates of which are specified in the third column of the said Schedule, and the notifications of which were published in the Gazettes specified in the fourth column, temporarily reserved under the authority of the said Act for the purposes specified in the second column of the said Schedule:

Now, therefore, I. Uchter, John Mark, Ear! of Ranfurly, the Governor of the Colony of New Zeadaad. in pursuance and exercise of the power and authority vested in me by the said Act, do hereby permanently reserve the lands so temporarily reserved as aforesaid, and enumerated in the first column of the Schedule hereto, for the purposes specified in the second column of the said Schedule, being the same purposes for which the said lands were so temporarily reserved as aforesaid.
*. 14*96

.

0 L 17 TO TO TO

| 1496 | | THE NEW | V ZE | ALA | ND | GAZETTE. | | [No. 48 | • |
|--|--|--|------------------------------|--|---|--|--|---|--------------------|
| - | - | | SCHI | EDULE | 2. '. | | | - | |
| | Fir. Descoert | st Column. on of Reserves. | | | | Second Column. | Third Column. | Fourth Column. | |
| toniii District. | ljon h 1163 s | Bection, | Islock. | | c'ik | Purpose for which Laud reserved, | Date of Nurrant. | . Gazette. | |
| Auckland | Orahiri S.D. | | - <u>x</u> x- | A. 3. | в. р. 015 | Railway | 1904. 24 Feb. | 1904. No. 19, 3 Mar. | j.252h |
| , , | Town of Paeroa | | ξ: xxn | $ \begin{bmatrix} 5 \\ 21 \\ 0 \end{bmatrix} $ | $\frac{10}{10}$ | Public-school site | | No. 29, 7 Apr. | 8.0234 1 8.4984 |
| " | Mangonui Eas | 1 275A 1 218A | | $\frac{121}{-13}$ | $ \frac{0}{2} \frac{0}{10} $ | Recreation Trublic-school-site, | ,, ,, | | 5.48851 |
| , | oeka Parish) Tokatoka S.D.*) | 375 | -XI. | 8 | 09 3 <u>10</u> | Night-soil depot | ,, ,, ,, ,, ,, ,, ,, ,, ,, ,, ,, ,, ,, | | 30-275 |
| Taranaki | Maramarua Parish Town of New Ply | | c = | -0= 0 | 2=0 0 4 | +Public-hall site = Cemetery | 7 April 26 Mar. | No. 30, 14 Apr. No. 29, 7 Apr. | 2093.m |
| Wellington | mouth Rimutaka S.D.* . Tiffin S.D.* . | 129 | I. I II., | 0 1285 | $\begin{array}{ccc} 1 & 24 \\ 0 & 0 \end{array}$ | Railway | | | |
| n - | Koitiata S.D.* | 192 191 | V. I. I. | 9 85 | $ \begin{array}{ccc} 0 & 0 \\ 2 & 0 \end{array} $ | Travelling stock Recreation | 11 April | No. 30, 14 Apr. | |
| Westland Canterbury | Kanieri S.D.* Pukaki S.D.* | Res. 356 Res. 3701 (for- | | 5 176 | $ \begin{array}{c} 0 & 19 \\ 0 & 0 \end{array} $ | Recreation | 1903. 9 Dec. | 1903. No. 95, 17 Dec. | |
| f, | Pukaki West S.D.* | Res. No. 182) Res. 3702 (for- |) XV.) | 1 | 0 6 | 1 D | 1904. 15 April | 1904. No. 33, 21 Apr. | |
| " ([.] Otago | Strachey S.D.* Town of Hawksbury | (merly part of Res. No. 183) 14 |) 111.) X111. | ; 1 | 00 | Recreation) Public-school site | 26 Mar. | No. 29, 7 Apr. | |
| Southland | Winton Hundred | 1, 2, 48, 49, 50 76 77 | | | $ 1 14 \\ 3 2 \\ 3 14 $ | Rifle range Rifle range | " | 10000 1 10000 | |
| • | | and of Hig Browl | * Survey | / Distric | t. | | r | · · · · · · · · · · · · · · · · · · · | |
| | hundred and fo | ur. | | | erno | T. Y | June, oue DUNCA: Ministe | n, n, N, Chands. | |
| Arrangements | for First Elections, | | ounty. | / - лого 1 Арроі | ntmer | t of Vice-Consul for th | he Netherla | uds. at Dunc di n. | |
| - | Colonial Se Well | cretary's Office, ington. 2nd June. 1 | 904. | | | Colonial Secre | al. staru's Offic | | p |
| HIS Exc. appoin | ellency the Governo at FREDERICK W | or has been pleas Vest | sed to | FT | IS E | xcellency the Governe | ellington, 60 or directs i | b June, 1904. t to be notified | |
| to be the person Aorere and Partituted by W | on to make up clector tkawau, in the Count The Counties Act. 1 | 's' rolls for the Rid y of Collingwood, a 886.2' and a The C | ings of is con- | cipal appoir | Secr | etary of State for the ut of Cronce Reserve | Colonies t | o recognise the | |
| wood County conduct the fir said county, a | Act. 1903"; also to rst elections of memb and to be the Clerk of | be Returning Off bers of the Council of the Council of the | icer to of the ie said | as Vic | :e-Co | nsul for the Netherland | , Issquire, ds, at Dunc J | din. . G. WARD. | |
| county for the hereof. | e purpose of presid | ing at the first m J. G. WA | neeting RD. | . | | Commissioner of Crown | Lands ann | ninted | |
| Rangers unde Whangaroa | er the Animals Pro | tection Acts, Mar te Districts, appoin | ngonui- ted. | | | Department of | Lands and | Survey, | |
| - | Colonial Secr Well | etary's Office, ington, 2nd June, 1 | 904. | Η, | S E app | xcellency the Govern | fington, 2n | en pleased to | |
| HIS Exce appoint ander "The A | llency the Governo t the undermentione Animals Protection | r has been pleas d persons to be Ra Act, 1880,'' and the | ed to augers | to be of A | Comi ickla | JAMES MACK missioner of Crown La nd. as from the 1st | enzie ands for the dav of J | Eand District une. 1904. vice | |
| mending the names respect Nam | a same, for the dist ively, viz. :— ne. | pricts set opposite District. | their | Gerha | rd Jo | bhn Mueller. | C. H For Minis | . MILLS, tter of Lands. | |
| TEPANA MAT Samuel Joh | THEWS IN ADAMS | Mangonui-Whanga Waitaki-Waimate. J. G. WA | roa.' RD. | | | | | | |
| Deputy | Registrar of Marriag | es, &c., appointed. | | | | Department of | Lands and | Survey, | |
| | Colonial Secre Well | atary's Office, ington, 6th June, 1 | 904. | H | S E: appo | We cellency the Governo int | llington, 2n or has bee | d June, 1904. en pleased to | |
| H appoint | niency the Governo t Eagle D'Arcy H | r nas been picas AMILTON | eu in | to be (| Çhief | JAMES MACRI Surveyor for the Lan | enzie d District o | f Auckland, as | |
| o he the Dej Births and De | puty of the Registra aths for the District | of Marriages a of Kawhia. J. G. WAI | nd of RD. | froni t | be la | at day of June, 1904, r | ice Gerhard C. H. For Minig | U John Mueller. MILLS, ter of Landa | |
| • | | | | | | · 、 | 141018 | | , |

· •

B.251112.2 Gazette Notice (NZ Gazette 17 November 1983 No 192 page 4000) declaring part (275 m² and 184 m²) adjoining State Highway No.10 to be stopped road and adding the land to Allotment 18A Parish of Mangonui to be held as a reserve for a public school site- 6.1.1984 at 10.15 o/c

B.251112.3 Gazette Notice (N/ Gazette 17 November 1983 No 192 page 3999) declaring part (409 m² and 979 m²) to be set apart for road- 6.1.1984 at 10.15 o'c

A.L.R.

'n





Parcel: Stopped Road Survey Office Plan 51928

Summary

| Appellation | Stopped Road Survey Office Plan 51928 | | | |
|-------------------------|---------------------------------------|-------------|--|--|
| Parcel Status | Current | | | |
| Intent | DCDB | | | |
| Associated Feature | - | | | |
| Land District | North Auckland | | | |
| Non Surveyed Definition | - | | | |
| Parcel ID | 4907253 | | | |
| Parcel Area | 0.0275 Ha (275 m²) | | | |
| Total Area | - | | | |
| Survey Number | Land District | Survey Date | | |
| <u>SO 51928</u> | North Auckland | - | | |
| Statutory Actions | | | | |
| Statutory Action | New Zealand Gazette 198 | 33 p 4000 | | |
| Туре | Gazette Notice | | | |
| Recorded | 4 April 2002 | | | |
| Action | Create | | | |
| Status | Current | | | |
| Statute | - | | | |
| Purpose | School Reserve | | | |
| Name | | | | |
| | - | | | |

4000

THE NEW ZEALAND GAZETTE

16/1

Dated at Wellington this 10th day of November 1983. J. R. BATTERSBY.

for Minister of Works and Development.

(P.W. 72/10/1/0; Ak. D.O. 50/23/102/0)

Declaring Land Held for Defence Purposes to be Crown Land in Block III, Whangaroa Survey District, Whangaroa County

PURSUANT to section 42 of the Public Works Act 1981, the Minister of Works and Development declares the land described in the Schedule hereto to be Crown land, subject to the Land Act 1948.

SCHEDULE

NORTH AUCKLAND LAND DISTRICT

ALL that piece of land containing 6 acres, 2 roods, and 31 perches (2.7089 hectares), situated in Block III, Whangaroa Survey District, and being part Matakaraka B Block; as shown coloured yellow on S.O. Plan 33991 (P.W.D. 123691), deposited in the office of the Minister of Works and Development at Wellington. Part Proclamation No. 11879.

Dated at Wellington this 10th day of November 1983.

J. R. BATTERSBY,

for Minister of Works and Development.

(P.W. 23/646/1; Ak. D.O. 50/8/5)

P.

A. R.

A =00

16/1

Land Held for Buildings of the General Government Set Apart for Agricultural Purposes in the City of Wellington

PURSUANT to section 52 of the Public Works Act 1981, the Minister of Works and Development hereby declares the land described in the Schedule hereto to be set apart for agricultural purposes and shall remain vested in the Crown.

SCHEDULE

WELLINGTON LAND DISTRICT

ALL those pieces of land situated in the City of Wellington, described as follows:

Being

0 0 10.22 Subdivision 16A, Pipitea Pa. Part Proclamation 3472, (258 m²) Wellington Land Registry; excepting that portion of 0.34 perches, contained in certificate of title 16A/1459.

0 0 13.51 Subdivision 17, 18, and part 28, Pipitea Pa. Part (341.5 m²) Proclamation 3472, Wellington Land Registry.

Dated at Wellington this 10th day of November 1983.

J. R. BATTERSBY,

for Minister of Works and Development.

16/1

Declaring Crown Land to be Set Apart for State Housing Purposes in the City of New Plymouth

PURSUANT to section 52 of the Public Works Act 1981, the Minister of Works and Development declares the land described in the Schedule hereto to be set apart for State housing purposes.

SCHEDULE

TARANAKI LAND DISTRICT

ALL those pieces of land situated in the City of New Plymouth, described as follows:

| m ² | Being | | | | | |
|----------------|---------------------|--|--|--|--|--|
| 610 | Lot 18, D.P. 10428. | | | | | |
| 539 | Lot 19, D.P. 10428. | | | | | |
| 559 | Lot 20, D.P. 10428. | | | | | |
| 675 | Lot 21, D.P. 10428. | | | | | |

Parts certificate of title, No. C3/143.

(P.W. 24/1729; Wn. D.O. 45/42/19)

Dated at Wellington this 8th day of November 1983. J. R. BATTERSBY, for Minister of Works and Development.

(P.W. 104/114/0; Wg. D.O. 5/64/0/7)

16/1

16/1

Declaring Crown Land to be Set Apart for State Housing Purposes in the City of New Plymouth

PURSUANT to section 52 of the Public Works Act 1981, the Minister of Works and Development declares the land described in the Schedule hereto to be set apart for State housing purposes.

SCHEDULE

TARANAKI LAND DISTRICT

ALL that piece of land containing 1672 square metres, situated in the City of New Plymouth, being Lot 141, D.P. 9889. Part Gazette notice 269169 (New Zealand Gazette, 1980, page 1865).

Dated at Wellington this 8th day of November 1983.

J. R. BATTERSBY,

for Minister of Works and Development.

(P.W. 104/114/0; Wg. D.O. 5/64/0/7)

Declaring Land Held for State Housing Purposes to be Crown Land in the City of New Plymouth

PURSUANT to section 42 of the Public Works Act 1981, the Minister of Works and Development declares the land described in the Schedule hereto, subject to the building line restriction contained in memorandum of acceptance 229063.1, to be Crown land.

SCHEDULE

TARANAKI LAND DISTRICT

ALL those pieces of land situated in Blocks VIII and IX, Paritutu Survey District, described as follows: Area

•

ha

Being Lot 1, D.P. 10490. All certificate of title No. E1/1018.

8.2708 Lot 1, D.P. 10490. All certificate of title No. E1/1018. 8.2733 Lot 2, D.P. 10490. All certificate of title No. E1/1019.

Both being part Maori Reserve 2 (Ararepe), part Sections 67 and 832, Grey District.

Dated at Wellington this 8th day of November 1983.

J. R. BATTERSBY,

for Minister of Works and Development.

(P.W. 104/114/0; Wg. D.O. 5/64/0/7)

16/1

Amending a Notice Declaring a Leasehold Estate in Land Held for Police Purposes (Residence) to be Set Apart for the Transmission of Electricity (Housing) in the Borough of Stratford

PURSUANT to section 55 of the Public Works Act 1981, the Minister of Works and Development hereby amends the notice dated 6 September 1983, and published in *Gazette*, 15 September 1983, No. 150, at page 3067, declaring a leasehold estate in land held for police purposes (residence) to be set apart for the transmission of electricity (housing) in the Borough of Stratford, by deleting from line 5 of the body of the said notice the figure "18001" and substituting the figure "193949" therefor.

Dated at Wellington this 8th day of November 1983.

J. R. BATTERSBY, for Minister of Works and Development.

(P.W. 92/16/35/6; Wg. D.O. 38/8/0)

18/1

Road Stopped in Block VI, Mangonui Survey District, Mangonui County, and Added to Land Held as a Reserve for a Public School Site

PURSUANT to sections 116 and 117 (7) of the Public Works Act 1981, the Minister of Works and Development hereby declares that the road described in the First Schedule hereto is hereby stopped and added to the land held as a reserve for a public school site in the Second Schedule hereto.

FIRST SCHEDULE

NORTH AUCKLAND LAND DISTRICT

ALL those pieces of road situated in Block VI, Mangonui Survey District, described as follows:

17 NOVEMBER

THE NEW ZEALAND GAZETTE

Area

m²

Area

m²

Adjoining or passing through Part Allotment 18A, Mangonui East Parish; marked "D" 275

on plan. Part Allotment 18A, Mangonui East Parish; marked "H" 184

on plan.

As shown marked as above mentioned on S.O. Plan 51928, lodged in the office of the Chief Surveyor at Auckland.

SECOND SCHEDULE

NORTH AUCKLAND LAND DISTRICT

ALL that piece of land containing 13 acres, 2 roods, and 10 perches, situated in Block VI, Mangonui Survey District and being Allotment 18A, Mangonui East Parish.

Dated at Wellington this 10th day of November 1983.

J. R. BATTERSBY, for Minister of Works and Development.

(P.W. 72/10/1/0; Ak. D.O. 50/23/102/0)

16/1

Amending a Notice Declaring Land to be Road and Road Stopped in Blocks I and V, Otanewainuku Survey District, Tauranga County

PURSUANT to section 55 of the Public Works Act 1981, the Minister of Works and Development, hereby amends the notice dated the 15th day of June 1983, published in *Gazette*, 23 June 1983, No. 88, page 1927, declaring land to be road and road stopped in Blocks I and V, Otanewainuku Survey District, Tauranga County, pursuant to Part VIII of the Public Works Act 1981, by omitting paragraph (b) (ii) thereto and substituting the following:

(ii) The area marked "B" on plan, now known as Allotment 330, Parish of Te Papa, shall vest in Henry Gerard Van Den Broek, of Papamoa, carpenter and Moira Jean Van Den Broek, of Papamoa, bank officer, subject to memoranda of mortgage H.467524.4 and H.467524.5, South Auckland Land Registry.

Dated at Wellington this 10th day of November 1983.

J. R. BATTERSBY,

for Minister of Works and Development. (P.W. 53/52/0; Hn. D.O. 24/0/212)

16/1

Land Declared to be Road in the Borough of Huntly

PURSUANT to Part VIII of the Public Works Act 1981, the Minister of Works and Development declares the land described in the Schedule hereto to be road and vested in The Huntly Borough Council.

SCHEDULE

SOUTH AUCKLAND LAND DISTRICT

ALL those pieces of land situated in the Borough of Huntly, described as follows:

Area m^2

Being

| *** | Domb |
|-------|---|
| 3681 | Lot 30, D.P. S. 25715. Part certificate of title No. 24A/239. |
| 2972 | Lot 268, D.P. S. 27897. All certificate of title No. 26A/358. |
| 6843 | Lot 269, D.P. S. 27896. All certificate of title No. 26A/351. |
| 1591 | Lot 50, D.P. S. 27898. All certificate of title No. 26A/374. |
| 7025 | Lot 118, D.P. S. 27895. All certificate of title No. 26A/334. |
| 2225 | Lot 270, D.P. S. 27894. All certificate of title No. 26A/316. |
| 2151 | Lot 271, D.P. S. 27893. All certificate of title No. 26A/299. |
| 2706 | Lot 272, D.P. S. 27893. All certificate of title No. 26A/300. |
| 1699 | Lot 278, D.P. S. 27892. All certificate of title No. 26A/279. |
| 6262 | Lot 277, D.P. S. 26175, All certificate of title No. 24B/799. |
| 702 | Lot 283, D.P. S. 26234. All certificate of title No. 24B/703. |
| 1355 | Lot 284, D.P. S. 26234, All certificate of title No. 24B/704. |
| 752 | Lot 285, D.P. S. 26234. All certificate of title No. 24B/705. |
| 1900 | Lot 18. D.P. S. 25714. Part certificate of title No. 24A/239. |
| 3733 | Lot 239, D.P. S. 25265, All certificate of title No. 25C/645. |
| 2253 | Lot 371, D.P. S. 26581. Part certificate of title |
| | No. 25A/1334. |
| 2105 | Lot 370, D.P. S. 26580. Part certificate of title |
| | No. 25A/1334. |
| 2201 | Lot 369, D.P. S. 26578, Part certificate of title |
| | No 25A/1307 |
| 3021 | Lot 368 D.P. S. 26577 Part certificate of title |
| | No 25A/1307 |
| 2345 | Lot 367 D.P. S. 26576 Part certificate of title |
| 23 43 | $N_0 25A/1318$ |
| | 110. 201 1 1010. |

Being

Lot 333, D.P. S. 25986. Part certificate of title No. 25C/649. Lot 85, D.P. S. 25984. Part certificate of title No. 25C/649. Lot 148, D.P. S. 25986. Part certificate of title No. 25C/649. Lot 143, D.P. S. 27367. All certificate of title No. 25A/1377. Lot 110, D.P. S. 27368. All certificate of title No. 25A/1378. Lot 43, D.P. S. 25278. Part certificate of title No. 25A/1378. Lot 43, D.P. S. 25281. Part certificate of title No. 25A/1281. 5201 1429 828 4873 3930 1128 1166

Dated at Wellington this 10th day of November 1983.

J. R. BATTERSBY, for Minister of Works and Development.

(P.W. 51/4892; Hn. D.O. 43/13/0)

Declaring Land to be Road in the Borough of Taupo

PURSUANT to section 114 of the Public Works Act 1981, the Minister of Works and Development declares the land described in the Schedule hereto to be road, which shall vest in The Taupo Borough Council

SCHEDULE

SOUTH AUCKLAND LAND DISTRICT

ALL that piece of land containing 463 square metres, situated in Block XIII, Tatua Survey District, being part Rangatira No. 2 Block; as shown marked "A" on S.O. Plan 51479, lodged in the office of the Chief Surveyor at Hamilton.

Dated at Wellington this 10th day of November 1983.

J. R. BATTERSBY,

for Minister of Works and Development.

(P.W. 35/817; Hn. D.O. 43/20/0/41)

16/1

Declaring Land to be Road in Block IX, Mangaorongo Survey District, Otorohanga District

PURSUANT to section 114 of the Public Works Act 1981, the Minister of Works and Development declares the land described in the Schedule hereto to be road, which shall vest in The Otorohanga District Council.

SCHEDULE

SOUTH AUCKLAND LAND DISTRICT

ALL that piece of land containing 2928 square metres, situated in Block IX, Mangaorongo Survey District, being part Lot 1, D.P. 25337; as shown marked "A" on S.O. Plan 51917, lodged in the office of the Chief Surveyor at Hamilton.

Dated at Wellington this 10th day of November 1983.

J. R. BATTERSBY, for Minister of Works and Development.

(P.W. 34/4593; Hn. D.O. 98/6/0/4)

16/1

Leasehold Estate Acquired for the Purposes of a Road in Westland County

PURSUANT to section 20 of the Public Works Act 1981, the Minister of Works and Development declares that, an agreement to that effect having been entered into, the leasehold estate in the land described in the Schedule hereto held by Jeffrey David Sweney, of Lake Kaniere, contractor, and Eileen Denise Sweney, his wife, under and by virtue of renewable lease farm land No. 374, recorded in Volume 54, follo 27, Westland L and Basistry, is basely negative of the 5A, folio 27, Westland Land Registry, is hereby acquired for the purposes of a road on the 17th day of November 1983.

SCHEDULE

WESTLAND LAND DISTRICT

ALL that piece of land containing 2383 square metres, situated in Block XII, Waitaha Survey District, being part Rural Section 5646; marked 'A' on S.O. Plan 9368, lodged in the office of the Chief Surveyor at Hokitika.

16/1





AGENT AUTHORISATION FORM

We, Oruaiti School Board *(the client),* legal property owner of 2554 State Highway 10, Mangonui 0494 give Hamish Gibson of LDE Ltd authorisation to act as the agent for our proposed applications with the Far North District Council.

We, the client will be responsible for the invoice payment for the consents and the council should contact Hamish as the preferred means of communication.

Signatories full name: Janene Quirk

Signatories role within school: Ministry of Education Property Adviser

Signature: JOUNK

Dated: 03/04/2025



Project Number:

Project Office: Project Manager: Issue Date:

Drawing Status: Resource Consent

Proposed Carpark Upgrades Oruaiti School Board of Trustees c/o Avail Pacific 2554 State Highway 10, Mangonui

| Drawing Index | | | | | | | | |
|---------------|--|------------------|----------|--|--|--|--|--|
| Sheet | Description | Status | Revision | | | | | |
| C001 | Existing Site Plan | Resource Consent | С | | | | | |
| C002 | Demolition and Reinstatement Plan | Resource Consent | В | | | | | |
| C100 | Construction Staging Layout | Resource Consent | В | | | | | |
| C200 | Earthworks Layout | Resource Consent | С | | | | | |
| C300 | Overall Site Layout | Resource Consent | D | | | | | |
| C301 | Parking Area & Accessway Design Plan Sheet 1 of 3 | Resource Consent | D | | | | | |
| C302 | Parking Area & Accessway Design Plan Sheet 2 of 3 | Resource Consent | D | | | | | |
| C303 | Parking Area & Accessway Design Plan Sheet 3 of 3 | Resource Consent | D | | | | | |
| C304 | Signage Layout | Resource Consent | А | | | | | |
| C305 | Concrete Saw-cuts Layout | Resource Consent | В | | | | | |
| C310 | Parking Area & Accessway Longitudinal Sections | Resource Consent | С | | | | | |
| C311 | Parking Area & Accessway Typical Cross Sections | Resource Consent | С | | | | | |
| C312 | Parking Area & Accessway 5m Interval Cross Sections Sheet 1 of 3 | Resource Consent | С | | | | | |
| C313 | Parking Area & Accessway 5m Interval Cross Sections Sheet 2 of 3 | Resource Consent | С | | | | | |
| C314 | Parking Area & Accessway 5m Interval Cross Sections Sheet 3 of 3 | Resource Consent | С | | | | | |
| C320 | Parking Area & Accessway Typical Details Sheet 1 of 2 | Resource Consent | D | | | | | |
| C321 | Parking Area & Accessway Typical Details Sheet 2 of 2 | Resource Consent | В | | | | | |
| C400 | Parking Area & Accessway Stormwater Management Plan | Resource Consent | С | | | | | |
| C410 | Parking Area & Accessway Stormwater Longitudinal Sections Sheet 1 of 2 | Resource Consent | С | | | | | |
| C411 | Parking Area & Accessway Stormwater Longitudinal Sections Sheet 2 of 2 | Resource Consent | С | | | | | |
| C420 | Parking Area & Accessway Typical Stormwater Device Details | Resource Consent | С | | | | | |
| C421 | Parking Area & Accessway Typical Stormwater Details | Resource Consent | С | | | | | |



21102

Warkworth Riaan de Swardt 1 April 2025



General Notes:

Library

Pool

- 1. All dimensions in metres unless noted otherwise.
- Coordinates are in terms of NZGD Mt Eden 2000. 2.
- All levels are in metres to NZVD2016. Origin of levels 3. RCM XIII SO 50961 RL 11.81m.
- 4. Survey undertaken by Donaldsons; 25 July 2022.
- Existing titles Pt Allot 18A PSH OF Mangonui East 5
- Boundaries and are approximate only and are subject 6. to change. Final boundaries will be in accordance with city plan and LINZ requirements. Boundaries not for building design purposes.
- Existing building, roading, and services information may have been obtained from third party records and should be verified on site prior to the commencement of any works. LDE does not in any way guarantee the accuracy of any underground service shown on this plan.
- Contractor to locate / mark / confirm and protect if 9. necessary all existing services prior to commencement of work on site. Any discrepancies to be reported to the Engineer.
- 10. Flood extent extracted from NRC Natural Hazards and shown indicatively for information only.

General Legend:

— — Boundary (Surveyed) Major Contour Minor Contour

NRC River Flood Hazard Zone - Regionwide Models

| - | _ | | |
|-------|----------|---|--|
| | <u> </u> | | |
| _ | _ | _ | |
| | | | |

10 Year Exent 50 Year Extent 100 Year CC Extent

Existing Services Legend:

| SW SW |
|-------|
| 8 E |
| S ⊂ |
| |
| |
| P P P |
| онон |
| T |
| F0 F0 |
| |
| |

Edge of Road Asphalt Surface Stormwater Pipe Stormwater Manhole, Wingwall, Scruffy Dome, Catchpit Open Drain Water Pipe Wastewater Pipe Underground Power Lines Power Pole & Over Head Lines Transformer Fibre Optic Lines Top of Bank Bottom of Bank Fence / Gate Retaining Wall

| 21 | | | | | 10 | 15 | 20 | 23 |
|----------|-----------------------|----------------------------------|-----------|--------------|-----------------|--------------|--------------|-----------|
| | | | | Scale 1:500 | (m) | | Original siz | e = A3 |
| | | | Design: | R. de Swardt | Project status: | ource | Conse | nt |
| | | | Drawn: | R. de Swardt | Project: | <u>ouroo</u> | | |
| | N. D | 28 Mar 2025 | Approved: | N. Douglas | Drawing No: | 211 | 102 | Issue/Rev |
| Signage) | N. D J. S Aprvd | 5 Sep 2024 1 May 2024 Date | Scale A3: | 1:500 | | C001 | 1 | С |

- This plan should be read in conjunction with the proposed plans included in the Detailed Design drawing set.
- proposed plans for details.
- Engineering Standards and the Health and Safety at Work Act 2015 (HSWA).
- protect if necessary all existing services prior to commencement of work on site.
- All disturbed public areas to be made good and restored to existing condition.
- All existing vegetation in the proposed works area to be removed and re-used.
- reinstatement works. Any excess topsoil to be used elsewhere on site as directed by the School's representatives.





Staging Notes:

57

- Contractor's Site Office shown indicatively. Exact area to be confirmed prior to construction.
 The contractor is fully responsible for Health and Safety measures during the construction phase.
 - Appropriate fencing, signage, etc. to be installed prior top the start of works. Contractor is to allow for all safety fencing for the
- during of the proposed works. The Staging Areas is to be confirmed prior to be commencement of construction.
- Contractor to obtain and maintain all traffic management requirements as per NZTA standards.

| | | | | | 10 | 15 | 20 | 25 | |
|------------|--------------|------------------------------------|-----------|---------|--------|-----------------|-------|--------------|-----------|
| | | | | Scale 1 | :500 | (m) | | Original siz | e = A3 |
| | | | Design: | R. de S | Swardt | Project status: | ource | Conse | nt |
| | | | Drawn: | R. de S | Swardt | Project | | | /// |
| | | | Approved: | N. Dou | uglas | Drawing No: | 211 | 02 | lesua/Rav |
| ce Consent | N. D N. D | 01 Apr 2025 28 Mar 2025 Date | Scale A3: | 1:50 | 00 | Drawing No. | C100 |) | B |
| 101011 | 740110 | Buto | | | | | | | |

engineers • scientists



General Notes:

- All dimensions in metres unless noted otherwise.
- Coordinates are in terms of NZGD Mt Eden 2000. 2.
- All levels are in metres to NZVD2016. Origin of levels 3. RCM XIII SO 50961 RL 11.81m.
- Survey undertaken by Donaldsons; 25 July 2022. 4.
- Existing titles Pt Allot 18A PSH OF Mangonui East 5
- Boundaries and are approximate only and are subject 6. to change. Final boundaries will be in accordance with city plan and LINZ requirements. Boundaries not for building design purposes.
- Existing building, roading, and services information may have been obtained from third party records and 8. should be verified on site prior to the commencement of any works. LDE does not in any way guarantee the accuracy of any underground service shown on this plan.
- Contractor to locate / mark / confirm and protect if 9. necessary all existing services prior to commencement of work on site. Any discrepancies to be reported to the Engineer.
- 10. Flood extent extracted from NRC Natural Hazards and shown indicatively for information only.

General Legend: Earthworks Notes:

- All works to be undertaken in accordance with FNDC Engineering Standards, GD05 Guidelines and the Health and Safety at Work Act 2015 (HSWA).
- All disturbed public areas to be made good and restored to previous condition.
- All earthworks to be undertaken during periods of fine weather.
- Earthworks are to be undertaken and completed in within the minimum possible time frame.
- The area is to be progressively finished as areas are brought to grade.
- The earthworks volumes shown is a solid measure.
- The earthworks volumes has been calculated in relation to the existing and final ground levels.
- Topsoil removal volumes is excluded in the total earthworks volumes.
- Additional box cut volumes has not been included in the earthworks volumes.

Erosion and Sediment Control Notes:

- The contractor is solely responsible for ensuring the the site has effective erosion control and sediment retention measures operating at all times.
- All erosion and sediment control measures must be operational prior to any other works commencing on site
- All sediment control measure are to comply with GD05 and FNDC requirements.

| Earthworks Cut / Fill Volumes | | | | | | | |
|-------------------------------|----------------------|--|--|--|--|--|--|
| Site Area | 41,210m ² | | | | | | |
| Earthworks Area | 2,760m² | | | | | | |
| Cut Volume | 20m³ | | | | | | |
| Fill Volume | 460m ³ | | | | | | |
| Net Fill Volume | 440m ³ | | | | | | |

| | | | | 0 5 | 10 | 15 | 20 | 25 |
|--------------|--------------|--------------------------|-----------|--------------|------------------------|-------|--------------|-----------|
| | | | | Scale 1:500 | (m) | | Original siz | e = A3 |
| | | | Design: | R. de Swardt | Project status: Res | ource | Conse | ent |
| | | | Drawn: | R. de Swardt | Project: | 044 | 00 | |
| | N. D | 28 Mar 2025 | Approved: | N. Douglas | Drawing No: | 211 | 02 | Issue/Rev |
| a + Signage) | N. D H. D | 5 Sep 2024 1 May 2024 | Scale A3: | 1:500 | | C200 | | С |



Proposed Services Notes:

- All works to be undertaken in accordance with FNDC Engineering Standards and the Health and Safety at Work Act 2015 (HSWA).
- 2. All dimensions in metres unless noted otherwise.
- Contractor to locate / ml dimensionark / confirm and protect if necessary all existing services prior to commencement of work on site.
- . All disturbed public areas to be made good and restored to existing condition.
- Manholes to be reinforced concrete chambers with a precast base. Min Ø1050mm manhole unless otherwise specified.
- 6. Catchpits to be 675 x 450mm reinforced concrete chambers, with trafficable frame & grid inlet lid.
- All PVC stormwater pipes to be uPVC SN16.
- 3. All concrete stormwater pipes to be RCRRJ Class 4.
- 9. Pipe bedding and installation to comply with relevant council details.
- All lane and parking markings as per MOTSAM.
 Parking bays as per Appendix 3D of the Far North
- District Plan (FNDP).
- 12. All kerbing to be machine extruded, unless specified otherwise by Engineer.

Reference Notes:

- 13. Refer to C301,C302, C303 for Parking Area Layouts.
- 14. Refer to C310 for Parking Area and Accessway Longitudinal Sections Layout.
- 15. Refer to C311 to C314 for Cross Section Layouts.
- 16. Refer to C320 & C321 for Roading details.
- 17. Refer to C400 for Stormwater Layout.
- 18. Refer to C410 to C411 Stormwater Longitudinal Section Layouts.
- 19. Refer to C420 & C421 for Stormwater details.

Existing Services Legend:



| | | | Design: | R. de Swardt | Project status: Resource Consent |
|---------------|--------------|--------------------------|-----------|--------------|-------------------------------------|
| | | | Drawn: | R. de Swardt | Project: |
| ce Consent | N. D | 01 Apr 2025 | Approved: | N. Douglas | 21102 |
| ea + Signage) | N. D H. D | 5 Sep 2024 1 May 2024 | Scale A3: | 1:500 | C300 Drawing No: |
| /ision | Aprvd | Date | | | |

20





| Proposed Services Notes: | Existing Services Legend: | Proposed Services Legend: |
|---|--|---|
| Proposed Services Notes: All works to be undertaken in accordance with FNDC Engineering Standards and the Health and Safety at Work Act 2015 (HSWA). All dimensions in metres unless noted otherwise. Contractor to locate / ml dimensionark / confirm and protect if necessary all existing services prior to commencement of work on site. All disturbed public areas to be made good and restored to existing condition. Manholes to be reinforced concrete chambers with a precast base. Min Ø1050mm manhole unless otherwise specified. Catchpits to be 675 x 450mm reinforced concrete chambers, with trafficable frame & grid inlet lid. All lane and parking markings as per MOTSAM. | Existing Services Legend: — — Boundary (Surveyed) — — Edge of Road Sw Stormwater Pipe Image: Stormwater Manhole, Wingwall, Scruffy Dome, Catchpit > >>>> Open Drain Image: Proper Proper Pole & Over Head Lines Image: Proper Proper Pole & Over Head Lines Image: Proper Proper Pole & Over Head Lines Image: Proper Proper Pole Bank Image: Proper Pole Bank | Proposed Services Legend: Prop. Kerb Block Prop. Kerb & Channel Prop. Flush Edge Beam Standard Concrete Surface Exposed Aggregate Concrete Gravel Surface Rock Filled Traffic Island Grassed Area Image: P SW Perforated Pipe & Rodding Point SW Manhole SW Catchpit SW Swale |
| Parking bays as per Appendix 3D of the FNDP. All kerbing to be machine extruded, unless specified otherwise by Engineer. | | →→→→ Fence |

Copyright: LDE All rights reserved / Do not scale off drawings / Confirm all dimensions on site prior to work Client

Oruaiti School Board of Trustees c/o Avail Pacific

2554 State Highway 10, Mangonui Proposed Carpark Upgrades

Project

Drawing Title Parking Area & Accessway Design Plan Sheet 2 of 3



| | | | | 0 2 | 5 | 5 | 7.5 | 10 | 12.5 |
|---------------|------------|----------------------------|-----------|---------|--------|-----------------|------------------------|--------------------------------|-------------------|
| | | | | Scale 1 | :250 | (m) | | Original siz | ze = A3 |
| | | | Design: | R. de S | Swardt | Project status: | ource | - Conse | -nt |
| | | | Drawn: | R. de S | Swardt | Project | | | 2111 |
| ce Consent | N. D | 01 Apr 2025 28 Mar 2025 | Approved: | N. Do | uglas | | 21 | 102 | |
| ea + Signage) | N.D H.D | 5 Sep 2024 1 May 2024 | Scale A3: | 1:2 | 50 | Drawing No: | C30 | 2 | D D |
| lision | Aprva | Date | | | | | Drive Location C., San | ed by RaanDeSwardt, Document E | 455509, Version 3 |

| | | Proposed Services N All works to be FNDC Engineerin Safety at Work Ac All dimensions in Contractor to loc and protect if neo to commencemen All disturbed pub restored to existin Manholes to be re a precast base. otherwise specifie Catchpits to be 6 chambers, with tra All lane and parkii Parking bays as p All kerbing to specified otherwise | otes: undertaken in accordance with g Standards and the Health and t 2015 (HSWA). metres unless noted otherwise. ate / ml dimensionark / confirm tessary all existing services prior t of work on site. lic areas to be made good and g condition. einforced concrete chambers with Min Ø1050mm manhole unless id. 75 x 450mm reinforced concrete afficable frame & grid inlet lid. mg markings as per MOTSAM. er Appendix 3D of the FNDP. be machine extruded, unless ie by Engineer. | Existing Service | es Legend: Boundary (Surveyed) Edge of Road Stormwater Pipe Stormwater Manhole, Wingwall, Scruffy Dome, Catchpit Open Drain Water Pipe Underground Power Lines Power Pole & Over Head Lines Transformer Fibre Optic Lines Top of Bank Bottom of Bank Fence / Gate Retaining Wall | Proposed Services Legen Prop. Ker Prop. Ker Prop. Flux Standard Exposed Gravel St Rock Fille SW Perfor SW Manh SW Catcl SW Swall O Fence | a: b Block b & Channel c & C |
|---|---|---|--|------------------|---|--|--|
| | | | | Road Marking L | <u>egend:</u> White Zebra Crossing 2.0m Long x 0,3m Wide, 0.6m Ga White Stop Sign White Bus Sign White Straight Arrow | Road Marking Leger 1650 Yell | <u>d:</u> Jmm Rubber Wheel Stop ow Disabled Parking Sign ow No-Parking Island |
| | 6 3.6 Handrails on both sic Nosing. Refer to Det | taircase including de and Contrast ail on C321 | N | | White Left-Turn Arrow White Parking Delineation Line 100mm Wide Yellow No-Stop Lines 1.0m Long x 1.0m Gap | | |
| | | Existing Post boxes to be protected and relocated to location confirmed by client Existing Asphalt Area to be Stripped & Replaced with Topsoil & Seeded with Grass | | | | | |
| A ST | | | < | | | | |
| | SH10 | Ø150 SED Timber Bollards @ 2m c/c 1m High and Concreted 1m into ground m Hotmix Transition between g asphalt and proposed concrete | | | | 0 25 | 5 7 5 10 12 5 |
| | | | | | | | (m) |
| Copyright: LDE All rights reserved / Do not scale off drawings / Confirm all dimensions on site prior to work Client Oruaiti School Board of Trustees c/o Avail Pacific | Project 2554 State Highway 10, Mangonui Proposed Carpark Upgrades | Drawing Title Parking Area & Accessway Design Plan Sheet 3 of 3 | | | Issued for Resource Consent N. D 281 Revision (Entrance Grassed Area + Signage) N. D 5 S | Design: R. de Swardt Drawn: R. de Swardt Mar 2025 Approved: N. Douglas | Project status: Project status: Project status: Project Projec Project Project Projeco |
| | | | engineers • scienti: | STS A I | Issued for Consent H. D 1 M Issue/Revision Aprvd | ay 2024 Scale A3: 1:250 Date | |





| | - | Upgraded (| Carpark | | | Upgraded Acces | sway | | Existin Access | ng way | Proper | ty Boundary 🖳 | |
|-------------------------|--------------------------|------------|---------|-------|-------|----------------|-------|-------|-------------------|-----------|--|---------------|----------|
| | | | | | | | | | | | Existing Fence | | <u> </u> |
| Vertical E Datum: (| Exaggeration: 2.5 0.0 | | | | | | | | | | Vertical Exaggeration: 2.5 Datum: 0.0 | | |
| Desigr | n Levels | 4.95 | 5.17 | 5.31 | 5.37 | 5.36 | 5.27 | 5.18 | | | Design Levels | | 4.29 |
| <u>Exis</u> ti <u>n</u> | ng Levels | 5.02 | 5.16 | 5.26 | 5.43 | 5.32 | 5.22 | 5.08 | | | Existing Levels | 4.32 | 4.21 |
| Cut/Fil | ll Depth | -0.08 | 0.01 | 0.05 | -0.06 | 0.03 | 0.05 | 0.10 | | | Cut/Fill Depth | | 0.07 |
| Chaina | age | 5.00 | 00.01 | 15.00 | 20.00 | 25.00 | 30.00 | 35.00 | | | Chainage | 5.00 | 10.00 |

Long Section (ACCESSWAY CL)

Long Section (PARKING AREA CL) Chainage 0m to 25m

| | | | | | | | | | | | | | | | | | | F |
|---------------------------------------|------|------|----------|--------------|----------|------|------|------|------|------|------|------|------|-------|-------|------|------|---|
| | | · · | <u> </u> | | <u> </u> | | | - | | | | | ~ | | | | | |
| | | | | | | | / | | | | | | | | | | | |
| | | | | | | | | | | | | | | | | | | |
| | | | | | | | | | | | | | | | | | | |
| | | | | | | | | | | | | | | | | | | |
| | | | | | | | | | | | | | | | | | | |
| Vertical Exaggeration: 2.5 Datum: 0.0 | | | | | | | | | | | | | | | | | | |
| | 4 | 0 | | | | N | 0 | | | m | ~ | 10 | 0 | m | 10 | 0 | (0) | |
| Design Levels | 4.3 | 4.3 | 4.3 | 4 | 4 .2 | 4 2 | 4.2 | 4.2 | 4.4 | 4.1 | 4.2 | 4.2 | 4.4 | 4.4 | 4.7; | 4.9 | 5.16 | |
| Existing Levels | .26 | 19 | .17 | , | .07 | 80 | .97 | 76. | .01 | .02 | .01 | 56. | 28 | .57 | 62. | 98 | .92 | |
| | 4 | 4 | 4 | 4 | 4 | m | ю | Ϋ́ | 4 | 4 | 4 | ო | 4 | 4 | 4 | 4 | 4 | |
| Cut/Fill Depth | 0.08 | 0.13 | 0.14 | 0.18 | 0.20 | 0.24 | 0.25 | 0.24 | 0.18 | 0.16 | 0.19 | 0.28 | 0.12 | -0.14 | -0.04 | 0.01 | 0.23 | |
| Chainage | 00 | 00 | 00. | E | 8 | 00. | 00 | 00. | 0 | 00. | 00. | 00 | 00. | 00. | 00. | 00.(| 00.2 | |
| | 25 | 30 | 35 | 40 | 45 | 20 | 55 | 60 | 65 | 20 | 75 | 80 | 85 | 6 | 95 | 100 | 105 | |

Long Section (PARKING AREA CL) Chainage 25m to End

Copyright: LDE All rights reserved / Do not scale off drawings / Confirm all dimensions on site prior to work Client





| | | | | | |
|-----|------|-----|------|-----|--|
| | | | | | |
| | | | | | |
| | | | | | |
| | | | | | |
| | | | | | |
| | | | | | |
| | | | | | |
| | | | | | |
| | | | | | |
| 7 | | 9 | | 4 | |
| 4.3 | | 4.3 | | 4 | |
| 24 | | 25 | | g | |
| 4.2 | | 4.2 | | 4 | |
| 13 | | 11 | | 8 | |
| 0.1 | | ò. | | 0.0 | |
| 00 | | 8 | | 8 | |
| 15. | | 20. | | 25. | |

| | | | | | Original size = A3 |
|---------------|--------------|---------------------------|-----------|---------------|-------------------------------------|
| | | | Design: | R. de Swardt | Project status: Resource Consent |
| | | | Drawn: | R de Swardt | Preiost |
| | | | Biami | rt. do ondiat | 21102 |
| | | | Approved: | N. Douglas | 21102 |
| ea + Signage) | N. D N. D | 28 Mar 2025 5 Sep 2024 | | | Drawing No: Issue/Rev |
| 5 57 | H. D | 1 May 2024 | Scale A3: | 1:250 | C310 C |
| vision | Aprvd | Date | | | |



| Design: R. de Swardt Project status: Resource Cons | ent |
|---|-----------|
| Drawn: R de Swardt | |
| Project O1400 | |
| Approved: N Develop 21102 | |
| N. D 28 Mar 2025 Approved. N. Douglas | Issue/Rev |
| ea + Signage) N. D 5 Sep 2024 | 0 |
| H. D 1 May 2024 Scale A3: 1:75 C31 | I C |
| ision Aprvd Date | |











Copyright: LDE All rights reserved / Do not scale off drawings / Confirm all dimensions on site prior to work Client

Oruaiti School Board of Trustees c/o Avail Pacific

Project 2554 State Highway 10, Mangonui Proposed Carpark Upgrades Drawing Title Parking Area & Accessway 5m Inteval Cross Sections Sheet 1 of 3





| | | | | | Original size | e = A3 |
|----------------|----------------------|--------------------------|-----------|--------------|-----------------------------------|----------|
| | | | Design: | R. de Swardt | Project status: Resource Conse | nt |
| | | | Drawn: | R. de Swardt | Project: | |
| | ND | 29 Mar 2025 | Approved: | N. Douglas | 21102 | |
| rea + Signage) | N. D N. D H. D | 5 Sep 2024 1 May 2024 | Scale A3: | 1:250 | C312 | csue/Rev |
| vision | Aprvd | Date | - | | | <u> </u> |







7.5

5

12.5

15

10





Copyright: LDE All rights reserved / Do not scale off drawings / Confirm all dimensions on site prior to work Client

Oruaiti School Board of Trustees c/o Avail Pacific

Project 2554 State Highway 10, Mangonui Proposed Carpark Upgrades Drawing Title Parking Area & Accessway 5m Inteval Cross Sections Sheet 2 of 3





| | | | | | Original | size = A3 |
|--------------|-------|-------------|-----------|--------------|-----------------|------------|
| | | | Desian: | R. de Swardt | Project status: | |
| | | | | | Resource Cons | sent |
| | | | Drawn: | R. de Swardt | Project | |
| | | | | | 21102 | |
| | ND | 28 Mar 2025 | Approved: | N. Douglas | 21102 | |
| a + Signage) | N. D | 5 Sep 2024 | | | Drawing No: | Issue/Rev: |
| | H. D | 1 May 2024 | Scale A3: | 1:250 | I C313 | |
| sion | Aprvd | Date | - | | | - |
| | | | | | | |
| | | | | | | |













C Issued for Resource Consent B Revision (Entrance Grassed Area + Signage) A Issued for Consent No. IssuedRevision







Copyright: LDE All rights reserved / Do not scale off drawings / Confirm all dimensions on site prior to work Client

Proiect



R. de Swardt Design: Resource Consent R. de Swardt Drawn: 21102 N. Douglas Approved: N. D B Revised and Issued for Resource Conse B B C321 Scale A3: As Shown

Original size = A3





Long Section (SWCP1 TO SWMH3)





Long Section (SWCP7 TO SWMH3)

Copyright: LDE All rights reserved / Do not scale off drawings / Confirm all dimensions on site prior to work Client

Oruaiti School Board of Trustees c/o Avail Pacific

Project 2554 State Highway 10, Mangonui Proposed Carpark Upgrades Drawing Title Parking Area & Accessway Stormwater Longitudinal Sections Sheet 1 of 2



ssued for Resource C

| | | | | | Original siz | e = A3 |
|---------------|-----------------------|----------------------------------|-----------|--------------|-----------------------------------|---|
| | _ | | Design: | R. de Swardt | Project status: Resource Conse | nt |
| | | | Drawn: | R. de Swardt | Project: | ,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,, |
| | N. D | 28 Mar 2025 | Approved: | N. Douglas | 21102 | Issue/Rev |
| ea + Signage) | N. D H. D Aprvd | 5 Sep 2024 1 May 2024 Date | Scale A3: | 1:250 | C410 | С |



Long Section (SPORTS FIELD CATCHPIT TO TANK)



Copyright: LDE All rights reserved / Do not scale off drawings / Confirm all dimensions on site prior to work Client

Oruaiti School Board of Trustees c/o Avail Pacific

Project 2554 State Highway 10, Mangonui Proposed Carpark Upgrades

Drawing Title Parking Area & Accessway Stormwater Longitudinal Sections Sheet 2 of 2





| | | | | | Original size = A3 |
|---------------|--------------|--------------------------|-----------|--------------|--|
| | | | Design: | R. de Swardt | Project status: Resource Consent |
| | | | Drawn: | R. de Swardt | Project. |
| | ND | 28 Mar 2025 | Approved: | N. Douglas | 21102 |
| ea + Signage) | N. D H. D | 5 Sep 2024 1 May 2024 | Scale A3: | 1:250 | C411 C |
| /ision | Aprva | Date | | | Drive Location C., Saved by ReanDeSwardt, Document 42 455509, Venion |





Sheet 39 Standard Precast Manhole 200 for pipes up to 300 dia. B 600 for pipes over 300 dia Opening to be over outlet for sanitan sewer only. Short no longer 2 than 1000mm for uP.V.C. Ceramic joints in half B -pipe channels to be filled with epoxy mortar SECTION CC NOTES: 1. This detail is applicable for pipe diameters up to 600mm & for manhole depths up to 5.0m 2. 150mm thick reinforced concrete lids with heavy duty ductile iron frames & covers to be used in driveways, carriageways & berms. 100mm thick concrete lids with light duty cast iron frames & covers may be used elsewhere. 3. Precast manhole bases shall be used in all instances with minimum sized holes cut for pipe entry. 4. No additional thin plastering of benching or benching of inverts is permitted. 5. All concrete to be 20MPa. 7. Where non-concrete pipe connections are made to concrete manholes, then a gritted starter pipe shall be installed to allow bond between manhole and pipe material. Also, a 3flexible joint should be specified as part of gritted starter pipe (as standard) 8. All manholes >1.2m in depth shall be provided with manhole step rungs. These shall follow the requirements on sheet 40 London junction Epoxy mortar around pipe Short - no longer than 1000mm Dropper pipe for sewer only (single piece) -Where the height of drop exceeds 1.5m the dropper pipe shall be rigidly attached to the wall of the manhole Mortar around pipe with approved Grade 316 SS or plastic supports at 1.5m centres max. -88° bend set in benching 50mm layer of – 20MPa concrete -Half pipe channel (ceramic or uPVC) SECTION A-A INTERNAL DROP Note 1. This detail is applicable for pipe diameters up to 250mm & for manhole depth up to 5.0m & for manhole diameters ≥ 1200mm. 2. External drops shall not be used STANDARD PRECAST MANHOLE sewer and stormwater FOR ALL ENVIRONMENTS FAR NORTH DISTRICT COUNCIL

FAR NORTH DISTRICT ENGINEERING STANDARDS

engineers • scientists

| Client |
|--|
| Oruaiti School Board of Trustees c/o Avail Pacific |

2554 State Highway 10, Mangonui Proposed Carpark Upgrades Parking Area & Accessway Typical Stormwater Details



| | | | | | • |
|----------------|-------|-------------|--------------|--------------|-------------------------------------|
| | | | Design: | R. de Swardt | Project status: Resource Consent |
| | | | | | |
| | | | Drawn: | R. de Swardt | Project: |
| | | | | | 01100 |
| | | | A | N. Davialaa | 21102 |
| | N. D | 28 Mar 2025 | Approved: | N. Douglas | Denvice Net |
| rea + Signage) | N. D | 5 Sep 2024 | | | Drawing No. Issuencev |
| | H. D | 1 May 2024 | Scale A3: As | As Shown | C421 C |
| vision | Aprvd | Date | | | • • • • • |



Oruaiti School Board of Trustees c/o Avail Pacific **CIVIL TECHNICAL SPECIFICATION** 2554 State Highway 10, Mangonui (Oruaiti School)

Project Reference: 21102 April 1, 2025

DOCUMENT CONTROL

| Version | Date | Comments |
|---------|--------------|------------------------------------|
| А | 2 May 2024 | Issued for Tender and Construction |
| В | 1 April 2025 | Revised and Re-issued |

| Version | Issued For | Prepared By | Reviewed & Authorised By |
|---------|----------------------------|--|--|
| В | Issued for Construction | Marsware | |
| | | Riaan De Swardt Senior Designer Engineer <i>MEngNZ</i> | Neil Douglas Civil Manager – Rodney / Kaipara / Northland <i>MEngNZ</i> |



Table of Contents

| 1 | GEI | NERAL | 3 |
|-----|------------|--|----------------------|
| 1 | 1.1 | Extent of Works | .3 |
| 1 | .2 | Compliance | .3 |
| 1 | .3 | Set Out | .4 |
| 1 | 4 | Protection of Property & Services | 4 |
| - | 5 | Materials | . 1 |
| | 1.0 | Sodimont & Frazian | / |
| | 1.0 | | .4 5 |
| | | | .5 |
| • | 1.0 | | .5 |
| 2 | DEN | NOLITION AND REMOVAL | 5 |
| 2 | 2.1 | General | .5 |
| 2 | 2.2 | Existing Concrete | .6 |
| 2 | 2.3 | Trees and Stumps | .6 |
| 3 | EAF | RTHWORKS | 6 |
| 3 | 3.1 | General | .6 |
| 3 | 3.2 | Scope | .6 |
| 3 | 3.3 | Standards | .7 |
| 2 | 34 | Drainage Control | 7 |
| | 3.5 | Clearing and Removal of Vegetation | 7 |
| | 2.0 2.6 | Tolerances | ۰، م |
| | 2.0 | Inspections and Approvals | υ. Ω |
| | 0.7 | | 0. |
| | 0.0 | | .0 |
| | 5.9 | | .ŏ |
| 3 | 3.10 | Materials (site won suitable for re-use) | .8 |
| č | 3.11 | Completion of Earthworks | .9 |
| 4 | PAV | /EMENT | 9 |
| 2 | 1.1 | Concrete | .9 |
| | 4.1.1 | 1 Subgrade | .9 |
| | 4.1.2 | 2 Basecourse | .9 |
| | 4.1.3 | 3 Form Work | .9 |
| | 4.1.4 | 4 Concrete Surface | 10 |
| | 4.1.5 | 5 Kerb and Channel | 10 |
| | 416 | 6 Hotmix Transition | 11 |
| 2 | 12 | Testing and Frequency | 11 |
| 2 | 13 | Pavement Marking | 11 |
| 5 | STC | | 11 |
| J . | | | 44 |
| 5 | D. I | General | 11 |
| 5 | 5.2 | Standards | 12 |
| 5 | 5.3 | Existing Services | 12 |
| 5 | 5.4 | Excavation | 12 |
| 5 | 5.5 | Materials | 12 |
| | 5.5.1 | 1 Bedding and Backfill | 13 |
| 5 | 5.6 | Minimum Cover During Construction | 13 |
| 5 | 5.7 | Manholes | 13 |
| 5 | 5.8 | Inspection Chambers | 13 |
| 5 | 5.9 | Catchpits | 13 |
| 5 | 5.10 | Drainage Pipes | 13 |
| Ę | 5.11 | Pipe Fittings | 13 |
| Ē | 5.12 | Subsoil Pipes | 14 |
| - | 5 13 | Rock Filled Traffic Island | 14 |
| 2 | 5 14 | Filtration & Subsoil Trenches | . . 1⊿ |
| 5 | 5 15 | Minimum Cover | 11 |
| 5 | 5 16 | Backfill | 14 17 |
| 5 | 5.10 | Conoroto Conning | 14 14 |
| 5 | 2.17 | | 14 |
| ÷ | 0.18 0 | | 15 |
| 6 | COI | MPLIANCE DELIVERABLES | 15 |
| 7 | HOI | LD POINTS AND ENGINEER INSPECTIONS 1 | 15 |
| 8 | COI | MPLETION OF WORKS | 16 |



1 GENERAL

1.1 Extent of Works

This specification is to be read in conjunction with the supplied documents and drawings. The works detailed herein are to construct a new carpark, accessway and associated stormwater infrastructure.

The works include the following:

- 1. Earthworks to form all new hardstand areas and drainage.
- 2. The removal of existing concrete, asphalt, and gravel in preparation for the construction of the new concrete carpark and accessway.
- 3. All necessary works to form concrete surfaced carparks, gravel accessway and a concrete footpath.
- 4. Stormwater drainage to service the new and existing hardstand areas as shown on the drawings, including the supply of all materials and labour.
- 5. The installation of stormwater treatment solutions including the supply of all materials and labour.
- 6. Remediation, topsoiling and grassing of all disturbed surfaces upon completion.

All temporary works and traffic management shall be included in the Contractor's tender documents.

Due consideration shall be made to allow the occupants of the neighbouring properties access to their property with minimal disturbance and inconvenience. The contractor shall be responsible for liaising with the occupants and advisement of the timing of works which may affect them.

1.2 Compliance

All works are to comply with the relevant Council Engineering Standards.

All work and operations are to comply with all respects of any Resource Consent and Building Consent issued for the work at all times.

Consent to work within the Road Reserves are to be obtained from the Council by the Contractor where required.

All internal quality assurance, control and testing are to be undertaken by the Contractor in accordance with the supplied documents.

The Contractor is to advise the Engineer and/or the Council (where necessary), a minimum of 24-48 hours prior to an inspection or testing required and allow for adequate time for inspection purposes in his programme.

Where any inspection and/or testing does not comply with the Specification, then the cost of re-inspection and/or testing shall be borne by the Contractor including any costs incurred by the Engineer or the Principal.



Upon completion of the works a Producer Statement will be required for all civil works including Earthworks, Drainage and Pavements stating that all the works have been constructed in accordance with all the requirements of the supplied documents. The Producer Statement is to be supported by all test data, engineering inspections etc as required by this Specification.

1.3 Set Out

The works shall be set out on site by the Contractor from control points supplied and confirmed by a Surveyor on site. Once the initial set out has been undertaken, it shall be the responsibility of the Contractor to protect and maintain any set out pegs. Set out pegs shall be maintained throughout the course of the works as reference points.

In the event that the Contractor desires to fix alternative and/or additional pegs for ease of setting out, he shall do so at his own expense. The Contractor shall be entirely responsible for ensuring and checking the accuracy of such additional pegs.

1.4 Protection of Property & Services

The Contractor shall take all necessary measures to protect all private and public property and services from damage during the course of the proposed works.

Any damage caused by the Contractor shall be reinstated at the Contractor's expense.

1.5 Materials

These sections of the specification detail the materials required for the works, and the quality assurance requirements for the verification of materials supplied. All sampling and testing of materials shall be undertaken in accordance with the relevant standards and specifications. All materials utilised in the works are to comply with the relevant standards and specifications not covered by a standard or specification shall be of the best quality available.

The Contractor shall take samples and carry out pre-acceptance testing where required by the Engineer. The Engineer reserves the right to collect additional samples from the source of supply themselves. Copies of all test results are to be forwarded to the Engineer upon receipt by the Contractor.

1.6 Sediment & Erosion

Sediment and erosion control shall be implemented where required prior to undertaking any work. These methods should comply with the relevant regional and district council standards. Suitable methods for controlling sediment loss and erosion on earthworks sites can be found in Auckland Councils GD05 Erosion and Sediment Control Guidelines. Where no other relevant council standards are available these shall apply.


The Contractor is responsible for organising inspections where required with the relevant district and regional council representatives and shall maintain sediment controls for the entire duration of the works to the required standards. Immediately upon completion of the works sediment controls shall be removed and the site shall be reinstated to fully vegetated condition as directed by the Engineer, construction plans and any council representative.

The Contractor can prepare an erosion and sediment control plan based on anticipated staging of work. These will be submitted and discussed with Council on site prior to commencement of physical works. Alternatively, the appointed Engineer can prepare these plans on the Contractor's behalf. Any changes or adjustments to the approved plans will be discussed with Councils monitoring officer prior to implementation on site.

Maintenance of all controls and devices is the responsibility of the Contractor.

1.7 Archaeological

In the event that any Koiwi (human remains) or artefacts which may be of historic or cultural importance are discovered, all work in the vicinity shall cease immediately and tangata whenua, the New Zealand Historic Places Trust and the relevant District Council shall be immediately notified to determine any necessary protection, remediation, or mitigation.

1.8 As-Built Plans

As-built plans will be required and should meet the requirements of the relevant Council authority where necessary.

An electronic copy of the as-built plans is required. As-built survey is to be in terms of the same control as the setout for the works. The Contractor is responsible to notify the Surveyor to capture relevant information as and when required on site.

2 DEMOLITION AND REMOVAL

Refer to drawing sheer 21102-C002 for the Demolition and Reinstatement Plan.

2.1 General

The extent of all structures/features requiring demolition and removal from site are shown on the drawings and are to be confirmed on site with the Engineer and School Principal/Representatives.

Unless instructed by the Engineer and School Principal/Representatives, all material is to be removed from the site and disposed off-site at an appropriate location depending on the material type.



2.2 Existing Concrete

The Contractor will account for the removal of the existing concrete with the disposal of all excess material off-site to a suitable location.

Where required, removal may involve saw cutting to allow for the construction of the new structures as shown on the drawings.

Saw cuts shall be made with a power saw and the cut is to extend through the full thickness of the surface layer in clean straight parallel lines.

2.3 Trees and Stumps

Where required, trees and stumps are to be removed and disposed of off-site to a suitable location. All stumps and root systems within the new carpark and access way are to be fully removed during the excavation. Stumps within proposed grassed/lawn areas shall be cut off at 150mm below the finished surface level.

3 EARTHWORKS

3.1 General

Earthworks will be required to form the levels shown in the construction drawings. Earthworks include the stripping and disposal off-site of vegetation, soil, or material within the extent of the earthworks, the stripping and stockpiling of any topsoil for reuse, and the excavation of the working bench to the specified lines and levels.

The earthworks are to be carried out in general accordance with NZS4431:2022 Engineered Fill Construction for Lightweight Structures.

If any non-engineered fill or contaminated material is encountered during the work, the Contractor shall notify the Engineer, Client, and any other relevant authority.

3.2 Scope

Earthworks include the stripping and disposal off-site of vegetation within the extent of the earthworks. Bulk earthworks will involve the cut to waste as show on the drawings that is to be disposed of off-site.

The Contractor shall carry out the earthworks on the site in accordance with the requirements detailed below and in conjunction with the requirements of the drawings and the various standards.

The cut slopes are not to exceed the parameters outlined in the drawings.

Work included in this specification:



- Excavation, load and dispose of material removed to form level ground.
- Preparation of any area to be filled including benching and subsoil drainage as required.
- Excavate, load, cart, spreading and drying if required, placement, stabilising if required and compaction of engineered fill material.
- Preparation and protection of all works until vegetation is re-established.
- Re-form the grassed area on the edge as shown in the design drawings.

3.3 Standards

The Standards to be used shall include, but not be limited to:

- NZS 4431: 2022 Engineered fill construction for lightweight structures
- NZS 4402: 1988 Methods for Testing Soils for Civil Engineering Purposes.
- NZS 4404: 2004 Land Development and Subdivision Engineering.

The documents listed above and cited in this section are deemed to form part of this specification. However, this specification takes precedence in the event of it being of a higher standard than the cited documents.

3.4 Drainage Control

All earthworks shall be carried out during extended periods of fine weather and in fully drained conditions with no free water on the working surfaces. All preparatory excavation work and subsequent excavations in cut areas or areas to be filled shall be kept effectively drained at all times. Cut and fill areas shall be sloped and graded adequately at all times so that they do not pond water.

Any fill or final excavation surface materials which have been allowed to become too wet or soft shall be removed and dried or replaced.

The Contractor shall dig such trenches and provide such other facilities as may be required to ensure that stormwater will drain quickly off the exposed areas. These must be constructed so as not to flood or damage any property and shall be filled in or removed on completion of the works.

The surface of fill material shall be temporally compacted and shaped to allow water runoff with appropriate plant when rain is impending.

3.5 Clearing and Removal of Vegetation

The Contractor shall remove all vegetation and topsoil from the area of earthworks and shall clear all obstructions and rubbish from the area of the works except those specifically identified by the Engineer as remaining.

All vegetation and rubbish are to be disposed of at a suitable location as directed by the Client or Engineer.



The maximum recovery of topsoil is essential to be able to satisfactorily reinstate the disturbed areas on completion of the earthworks. Topsoil should be stockpiled for later use.

The earthworks profiles shall generally be trimmed to match and blend with adjacent undisturbed existing ground.

3.6 Tolerances

All earthworks shall be carried out to the lines, levels and grades shown on the Construction Drawings or as otherwise instructed by the Engineer. Tolerances shall be as follows:

• Finished levels ± 15mm

3.7 Inspections and Approvals

Before fill is placed in any area the Engineer shall be notified so that he can inspect the stripped surface and instruct whether further excavation and/or undercutting and backfilling is required or other works such as drainage are necessary. No fill shall be undertaken in an area until such inspections of the stripped surface, and any other works that may be required below the stripped surface have been made and the Engineer has approved the commencement of installation of fill.

3.8 Cut to Fill

The cut and fill areas are shown on the drawings.

Fill materials shall be sourced from areas of cut and installed where required in layers. The Contractor shall take all precautions and maintain a tidy operation. The Contractor shall also ensure that all fill is free of organic matter or other unsuitable materials.

3.9 Cut to Waste

The Contractor is responsible for finding a suitable disposal site for the excess material and placing and compacting the material in a suitable manner to not cause any land instability or erosion.

The Contractor shall determine a safe method to form and maintain stable cut slopes during construction.

3.10 Materials (site won suitable for re-use)

The fill material shall only contain suitable material i.e. free from any organic material or soft plastic clays. It is anticipated that material stockpiled during the excavation may be reused, subject to the approval of the Engineer.

In case there is not enough material suitable for reuse, GAP65 or similar engineered aggregate will be used.



3.11 Completion of Earthworks

The finished surface shall be shaped so it sheds surface water. Any exposed slopes shall be track rolled as detailed in Auckland Councils ARC GD05 erosion and sediment control guidelines, to reduce the risk of erosion. The exposed surface is to be stabilised immediately following the completion of work. All exposed soil areas are to be mulched and seeded immediately following the completion of the works and are to achieve a minimum grass strike of 75% or more within 6 weeks.

Work shall not be deemed complete until passed by the Engineer and the local Territorial Authority if required.

4 PAVEMENT

Pavement for the carparks and accessway shall be constructed to the following specifications.

The Contractor will organise the testing and inspection with certifying Engineer prior to any backfilling commences. When work is to be undertaken over multiple days and stages, schedule of inspections will be discussed with Engineer.

4.1 Concrete

4.1.1 Subgrade

The subgrade shall have any soft spots dug out, backfilled and thoroughly compacted. The subgrade shall be free of any surface water, shall be trimmed accurately to shape and roll, to ensure a tight well shaped surface which will not pond or absorb water. Camber should be as shown in the design plans.

4.1.2 Basecourse

When placing a basecourse, watering to achieve optimum water content is often critical. However, over watering and over rolling can be more damaging than underwatering and rolling and the critical point can be reached very quickly. Compaction should generally consist of 5 to 8 passes with a small steel wheel roller on GAP40 material.

Only use a vibrator roller for compacting the upper layers of basecourse sparingly as it crushes the aggregate creating more fines and making the basecourse more susceptible to plastic failure in the presence of water.

4.1.3 Form Work

Form work shall be straight and true and set out in a "pleasing to the eye" manner. Curves shall be curvilinear with a smooth transition to the adjoining straights or other curves.

Form work shall be well secured so as not to dislodge under induced working loads from poured concrete etc.



Unless otherwise approved, form work shall be well oiled and maintained in good order.

4.1.4 Concrete Surface

Concrete work shall be carried out in general accordance with NZS 3109:1997 Concrete Construction, and the General Requirements of the relevant Council Engineering Standards.

Concrete shall have a 28-day compressive strength of not less than 20 MPa, a slump of 100 mm, and aggregate size of approximately 19 mm maximum and shall be from an approved matching plant delivered by a ready-mix truck. The concrete aggregate shall be reasonably free of silt and clay particles sized fines as these can contribute to shrink /swell and associated cracking. The Client will confirm preferred concrete finish prior to construction.

The footpath shall be cast as 100 mm thick concrete slab. The concrete should have crossfall as noted on the drawings. The concrete footpath shall have an exposed aggregate finished surface.

The carpark shall be cast as 150 mm thick concrete slab, with 665 Mesh placed centrally. The concrete should have a crossfall as noted on the drawings. The concrete carpark shall have a Class U5 Broomed finish in accordance with NZS 3114:1987.

Prior to concreting, the subgrade aggregate shall be moistened to reduce moisture draw out from the concrete.

Any potential cold joints between pours shall be squared off to provide control joints as per Schedule of Quantities comments.

4.1.4.1 Protection and Curing

Steps must be taken to protect the new surface from the effects of heavy rain, pedestrian, and other traffic. During hot weather, or at other times as required, curing of the concrete is to be affected by means of covering with damp hessian, sand, curing membrane or other means.

4.1.4.2 Crack Control

Crack control joints (saw-cuts) shall be formed as shown in the drawings at max 3 metre centres (as per Schedule of Quantities). Saw-cuts depths shall be 1/3 of the concrete thickness. Any deviations shall be discussed and confirmed by the Engineer prior to the commencement of works.

4.1.5 Kerb and Channel

Concrete kerb and channel are to be installed as specified in the engineering drawings. Saw cuts are to be installed at maximum 3m centres, in line with footpath saw-cuts if possible. In-situ concrete shall have a 28-day compressive strength of not less than 20 MPa.



4.1.6 Hotmix Transition

A 500mm Hotmix Transition shall be installed between the edge of the SH10's existing asphalt and the proposed concrete carpark in accordance with the relevant Council standards.

4.2 Testing and Frequency

The Contractor is responsible for carrying out the testing. The Engineer shall carry out random testing in addition to the Contractor's regular test records to confirm that the required compaction is being achieved.

Testing is to be carried out as denoted in Table 2 below.

| Material | Test | Frequency | Target | |
|---|--|---|-----------------------|--|
| | Shear Vane (for cohesive material) | 1 test every 50m², every 3 lifts (max 0.6m) | 100kPa/good ground | |
| Subgrade | Scala Penetrometer (for non-cohesive material) | 1 test every 50m ² | Average 2 blows/100mm | |
| Basecourse for concrete Clegg impact test carpark | | 1 test every 50m ² | CIV≥15 | |
| Basecourse for footpath | course Clegg impact test 1 test every 50m ² | | CIV≥15 | |

Table 2 - Testing and Frequency for new pavement.

4.3 Pavement Marking

On completion of the final seal coat, the contractor shall undertake the markings that are required as part of this contract.

All materials, equipment and procedures shall be in accordance with the current requirements of MOTSAM Manual of Traffic Signs & Markings.

All paint for markings shall conform to NZTA Specification M07 Notes and be to the specific line width and spacing as set out in the manual.

5 STORMWATER DRAINAGE

5.1 General

This section includes the supply of materials, excavation for, installation of and backfilling and completion of all stormwater and subsoil drainage.



All pipework and subsoil drains are to be laid to the lines and levels as detailed on the plans. All materials used and all work shall meet the relevant standards including but not limited to those covered in NZS, NZ Building Code and relevant Council Standards.

5.2 Standards

The Standards to be used shall include but not be limited to:

| NZS 3107:1978 | Specification for pre-cast concrete drainage and pressure pipes | | | |
|--------------------|---|--|--|--|
| AS/NZS 5065:2005 | Polyethylene and polypropylene pipes and fittings for drainage an sewerage applications | | | |
| NZS/AS 3725:1989 | Loads on buried concrete pipes | | | |
| AS/NZS1260:2003 | PVC-U pipes and fittings for drain, waste, and vent application | | | |
| AS/NZS 3500.2:2003 | Sanitary plumbing and drainage | | | |
| AS/NZS 3500.3:2003 | Stormwater drainage | | | |
| TNZ F/2 | Specification for pipe subsoil drain construction | | | |
| TNZ F/6 | Specification for geotextile wrapped aggregate subsoil drain construction | | | |
| TNZ F/7 | Specification for geotextiles | | | |
| CPAA TN-01 | Installing small diameter concrete pipe under roads (Nov 2010) | | | |

The latest version shall be used unless indicated otherwise.

5.3 Existing Services

All existing services to be located and protected at all times during construction. Any damage and remediation to existing services shall be the responsibility of the Contractor.

5.4 Excavation

Excavation for all drains as necessary to provide the required cover, bedding depth and trench width.

All pipes to be laid to the levels and grades as shown on the plans.

All excavated excess material to be disposed off-site unless instructed otherwise by Client or Engineer.

5.5 Materials

All materials shall comply with the Standards above and be as detailed on the design drawings.



5.5.1 Bedding and Backfill

Bedding material shall be 7-20mm clean, free draining drainage metal.

All engineered backfill is to be compacted to 95% of NZS Standard Density. Care is to be taken to ensure good compaction is achieved along the sides of the pipes, especially under the new pavement.

5.6 Minimum Cover During Construction

All pipework to have a minimum 600mm of compacted cover prior to loading with construction vehicles, unless noted otherwise.

5.7 Manholes

All manholes are to be Ø1050mm precast concrete chambers, and depth in accordance with design drawings and longitudinal sections, unless noted otherwise. Chambers shall be with a precast base. A trafficable frame and lid will be used for the areas with vehicle loading.

5.8 Inspection Chambers

Inspection chambers are to be Ø675mm precast concrete chambers, and depth in accordance with design drawings and longitudinal sections. Chambers shall be fit with a precast base. A trafficable frame and lid will be used for the areas with vehicle loading.

5.9 Catchpits

All catchpits to be precast concrete 450x675mm internal dimensions, fitted with trafficable frame and grated lid. Sump depth to be min 300mm below invert of the outfall pipe, or in accordance with the design drawings.

All catchpits to be fitted with a "Stormwater360 Litta Trap" and installed as per supplier specifications.

5.10 Drainage Pipes

Stormwater drainage pipe materials and sizes shall be as detailed on the engineering drawing. Any changes will be discussed and approved by the Engineer prior to ordering and installing the alternative product.

5.11 Pipe Fittings

Pipe fittings are to be jointed and compatible with the pipe used.



5.12 Subsoil Pipes

Subsoil pipes are to be HDPE corrugated perforated pipe to the requirements of TNZ F/2.

5.13 Rock Filled Traffic Island

The rock filled filtration island shall be fitted with 100-150mm diameter rocks to a depth of at least 200mm.

5.14 Filtration & Subsoil Trenches

Filtration trenches to be constructed in accordance with the design drawings and. Trenches to be filled will clean drainage metal and wrapped in a filter cloth.

5.15 Minimum Cover

Minimum cover to pipes is to comply with table below.

| Location | Depth |
|--------------------------------------|-------|
| Areas not subject to traffic loading | 600mm |
| Areas under carriageway | 900mm |
| Areas under concrete footpath | 300mm |

Where minimum cover cannot be achieved then reinforced concrete capping is to be provided.

All stormwater pipes are to have a minimum 900mm of compacted cover prior to loading with construction vehicles. Alternatively, temporary protection should be provided in instances where this is not possible.

5.16 Backfill

Any trench located in an area subject to vehicle loading is to be backfilled to the underside of the basecourse layer with GAP65 aggregate compacted in layers not exceeding 200mm loose depth to achieve at least 95% MDD.

Backfill in lawn areas is to be with suitable ordinary fill gained from the excavation and should extend to min 100mm below finished ground level. The top of the trench should be topsoiled and grassed.

5.17 Concrete Capping

Where the required minimum cover cannot be achieved then the pipe shall be capped to provide additional protection, in accordance with the relevant Council Engineering Standards.



Capping shall consist of a 100mm thick 20MPa concrete slab with 688 mesh placed central. The underside of the slab shall be at the top of the bedding/haunching material, i.e. at least 100mm above the top of the pipe, and shall extend the full width of the trench. Any capping required shall be discussed with the Engineer prior to implementation.

5.18 StormFilter

All treatment devices to be ordered and installed as per the design and supplier specifications.

6 COMPLIANCE DELIVERABLES

The Contractor must deliver reports of regular checks for construction, including but not limited to:

- 1. Successful set out of coordinates achieved.
- 2. Successful demolition/removal of existing structures, pavement etc.
- 3. Successful installation of stormwater drainage.
- 4. Successful excavation to design subgrade levels.
- 5. Successful placement and compaction of basecourse.
- 6. Successful access way construction achieved.
- 7. Successful footpath construction achieved.
- 8. Successful installation of the membrane seal.
- As-Built drawings (pdf and dwg) the Contractor shall provide a stamped and signed set of as-built drawings within 5 working days of construction being completed. The as-built drawings shall show exact details of constructed items, levels and any changes to the designed details that have been made during construction. Contractor is also required to provide a PS3 (Producer Statement Construction) to accompany the As-Built drawings.

7 HOLD POINTS AND ENGINEER INSPECTIONS

The following schedule defines the hold points at which times the work covered by this specification shall not proceed until the Engineer has been given the opportunity to inspect the works and/or has been provided the specified information and has approved such. Within 24 hours the Engineer will notify the Contractor if construction may proceed. Hold points will occur:

- 1. Pre Construction Meeting
- 2. To confirm set out coordinates are accurately identified.
- 3. To confirm all demolition/removal has been completed.
- 4. During the installation of drainage to confirm all pipes and manholes have sealed connections are in place and prior to backfilling.



- 5. After installation of stormwater drainage confirming pipe grades and inverts have been achieved.
- 6. During the installation of subsoil drains and connections to catchpits.
- 7. During excavation for carparking and access ways to design subgrade levels including testing.
- 8. Upon completion of placement and compaction of basecourse material for access and footpaths.
- 9. Upon placement of kerbing.
- 10. Upon placement of the membrane seal.
- 11. Final walkover and inspection for defects after completion of the works.

At least 48-24h notice will be given to Engineer prior to required site testing and inspection.

Through duration of the construction certifying Engineer will confirm if any other inspections might be required.

8 COMPLETION OF WORKS

Upon completion all surplus materials and debris are to be removed leaving the whole site in an attractive, tidy, and safe condition to the satisfaction of the Engineer. All existing surfaces which have been disturbed shall be reinstated to at least their original condition. All exposed areas shall be topsoiled, mulched, and seeded where required.





Oruaiti School Board of Trustees c/o Avail Pacific **STORMWATER NEUTRALITY REPORT** 2554 State Highway 10, Mangonui (Oruaiti School)

Project Reference: 21102 April 1, 2025

DOCUMENT CONTROL

| Version | Date | Comments |
|---------|--------------|---|
| А | 1 May 2024 | Issued for Consent |
| В | 10 June 2024 | Compliance Assessment Included – Issued for Consent |
| С | 1 April 2025 | Updated Drawing Set |

| Version | Issued For | Prepared By | Reviewed & Authorised By |
|---------|-----------------------|--|--|
| С | Issued for Consent | Marswart | |
| | | Riaan De Swardt Senior Design Engineer <i>MEngNZ</i> | Neil Douglas Civil Manager – Rodney / Kaipara / Northland <i>MEngNZ</i> |



CONTENTS

| 1 | PRC | OJECT DESCRIPTION | . 1 |
|---|-------|---|-----|
| 2 | SITE | E DESCRIPTION | . 1 |
| 3 | PRC | OPOSED DEVELOPMENT | . 2 |
| 4 | EXIS | STING FLOW REGIME & DOWNSTREAM FLOODING | . 3 |
| 5 | ATT | ENUATION REQUIREMENT. | . 7 |
| 6 | STO | DRMWATER | . 9 |
| (| 3.1 | Stormwater Analysis | . 9 |
| (| 3.2 | Analysis Parameters | . 9 |
| (| 3.3 | Pre-development Peak Runoff | 10 |
| (| 3.4 | Post-development Peak Runoff | 10 |
| (| 3.5 | Analysis Results | 10 |
| (| 3.6 | Stormwater Treatment Devices Recommendation | 11 |
| | 6.6.1 | 1 Sheet Flows from State Highway – New Traffic Island | 11 |
| | 6.6.2 | 2 Stormwater Catchpits | 11 |
| | 6.6.3 | 3 Stormwater Filtration System | 12 |
| 7 | CON | MPLIANCE ASSESSMENT | 12 |
| 8 | LIM | ITATIONS | 18 |
| | | | |

APPENDIX A: HEC-HMS MODEL AND RESULTS APPENDIX B: ENGINEERING DRAWINGS APPENDIX C: SUPPLIER INFORMATION AND TECHNICAL DRAWINGS



1 PROJECT DESCRIPTION

LDE Ltd was engaged by Avail Pacific on behalf of the Oruaiti School Board of Trustees to undertake a stormwater neutrality design related to upgrade of the existing carpark located at 2554 State Highway 10, Mangonui.

The purpose of this report is to determine appropriate stormwater management devices and treatment of the first flush relating to the upgrades. This report has been prepared to support a resource and building consent application.



Figure 1: Site location in relation to the surrounding Mangonui suburbs. Aerial sourced from NRC Hazard Maps

2 SITE DESCRIPTION

Oruaiti School is a primary and intermediate school, catering for student from year one to year eight, and is located at 2554 State Highway 10, Mangonui. The legal description of the site is Pt Allot 18A PSH OF Mangonui East with an area of 41,600 m². The site is located approximately 8km east of the Mangonui township.

The school site contains an admin office, classrooms, a gym, playground, and pool, with existing gravel and asphalt accessways. An existing carpark is located along the frontage of the school on the northern side of the SH10. The school currently has approximately >190 students and 15 staff members. The students are dropped off and picked-up from the carpark by busses and parents in vehicles. See figure 2 below.





Figure 2: Aerial photo of the subject site (indicative boundary in red) sourced from NRC Hazards Maps showing the school and parking area, Oruaiti River, and State Highway 10.

3 PROPOSED DEVELOPMENT

The school's carpark currently faces numerous health and safety risk for anyone visiting the school. The carpark is located along the frontage of the school and SH10. Vehicles leaving the carpark are at times forced to back into the state highway with traffic approaching from the east and west at speeds of around 100km/h.

The development proposal as discussed and agreed to by the school, is to upgrade the carpark in order to make it safer for vehicles and pedestrians. The design approach is to provide a dedicated entrance to the school and exit from the school, with a separate dedicated lane for cars and busses. There will also be a turning lane incorporated for vehicles coming from the west and turning left into the carpark.

There is currently some existing stormwater pipes and structures collecting and conveying runoff to the south via a 900Ø concrete pipe culvert, discharging into the Oruaiti School Environmental Wetlands.





Figure 3: Proposed layout of upgraded carpark and wetland location. Source: LDE Drawings

4 EXISTING FLOW REGIME & DOWNSTREAM FLOODING

A desktop study was undertaken to determine and identify a catchment area of approximately 10ha, flowing from the north towards the south. 3D Modelling software was used to process LINZ DEM data and build an existing ground surface to determine approximate flow paths as indicated below. By utilizing the existing surface data and aerial imaging it was determined that the existing flows are conveyed by both natural and man-made channels throughout the catchment area. The channels connect into pipe culverts at various locations around the perimeter of the school, where the flows are conveyed to the discharge point.

During a site walkover undertaken by an Engineer from LDE in December 2023, some of the existing inlet structures and pipe culvert were identified. It appears that most of the over land flows are captured and conveyed to the discharge point via 2 x existing pipe culverts underneath the school's sport field. One Ø375mm PVC pipe culvert could be identified on site where it starts at a scruffy dome manhole and flows from the north-east towards the south-west. The second culvert's exact location and size is unknown, but from survey and historic site information, it is estimated that a Ø900mm concrete pipe originates from the neighbouring property to the west of the school and flows towards the south-east.



The culvert from the north-east connects into a concrete tank located in the southern edge of the sports field. Information from a drainlayer who installed the tank say that it is a 25,000L Underground Duracrete Tank. From here the tank is fitted with what is assumed to be a Ø900mm concrete outlet pipe, which connects to a second large underground structure directly downstream of the tank. The exact size and depth of this structure is unknown, but it is assumed that it is also a 25,000L underground tank. The second culvert from the north-west, connects into this second tank. From this tank it is assumed that a Ø900mm concrete pipe culvert crosses the State Highway and discharges into a downstream wetland, which continues to flow into the into the tidal zone of the Oruaiti River.



Figure 4: 10ha Catchment Area (red), tanks and culverts (green) and flow directions (blue arrows) and discharge to the wetland. Not to scale.





Photo 1: Scruffy dome and pipe culvert flowing from the north-east. (LDE Site Photo)



Photo 2: First underground concrete tank with inlet from the north-east cuvlvert. (LDE Site Photo)



Photo 3: First underground concrete tank flowing into second underground structure. (LDE Site Photo)





Figure 5: Historic site data indicating both culvert sizes. (Provided by the school)

As per Northland Regional Council Hazard Maps, the risk and extent of the river and coastal flooding is shown to occur downstream of the subject catchment area. The flooding maps can be seen in the figures below.



Figure 6: 10-Year, 50-Year and 100-Year River Flood Hazard Zones - Regionwide Models. (NRC Hazards Maps)





Figure 7: Current, 50 Years, 100Years and 100 Years + Rapid Sea Level Rise Scenario Coastal Flood Hazard Zones. (NRC Hazards Maps)

5 ATTENUATION REQUIREMENT

Due to the discharge of stormwater occurring into the tidal zone of the Oruaiti River (<RL 2.0m), the FNDC Engineering Standards; Dated May 2023, Table 4-1 of the standards is shown below and states that "*If the proposed stormwater discharge is into a tidal zone, then no attenuation is required.*"

| ar North District Council E | ingineering Standards Chapter 4: Sto | rmwater and Drainage |
|---|---|--|
| Criteria | Design Parameter | When required |
| Flood Control (1% AEP event) | Detention required, limiting the post-development 1% AEP event flow rates to 80% of the pre-development 1% AEP event flow rates. | Where downstream flooding hazard has been identified. Where there is no CMP or site-specific SMP [.] 1. Refer to Flood Hazard Areas in the <u>District Plan</u> and any known downstream restrictions causing flooding. |
| Flow attenuation (Attenuation of the 50% and 20% AEP events) | Limit the post-development 50% and 20% AEP event flow rates to 80% of the pre-development flows through controlled attenuation and release. | Where there is no CMP or site-specific SMP [.] Catchment location dependent. Typically, always required in the upper catchment and sometimes not required where development site is located in proximity to the catchment outlet, discharging to a watercourse with sufficient network capacity, and where flow attenuation may worsen flooding hazards due to relative timing of peak flows. This is subject to assessment demonstrating no negative impacts would occur. If the proposed stormwater discharge is into a tidal zone, then no attenuation is required. |



We have utilised HEC-HMS software to analyse the pre- and post-development runoff flows generated from the catchment area to determine if the existing Ø900mm concrete culvert under the road discharging into the wetland area has sufficient capacity to accommodate the increase in impervious area generated from the carpark upgrades. It was assumed that 10% of the upstream catchment area is impervious and 90% is pervious, as shown in Table 1 below. The impervious allocation includes the upgraded carpark areas in.

| Table 1: Pre- and Post Development catchment areas and peak run-off flow rates | | | | | | |
|--|--|--|--|--|--|--|
| Pre-Development Catchment Assessment | | | | | | |
| Drainage Area (ha) Peak Discharge (m ³ /s) | | | | | | |
| Pervious | 9.0 | 0.909 | | | | |
| Impervious | 1.0 | 0.151 | | | | |
| Total | 10.0 1.059 | | | | | |
| Post-Development Catchment Assessment | | | | | | |
| Post-De | velopment Catchment Assess | ment | | | | |
| Post-De | velopment Catchment Assess Drainage Area (ha) | ment Peak Discharge (m³/s) | | | | |
| Post-De Pervious | velopment Catchment Assess Drainage Area (ha) 9.0 | ment Peak Discharge (m³/s) 1.175 | | | | |
| Post-De Pervious Impervious | velopment Catchment Assess Drainage Area (ha) 9.0 1.0 | ment Peak Discharge (m ³ /s) 1.175 0.181 | | | | |

Table below shows the parameters used in the HEC-HMS hydrological model of the catchment area using a 24-hr duration rainfall event with a SCS Type 1A storm profile and 20% Climate Change factor applied. A hydrological soil group, Class C, was adopted for the HEC-HMS model. The hydrological soil group was inferred based on the parameters set out in the FNDC engineering standards Table 4-3, with a time of concentration of 10 minutes.

| Land use, group C soils | Runoff curve Number | Initial rainfall abstraction (mm) |
|--------------------------|---------------------|-----------------------------------|
| Permeable (Grass/Garden) | 74 | 5 |
| Impervious Pavement Area | 98 | 0 |

Table 2: HEC-HMS model parameters

The pre- and post-development peak runoff rate for a 1% AEP rainfall event amounts to 1.059 m³/s and 1.356 m³/s respectively for the entire catchment area, as shown in the HEC-HMS outputs in the Appendix. Hydraflow Express software was used to calculate the capacity of the existing culvert. The calculation parameters used was for a concrete pipe with an n-value or 0.013, laid at a grade of 0.75%, and flow rates of 1.059 m³/s and 1.356m³/s.

The results illustrate that the existing culvert has a pre-development flow depth of approximately 540mm, and a post-development flow depth of approximately 620mm, which results in an increased flow depth of 80mm. This indicates that the existing culvert will have sufficient capacity during a post-development 1% AEP rainfall event. Analyses results in the Appendix.

We consider that there are no downstream effects on other buildings or property as the flows are shown to be able to be entirely conveyed by the pipe network in the 1% AEP storm event. Therefore, there will be no effect from the increase in impermeable areas on the State Highway. Once flows reach the main river, the 15L/s increase is



negligible compared to the expected overall flows from the river catchment. Accordingly, we consider that no attenuation of new impermeable areas is required.

6 STORMWATER

6.1 Stormwater Analysis

We have utilised HEC-HMS software to analyse both the pre- and post-development scenarios. As stated in section 5 above, stormwater attenuation for the increase in impervious areas is not a requirement and therefore only stormwater treatment devices will be considered to minimise any increased adverse effects on the downstream wetland.

6.2 Analysis Parameters

The pre-development rainfall depths used in the stormwater analysis have been taken from the historical data from NIWA HIRDS V4. For post-development the rainfall has been increased by 20% to take climate change into account.

| Rainfall depths (mm): | | | | | | | | | |
|-----------------------|-------|------|------|------|------|------|------|------|------|
| ARI | AEP | 10m | 20m | 30m | 1h | 2h | 6h | 12h | 24h |
| 1.58 | 0.633 | 8.71 | 13.4 | 16.9 | 24.5 | 34.4 | 54.8 | 70.4 | 87.4 |
| 2 | 0.5 | 9.54 | 14.6 | 18.5 | 26.9 | 37.7 | 60.2 | 77.3 | 96 |
| 5 | 0.2 | 12.4 | 19 | 24.1 | 35 | 49.2 | 78.7 | 101 | 126 |
| 10 | 0.1 | 14.5 | 22.3 | 28.2 | 41.1 | 57.8 | 92.5 | 119 | 148 |
| 20 | 0.05 | 16.7 | 25.7 | 32.5 | 47.3 | 66.7 | 107 | 138 | 172 |
| 30 | 0.033 | 18 | 27.7 | 35.1 | 51.1 | 72 | 116 | 149 | 186 |
| 40 | 0.025 | 18.9 | 29.1 | 36.9 | 53.8 | 75.9 | 122 | 157 | 196 |
| 50 | 0.02 | 19.6 | 30.3 | 38.4 | 56 | 78.9 | 127 | 163 | 204 |
| 60 | 0.017 | 20.3 | 31.2 | 39.5 | 57.7 | 81.4 | 131 | 169 | 210 |
| 80 | 0.013 | 21.2 | 32.7 | 41.4 | 60.5 | 85.3 | 137 | 177 | 221 |
| 100 | 0.01 | 22 | 33.8 | 42.9 | 62.7 | 88.4 | 142 | 184 | 229 |
| 250 | 0.004 | 25 | 38.6 | 49 | 71.6 | 101 | 163 | 210 | 263 |

Table 3: NIWA HIRDS V4 Rainfall Data

The pre- and post-development rainfall parameters used in HEC-HMS is shown in the table below.

| Table 4: Rainfall Parameters | | | | | | |
|------------------------------|-------------------------------|---------------------------|--|--|--|--|
| Rainfall Event | Pre-development Rainfall (mm) | Post-development Rainfall | | | | |
| 50% AEP 24-hour | 96 | 115.2 | | | | |
| 20% AEP 24-hour | 126 | 151.2 | | | | |
| 1% AEP 24-hour | 229 | 274.8 | | | | |

The table below shows the parameters used in the HEC-HMS hydrological model of the site. A hydrological soil group, Class C, was adopted for the HEC-HMS model. The hydrological soil group was inferred based on the parameters set out in the FNDC ES Table 4-3, with a time of concentration of 10 minutes.



| Table 5: HEC-HMS model parameters | | | |
|-----------------------------------|---|---------------------------------------|--|
| Land use, group C soils | Runoff curve Number Initial rainfall abstraction (mm) | | |
| | | , , , , , , , , , , , , , , , , , , , | |
| | | | |
| Permeable (Grass/Garden) | 74 | 5 | |
| Impervious Pavement Area | 98 | 0 | |

A summary of pervious and impervious areas for the pre- and post- development scenarios relevant to the modelling are shown below.

|--|

| Description | Pre-development | Post-development |
|---------------------|--------------------|--------------------|
| Pervious Area | 1240m ² | 110m ² |
| Impervious Area | 1860m ² | 2990m ² |
| Total Analyzed Area | 3100m ² | |

6.3 Pre-development Peak Runoff

Using the parameters above and 24-hr duration rainfall event with a SCS Type 1A storm profile, the peak discharges are calculated as follows.

Table 7: Pre-development peak runoff flow rates for the 50%, 20% and 1% AEP rainfall events

| 50% AEP (L/s) | 20% AEP (L/s) | 1% AEP (L/s) |
|---------------|---------------|--------------|
| 14.8 | 20.3 | 40.6 |

6.4 Post-development Peak Runoff

Using the parameters above for the development of new carpark, a 24-hr duration rainfall event with 20% increase allowance for climate change and SCS Type 1A storm profile, the peak discharges are calculated as follows.

|--|

| 50% AEP (L/s) | 20% AEP (L/s) | 1% AEP (L/s) |
|---------------|---------------|--------------|
| 22.9 | 30.3 | 55.7 |

6.5 Analysis Results

Table below shows a summary of the pre- and post-development peak flow rates and difference generated by the proposed development. The full output tables from the HEC-HMS modelling are attached in Appendix A.



| Table 9: Peak runoff flow rates for the 50%, 20% and 1% AEP rainfall events | | | |
|---|-----------------------|------------------------|----------------|
| AEP Rainfall Event | Pre-development (L/s) | Post-development (L/s) | Increase (L/s) |
| 50% AEP | 14.8 | 22.9 | 8.1 |
| 20% AEP | 20.3 | 30.3 | 10.0 |
| 1% AEP | 40.6 | 55.7 | 15.1 |

The post-development flow results shown, are then input into the capacity assessment of the existing Ø900mm concrete culvert outlet from the site to the wetland. The results show that the culvert will be 77% full in the 1% AEP event and has sufficient capacity to convey the peak flows from the proposed development.

If impermeable areas greater than those analysed in this design are proposed, then a revision of the design outlined in this report will be required and we should be contacted.

6.6 Stormwater Treatment Devices Recommendation

6.6.1 Sheet Flows from State Highway – New Traffic Island

Some of the stormwater runoff from the state highway, will be captured and pass through a new traffic island constructed to serve as delineation between the school carpark and the turning lane for vehicles traveling from the west and entering the carpark.

It is proposed that the island be constructed with 150mm high block kerbs, with 0.5m wide gaps formed at 3.0m spacings. This will allow runoff to sheet flow into the island area. The Island is intended to be filled with Ø100-150mm aggregate at a depth of 300mm. Underneath this a 300mm wide x 500mm min deep subsoil drain trench will be constructed. The trench will be filled with clean drainage metal, wrapped in geotextile with a Ø110mm perforated drain coil. At both ends of the island a Ø675mm x 1.0m deep chamber with a scruffy dome will be installed and serve as a connection point for the drain coil. The chamber will be connected to the proposed stormwater system.

This will serve as a collection device for the runoff from the state highway. See the appended Engineering Drawings for details.

6.6.2 Stormwater Catchpits

To serve as a gross pollutant trap, the Stormwater 360 LittaTrap is to be installed in all catchpits for the collection of litter, leaves, debris, and other sediment from the carpark areas. Refer to the operation manual appended to this report.



6.6.3 Stormwater Filtration System

It is intended that stormwater runoff from the carpark will sheet flow towards low points and be collected via catchpits. From here all run-off will flow via a pipe network and it will be connected to the existing outlet structure, before discharging downstream into the wetland.

A Stormwater 360 Stormfilter is a cartridge filtration system that will be installed upstream of the connection point to the existing outlet structure. This passive filtration system consists of a precast concrete manhole with cartridges installed to provide treatment of the 'first flush' through filtration media to effectively remove contaminants generated by the movement of vehicles through the carpark. Refer to the supplier information and LDE engineering drawings appended to this report.

7 COMPLIANCE ASSESSMENT

The Northland Regional Council's Proposed Regional Plan for Northland; Dated February 2024, has rules in relation to wetlands. Section C.4.1.9 Land drainage and flood control general conditions, and Section C.6.4.2 Other stormwater discharges, has been assessed to determine if it is a permitted activity.

The design intent for the proposed works was to introduce treatment devices to improve the existing conditions, with a less than minor increase in adverse effects. The treatment devices selected provides a reasonable amount of treatment to mitigate the effects on the receiving environment.

"General conditions apply to activities when referred to in the rules of C.4.1 Land drainage and floor control."

| Section C.4.1.9 Land drainage and flood control general conditions | | |
|--|---|---|
| 1 | There are no adverse flooding, erosion or over-drainage effects on other property. | Refer to Planning Assessment and/or Ecological Memo |
| 2 | The activity does not alter the course of a lake or continually or intermittently flowing river. | Refer to Planning Assessment and/or Ecological Memo |
| 3 | New land drainage does not occur within 50 metres of any natural wetland. | Refer to Planning Assessment and/or Ecological Memo |
| 4 | Drainage does not cause any change to the seasonal or annual range in water level of a natural wetland to an extent that may adversely affect the wetland's natural ecosystem. | Refer to Planning Assessment and/or Ecological Memo |
| 5 | No vegetation, soil or other debris generated from the activity is placed in a position where it may be carried into a river or natural wetland, lake or the coastal marine area. | Refer to Planning Assessment and/or Ecological Memo |



| 6 | There is no damage to a flood defence or any other authorised structure. | n/a |
|----|--|--|
| 7 | Fish passage is maintained, unless an existing authorisation provides otherwise, or temporary works to enable repair and replacement works are being carried out. | Refer to Planning Assessment and/or Ecological Memo |
| 8 | Eels, fish (other than pest fish), kõura (freshwater crayfish) and kākahi (freshwater mussels) unintentionally removed during mechanical clearing of drainage channels are returned to the drainage channel as soon as practicable, but no later than one hour after their removal. | Refer to Planning Assessment and/or Ecological Memo |
| 9 | Refuelling of machinery does not take place in the bed of a river or lake. | n/a |
| 10 | Any discharge of drainage water does not contain concentrations of contaminants which have or are likely to have significant adverse effects on aquatic life in any river, wetland, or the coastal marine area. | Refer to Planning Assessment and/or Ecological Memo |
| 11 | The discharge to the water body or coastal marine area does not, beyond the zone of reasonable mixing: | |
| a) | result in any conspicuous oil or grease films, scums or foams, or floatable or suspended material except where caused by natural events in the receiving water, and | LittaTraps shall be installed in all catchpits, and each catchpit shall have a 300mm min sump which will remove gross pollutants from stormwater flows. A Stormfilter device shall be installed within the proposed carpark. The stormfilter is effective at removing 80% TSS as well as oil and grease which meets water quality requirements. The stormfilter operates like a siphon through engineered media, so these pollutants will remain trapped inside the treatment chamber. The chamber will be subject to maintenance at intervals as directed by the manufacturer. Therefore, with the treatment train provided, we consider that there will be no increase in contaminants discharged from the proposed carpark construction. |
| b) | cause the pH of the receiving water to fall outside the range of 6.5 to 9.0 (except where caused by natural events, or when natural background levels fall outside that range), and | The site is being used as a school carpark, and is not expected to have any significant impact on runoff PH levels. |
| c) | cause any emission of objectionable odour in the receiving water, and | The site use being a school carpark is not expected to have any contaminants which would cause objectionable odour of stormwater runoff. |
| d) | cause any conspicuous change in colour or visual clarity of the receiving water, and | The carpark is proposed to be sealed reducing the likelihood of suspended sediments. All runoff from the proposed carpark is treated prior to discharge to further remove suspended solids so we consider there to be no change to the colour or clarity of stormwater runoff from the subject area. |



| e) | cause the natural temperature of the receiving water body to be changed by more than three degrees Celsius, except in an Outstanding Freshwater Body where it must not be changed by more than one degree Celsius, and | It is not anticipated that the proposed works will have an effect on the temperature of the receiving water due to the short time of concentration being less than 10 minutes. |
|----|--|--|
| f) | cause contamination which may render freshwater taken from a mapped priority drinking water abstraction point (refer I Maps Ngā mahere matawhenua) unsuitable for human consumption after existing treatment. | According to NRC Proposed Regional Plan Maps, the closest drinking water abstraction point is located approximately 6km "as the crow flies" from the area of proposed works. This point is also located inland with a radius of approximately 600m to the closest body of water. See Figure below. |
| 12 | Any discharge of sediment associated with repair and maintenance activities does not occur for more than five consecutive days and must not occur for more than 12 hours on any one day. | n/a |
| 13 | Where in-river works involve bed disturbance from mechanical vegetation clearance or sediment removal: | |
| a) | if undertaken between 1 August and 31 December, a visual inspection of the works area must be undertaken, immediately prior to in-river work starting. If a shoal of whitebait is present, no in-river works shall be undertaken until the shoal passes; and | n/a |
| b) | the works shall not occur more than once in any area between 1 August and 31 December of any year. | n/a |
| 14 | Riverbank disturbance is limited to one side of the waterway, at any one time. | Refer to Planning Assessment and/or Ecological Memo |
| 15 | When mechanically clearing aquatic vegetation, a weed bucket shall be used with a curved flat base and a slatted back. | n/a |
| 16 | The activity does not take place in an īnanga spawning site between 1 March and 30 September. | Refer to Planning Assessment and/or Ecological Memo |





Figure 9: Subject Site and Nearest Drinking Water Abstraction Point (NRC Proposed Regional Plan Maps)

"The diversion and discharge of stormwater into water of onto or into land where it may enter water from an impervious area or by way of a stormwater collection system, is a permitted activity, provided:"

| | Section C.6.4.2 Other stormwater discharges | | |
|----|--|---|--|
| 1 | the discharge or diversion is not from: | | |
| a) | a public stormwater network, or | n/a | |
| b) | a high-risk industrial or trade premises, and | n/a | |
| 2 | the diversion and discharge does not cause or increase flooding of land on another property in a storm event of up to and including a 10 percent annual exceedance probability, or flooding of buildings on another property in a storm event of up to and including a one percent annual exceedance probability, and | Refer to Planning Assessment and/or Ecological Memo | |
| 3 | where the diversion or discharge is from a hazardous substance storage or handling area: | | |
| a) | the stormwater collection system is designed and operated to prevent hazardous substances stored or used on the site from entering the stormwater system, or | n/a | |



| b) | there is a secondary containment system in place to intercept any spillage of hazardous substances and either discharges that spillage to a trade waste system or stores it for removal and treatment, or | n/a |
|----|---|---|
| c) | if the stormwater contains oil contaminants, the stormwater is passed through a stormwater treatment system designed in accordance with the Environmental Guidelines for Water Discharges from Petroleum Industry Sites in New Zealand (Ministry for the Environment, 1998) prior to discharge, and | n/a |
| 4 | where the diversion or discharge is from an industrial or trade premises: | |
| a) | the stormwater collection system is designed and operated to prevent any contaminants stored or used on the site, other than those already controlled by condition 3) above, from entering stormwater unless the stormwater is discharged through a stormwater treatment system, and | n/a |
| b) | any process water or liquid waste stream on the site is bunded, or otherwise contained, within an area of sufficient capacity to provide secondary containment equivalent to 100 percent of the quantity of any process water or liquid waste that has the potential to spill into a stormwater collection system, in order to prevent trade waste entering the stormwater collection system, and | n/a |
| 5 | the diversion or discharge is not into potentially contaminated land, or onto potentially contaminated land that is not covered by an impervious area, and | The discharge is to the existing road culvert which then discharges to the watercourse. No change to the flow regime is proposed as part of this development. |
| 6 | the diversion and discharge does not cause permanent scouring or erosion of the bed of a water body at the point of discharge, and | The proposed works will slightly increase the pre- development flows by 15l/s during a 1% AEP storm event, with increases of 8.1l/s and 10l/s for the 50%- and 20% AEP events. It is not anticipated that this will cause scouring or erosion to the receiving bed of water as the existing culvert is shown to be capable of discharging these flows as existing so there will be no increase in risk of erosion and scour. |
| 7 | the discharge does not contain more than 15 milligrams per litre of total petroleum hydrocarbons, and | High volumes of hydrocarbons are not expected from the carpark area. Any hydrocarbons that are collected in stormwater runoff will generally be trapped in the stormfilter device due to its syphonic function. Information received from the supplier of the Stormfilter Treatment device, states that the device can achieve greater than 80% TSS removal and meet water quality requirements. It is therefore anticipated that the Stormfilter, in conjunction with other treatment methods, will provide sufficient treatment of the run-off from the existing and proposed work. |



| 8 | the discharge does not cause any of the following effects in the receiving waters beyond the zone of reasonable mixing: | |
|----|--|--|
| a) | the production of conspicuous oil or grease films, scums or foams, of floatable or suspended materials, or | LittaTraps shall be installed in all catchpits, and each catchpit shall have a 300mm min sump which will remove gross pollutants from stormwater flows. A Stormfilter device shall be installed within the proposed carpark. The stormfilter is effective at removing 80% TSS as well as oil and grease which meets water quality requirements. The stormfilter operates like a siphon through engineered media, so these pollutants will remain trapped inside the treatment chamber. The chamber will be subject to maintenance at intervals as directed by the manufacturer. Therefore, with the treatment train provided, we consider that there will be no increase in contaminants discharged from the proposed carpark construction. |
| b) | a conspicuous change in the colour or visual clarity, or | The carpark is proposed to be sealed reducing the likelihood of suspended sediments. All runoff from the proposed carpark is treated prior to discharge to further remove suspended solids so we consider there to be no change to the colour or clarity of stormwater runoff from the subject area. |
| c) | an emission of objectionable odour, or | The site use being a school carpark is not expected to have any contaminants which would cause objectionable odour of stormwater runoff. |
| d) | the rendering of freshwater unsuitable for consumption by farm animals, or | Run-off from the existing and proposed impervious areas will be collected in catchpits with a 300mm min sump, from where it will be treated by the Stormfilter device, prior to discharging into an existing structure. From this structure the run-off will discharge into a wetland where further treatment will take place, before discharging into the Oruaiti River, from where farm animals would have access to the freshwater for consumption. The proposed works have been designed to mitigate this and enhance the existing conditions. |
| e) | the rendering of freshwater taken from a mapped priority drinking water abstraction point (refer I Maps Ngā mahere matawhenua) unsuitable for human consumption after existing treatment. | According to NRC Proposed Regional Plan Maps, the closest drinking water abstraction point is located approximately 6km "as the crow flies" from the area of proposed works. This point is also located inland with a radius of approximately 600m to the closest body of water. |



8 **LIMITATIONS**

This report should be read and reproduced in its entirety including the limitations to understand the context of the opinions and recommendations given.

This report has been prepared exclusively for Oruaiti School Board of Trustees c/o Avail Pacific in accordance with the brief given to us or the agreed scope and they will be deemed the exclusive owner on full and final payment of the invoice. Information, opinions, and recommendations contained within this report can only be used for the purposes with which it was intended. LDE accepts no liability or responsibility whatsoever for any use or reliance on the report by any party other than the owner or parties working for or on behalf of the owner, such as local authorities, and for purposes beyond those for which it was intended.

This report was prepared in general accordance with current standards, codes, and best practice at the time of this report. These may be subject to change.



APPENDIX A

HEC-HMS MODEL AND RESULTS







PRE-DEVELOPMENT CATCHMENT ASSESSMENT

| Proj | ect: 21102 - Carpark Sto | rmwater Simulation | Run: Catchment 100-Ye | ear Pre |
|-----------------------|--|--|--|-----------------------|
| Sta Enc Cor | rt of Run: 01Jan2000, l of Run: 02Jan2000, npute Time:01May2024, | 00:00 Basin Mo 00:00 Meteoro 16:22:06 Control \$ | odel: Catchme logic Model: 100-Year Specifications:Control | nt Pre Pre |
| Show Elements: Al | l Elements 🖂 Volum | ne Units: 🔿 MM 🛛 10 | 000 M3 Sorting: V | Vatershed Explorer $$ |
| Hydrologic Element | Drainage Area (KM2) | Peak Discharge (M3/S) | Time of Peak | Volume (1000 M3) |
| Grass Area | 0.0900 | 0.9085 | 1 January 2000, 08 | 13.3053 |
| Impervious | 0.0100 | 0.1507 | 1 January 2000, 07 | 2.2330 |
| Outlet | 0.1000 | 1.0587 | 1 January 2000, 08 | 15.5382 |

EXISTING CULVERT PRE-DEVELOPMENT CAPACITY ASSESSMENT

| Section Type = Circular Btm Width (m) -0- Side Slope, z:1 = -0- Diameter(m) = 0.9000 | |
|--|--|
| Btm Width (m) -0- Side Slope, z:1 = -0- Diameter(m) = 0.9000 | |
| Side Slope, z:1 = -0- Diameter(m) = 0.9000 | |
| Diameter(m) = 0.9000 | |
| | |
| Inv Elev(m) = 5.0000 | |
| Slope (%) = 0.7500 | |
| n-value = 0.013 | |
| Compute by = Known Q | |
| Q (cms) = 1.0587 | |
| Run | |




POST-DEVELOPMENT CATCHMENT ASSESSMENT

| Ρ | roject | ect: 21102 - Carpark Stormwater | | | Simulation R | ear Post | | |
|--|--------|---------------------------------|--------------|------------------------------------|-------------------------------------|------------------|----------|-----------------------|
| Start of Run: 01Jan2000, 00:00 End of Run: 02Jan2000, 00:00 Compute Time:01May2024, 16:22:06 | | | | Basin Mo Meteorolo Control S | del: ogic Model: pecification | nt +CC r Post | | |
| Show Elements: | All e | elements 🗸 | Volum | e Units: 🔇 |) MM 🔾 10 | 00 M3 | Sorting: | Watershed Explorer $$ |
| Hydrologic Element | | Drainage (KM | e Area 2) | Peak I (№ | Discharge 13/S) | Time | of Peak | Volume (1000 M3) |
| Grass Area | | 0.09 | 00 | 1. | 1753 | 1 January | 2000, 08 | . 17.0019 |
| Impervious | | 0.01 | 00 | 0. | 1810 | 1 January | 2000, 07 | . 2.6895 |
| Outlet | | 0.10 | 00 | 1. | 3557 | 1 January | 2000, 08 | . 19.6914 |

EXISTING CULVERT POST-DEVELOPMENT CAPACITY ASSESSMENT

| Section Type = | Circular |
|-------------------|----------|
| Btm Width (m) | -0- |
| Side Slope, z:1 = | -0- |
| Diameter(m) = | 0.9000 |
| Inv Elev(m) = | 5.0000 |
| Slope (%) = | 0.7500 |
| n-value = | 0.013 |
| Compute by = | Known Q |
| Q (cms) = | 1.3560 |
| | Run |
| | |
| | |
| | |
| | |
| | |





PRE-DEVELOPMENT PEAK RUN-OFF - 50% AEP

| | Project: 21102 - Carpark Stormwater Simulation Run: 2-Year Pre | | | | | | |
|-----------------------|--|--|-----------------------|------|--|---------------------|---|
| | Start of Run: () End of Run: () Compute Time:() | art of Run: 01Jan2000, 00:00 d of Run: 02Jan2000, 00:00 mpute Time:01May2024, 16:22:09 | | | Model: rologic Moo ol Specificat | ev ar Pre rol | |
| Show Elements: A | All Elements \vee | Volum | e Units: 🔿 MM | 0 10 | 00 M3 | Sorting: | Watershed Explorer $\ \!$ |
| Hydrologic Element | Drainage (KM | e Area 2) | Peak Discha (M3/S) | arge | Time | of Peak | Volume (1000 M3) |
| Grass | 0.00 | 12 | 0.0031 | | 1 January | 2000, 08 | 0.0498 |
| Gravel | 0.00 | 08 | 0.0050 | | 1 January | 2000, 07 | 0.0727 |
| Parking Asphalt | 0.00 | 05 | 0.0029 | | 1 January | 2000, 07 | 0.0427 |
| Existing Road Asph | alt 0.00 | 05 | 0.0029 | | 1 January | 2000, 07 | 0.0427 |
| Concrete | 0.00 | 01 | 0.0007 | | 1 January | 2000, 07 | 0.0109 |
| Underground Tanks | s 0.00 | 31 | 0.0148 | | 1 January | 2000, 07 | 0.2608 |
| Outlet | 0.00 | 31 | 0.0148 | | 1 January | 2000, 07 | 0.2608 |

PRE-DEVELOPMENT PEAK RUN-OFF – 20% AEP

| | Project: 21102 · | Carpark Stormwate | Simulation Ru | n: 5-Year Pre | |
|-----------------------|---|--|--|---------------|-----------------------|
| S E C | Start of Run: 01Ja End of Run: 02Ja Compute Time:01Ma | n2000, 00:00 n2000, 00:00 ay2024, 16:22:11 | 0:00 Basin Model: PreDev 0:00 Meteorologic Model: 5-Year Pre 16:22:11 Control Specifications:Control | | |
| Show Elements: A | ll Elements 🖂 | /olume Units: 🔿 MM | 1 O 1000 M3 | Sorting: \ | Watershed Explorer $$ |
| Hydrologic Element | Drainage Are (KM2) | ea Peak Disch (M3/S) | arge Tim | e of Peak | Volume (1000 M3) |
| Grass | 0.0012 | 0.0049 | 1 Janua | y 2000, 08 | 0.0769 |
| Gravel | 0.0008 | 0.0066 | i Janua | y 2000, 07 | 0.0965 |
| Parking Asphalt | 0.0005 | 0.0039 | 1 Janua | y 2000, 07 | 0.0567 |
| Existing Road Aspha | alt 0.0005 | 0.0039 |) 1 Janua | y 2000, 07 | 0.0567 |
| Concrete | 0.0001 | 0.0010 |) 1 Janua | y 2000, 07 | 0.0145 |
| Underground Tanks | 0.0031 | 0.0203 | 1 Janua | y 2000, 08 | 0.3355 |
| Outlet | 0.0031 | 0.0203 | 1 Janua | y 2000, 08 | 0.3355 |

PRE-DEVELOPMENT PEAK RUN-OFF – 1% AEP

Project: 21102 - Carpark Stormwater Simulation Run: 100-Year Pre

| S E C | Start of Run: 0 End of Run: 0 Compute Time:0 |)1Jan2000,)2Jan2000,)1May2024 | 00:00 00:00 , 16:22:07 | Basin M Meteoro Control | lodel: blogic Mode Specificatic | PreDev I: 100-Yea | ar Pre |
|-----------------------|--|---------------------------------------|------------------------------|-------------------------------|---------------------------------------|----------------------|----------------------|
| Show Elements: | All Elements 🖂 | Volum | e Units: 🔿 MM | 1 🔾 10 | 000 M3 | Sorting: V | Vatershed Explorer 🖂 |
| Hydrologic Element | Drainag (KN | ge Area 42) | Peak Disch (M3/S | narge) | Time | of Peak | Volume (1000 M3) |
| Grass | 0.0 | 012 | 0.012 | 5 | 1 January | 2000, 08 | 0.1833 |
| Gravel | 0.0 | 008 | 0.012 | 1 | 1 January | 2000, 07 | 0.1786 |
| Parking Asphalt | 0.0 | 005 | 0.007 | 1 | 1 January | 2000, 07 | 0.1049 |
| Existing Road Asph | nalt 0.0 | 005 | 0.007 | 1 | 1 January | 2000, 07 | 0.1049 |
| Concrete | 0.0 | 001 | 0.001 | 3 | 1 January | 2000, 07 | 0.0268 |
| Underground Tank | s 0.0 | 031 | 0.040 | 5 | 1 January | 2000, 07 | 0.6198 |
| Outlet | 0.0 | 031 | 0.040 | 5 | 1 January | 2000, 07 | 0.6198 |



POST-DEVELOPMENT PEAK RUN-OFF - 50% AEP

| | Project: 21102 - Carpa | lation Run: 2-Year Post | : | | | |
|-----------------------|--|--|--------------------|-----------------------|--|--|
| St Er Co | art of Run: 01Jan2000 nd of Run: 02Jan2000 ompute Time:01May2024 | t of Run: 01Jan2000, 00:00 Basin Model: PostDev of Run: 02Jan2000, 00:00 Meteorologic Model: 2-Year Po ipute Time:01May2024, 16:22:09 Control Specifications:Control | | | | |
| Show Elements: Al | Elements Volum | e Units: 🔿 MM 🝳 10 | 000 M3 Sorting: \ | Vatershed Explorer $$ | | |
| Hydrologic Element | Drainage Area (KM2) | Peak Discharge (M3/S) | Time of Peak | Volume (1000 M3) | | |
| Asphalt Carpark | 0.0020 | 0.0150 | 1 January 2000, 07 | 0.2198 | | |
| Existing Road Aspha | lt 0.0005 | 0.0035 | 1 January 2000, 07 | 0.0517 | | |
| Gravel | 0.0003 | 0.0024 | 1 January 2000, 07 | 0.0352 | | |
| Asphalt Walkway | 0.0002 | 0.0015 | 1 January 2000, 07 | 0.0220 | | |
| Grass | 0.0001 | 0.0004 | 1 January 2000, 08 | 0.0059 | | |
| Underground Tanks | 0.0031 | 0.0229 | 1 January 2000, 07 | 0.3579 | | |
| Outlet | 0.0031 | 0.0229 | 1 January 2000, 07 | 0.3579 | | |

POST-DEVELOPMENT PEAK RUN-OFF – 20% AEP

| | Project: 21102 - Carpa | ark Stormwater Simu | lation Run: 5-Year Pos | t | | |
|-----------------------|---|--------------------------|------------------------|-----------------------|--|--|
| S E C | Start of Run:01Jan2000, 00:00Basin Model:PostDevEnd of Run:02Jan2000, 00:00Meteorologic Model:5-Year PostCompute Time:01May2024, 16:22:10Control Specifications:Control | | | | | |
| Show Elements: A | Il Elements 🖂 Volum | e Units: 🔿 MM 🗿 10 | 000 M3 Sorting: | Watershed Explorer $$ | | |
| Hydrologic Element | Drainage Area (KM2) | Peak Discharge (M3/S) | Time of Peak | Volume (1000 M3) | | |
| Asphalt Carpark | 0.0020 | 0.0198 | 1 January 2000, 07 | . 0.2915 | | |
| Existing Road Aspha | lt 0.0005 | 0.0047 | 1 January 2000, 07 | . 0.0685 | | |
| Gravel | 0.0003 | 0.0032 | 1 January 2000, 07 | . 0.0466 | | |
| Asphalt Walkway | 0.0002 | 0.0020 | 1 January 2000, 07 | . 0.0292 | | |
| Grass | 0.0001 | 0.0006 | 1 January 2000, 08 | . 0.0090 | | |
| Underground Tanks | 0.0031 | 0.0303 | 1 January 2000, 07 | . 0.4641 | | |
| Outlet | 0.0031 | 0.0303 | 1 January 2000, 07 | . 0.4641 | | |

POST-DEVELOPMENT PEAK RUN-OFF – 1% AEP

Project: 21102 - Carpark Stormwater Simulation Run: 100-Year Post

| Start End o Comp | of Run: 01Jan2000, of Run: 02Jan2000, oute Time:01May2024, | 00:00 Basin Meteoro 00:00 Meteoro 16:22:06 Control | odel: PostDev logic Model: 100-Yea Specifications:Control | ir Post | |
|------------------------|--|--|---|-----------------------|--|
| Show Elements: All E | Elements Volum | e Units: 🔵 MM 🗿 10 | 00 M3 Sorting: V | Watershed Explorer $$ | |
| Hydrologic Element | Drainage Area (KM2) | Peak Discharge (M3/S) | Time of Peak | Volume (1000 M3) | |
| Asphalt Carpark | 0.0020 | 0.0362 | 1 January 2000, 07 | 0.5379 | |
| Existing Road Asphalt | 0.0005 | 0.0085 | 1 January 2000, 07 | 0.1264 | |
| Gravel | 0.0003 | 0.0058 | 1 January 2000, 07 | 0.0861 | |
| Asphalt Walkway | 0.0002 | 0.0036 | 1 January 2000, 07 | 0.0538 | |
| Grass | 0.0001 | 0.0014 | 1 January 2000, 08 | 0.0208 | |
| Underground Tanks | 0.0031 | 0.0557 | 1 January 2000, 07 | 0.8406 | |
| Outlet | 0.0031 | 0.0557 | 1 January 2000, 07 | 0.8406 | |



APPENDIX B

ENGINEERING DRAWINGS





Project Number:

Project Office: Project Manager: Issue Date:

Drawing Status: Resource Consent

Proposed Carpark Upgrades Oruaiti School Board of Trustees c/o Avail Pacific 2554 State Highway 10, Mangonui

| Drawing Index | | | | | | | | |
|---------------|--|------------------|----------|--|--|--|--|--|
| Sheet | Description | Status | Revision | | | | | |
| C001 | Existing Site Plan | Resource Consent | С | | | | | |
| C002 | Demolition and Reinstatement Plan | Resource Consent | В | | | | | |
| C100 | Construction Staging Layout | Resource Consent | В | | | | | |
| C200 | Earthworks Layout | Resource Consent | С | | | | | |
| C300 | Overall Site Layout | Resource Consent | D | | | | | |
| C301 | Parking Area & Accessway Design Plan Sheet 1 of 3 | Resource Consent | D | | | | | |
| C302 | Parking Area & Accessway Design Plan Sheet 2 of 3 | Resource Consent | D | | | | | |
| C303 | Parking Area & Accessway Design Plan Sheet 3 of 3 | Resource Consent | D | | | | | |
| C304 | Signage Layout | Resource Consent | А | | | | | |
| C305 | Concrete Saw-cuts Layout | Resource Consent | В | | | | | |
| C310 | Parking Area & Accessway Longitudinal Sections | Resource Consent | С | | | | | |
| C311 | Parking Area & Accessway Typical Cross Sections | Resource Consent | С | | | | | |
| C312 | Parking Area & Accessway 5m Interval Cross Sections Sheet 1 of 3 | Resource Consent | С | | | | | |
| C313 | Parking Area & Accessway 5m Interval Cross Sections Sheet 2 of 3 | Resource Consent | С | | | | | |
| C314 | Parking Area & Accessway 5m Interval Cross Sections Sheet 3 of 3 | Resource Consent | С | | | | | |
| C320 | Parking Area & Accessway Typical Details Sheet 1 of 2 | Resource Consent | D | | | | | |
| C321 | Parking Area & Accessway Typical Details Sheet 2 of 2 | Resource Consent | В | | | | | |
| C400 | Parking Area & Accessway Stormwater Management Plan | Resource Consent | С | | | | | |
| C410 | Parking Area & Accessway Stormwater Longitudinal Sections Sheet 1 of 2 | Resource Consent | С | | | | | |
| C411 | Parking Area & Accessway Stormwater Longitudinal Sections Sheet 2 of 2 | Resource Consent | С | | | | | |
| C420 | Parking Area & Accessway Typical Stormwater Device Details | Resource Consent | С | | | | | |
| C421 | Parking Area & Accessway Typical Stormwater Details | Resource Consent | С | | | | | |



21102

Warkworth Riaan de Swardt 1 April 2025



General Notes:

Library

Pool

- 1. All dimensions in metres unless noted otherwise.
- Coordinates are in terms of NZGD Mt Eden 2000. 2.
- All levels are in metres to NZVD2016. Origin of levels 3. RCM XIII SO 50961 RL 11.81m.
- 4. Survey undertaken by Donaldsons; 25 July 2022.
- Existing titles Pt Allot 18A PSH OF Mangonui East 5
- Boundaries and are approximate only and are subject 6. to change. Final boundaries will be in accordance with city plan and LINZ requirements. Boundaries not for building design purposes.
- Existing building, roading, and services information may have been obtained from third party records and should be verified on site prior to the commencement of any works. LDE does not in any way guarantee the accuracy of any underground service shown on this plan.
- Contractor to locate / mark / confirm and protect if 9. necessary all existing services prior to commencement of work on site. Any discrepancies to be reported to the Engineer.
- 10. Flood extent extracted from NRC Natural Hazards and shown indicatively for information only.

General Legend:

— — Boundary (Surveyed) Major Contour Minor Contour

NRC River Flood Hazard Zone - Regionwide Models

| - | _ | | |
|-------|----------|---|--|
| | <u> </u> | | |
| _ | _ | _ | |
| | | | |

10 Year Exent 50 Year Extent 100 Year CC Extent

Existing Services Legend:

| SW SW |
|-------|
| 8 E |
| S ⊂ |
| |
| |
| P P P |
| онон |
| T |
| F0 F0 |
| |
| |

Edge of Road Asphalt Surface Stormwater Pipe Stormwater Manhole, Wingwall, Scruffy Dome, Catchpit Open Drain Water Pipe Wastewater Pipe Underground Power Lines Power Pole & Over Head Lines Transformer Fibre Optic Lines Top of Bank Bottom of Bank Fence / Gate Retaining Wall

| 21 | | | | | 10 | 15 | 20 | 23 |
|----------|-----------------------|----------------------------------|-----------|--------------|-----------------|--------------|--------------|-----------|
| | | | | Scale 1:500 | (m) | | Original siz | e = A3 |
| | | | Design: | R. de Swardt | Project status: | ource | Conse | nt |
| | | | Drawn: | R. de Swardt | Project: | <u>ouroo</u> | | |
| | N. D | 28 Mar 2025 | Approved: | N. Douglas | Drawing No: | 211 | 102 | Issue/Rev |
| Signage) | N. D J. S Aprvd | 5 Sep 2024 1 May 2024 Date | Scale A3: | 1:500 | | C001 | 1 | С |

- This plan should be read in conjunction with the proposed plans included in the Detailed Design drawing set.
- proposed plans for details.
- Engineering Standards and the Health and Safety at Work Act 2015 (HSWA).
- protect if necessary all existing services prior to commencement of work on site.
- All disturbed public areas to be made good and restored to existing condition.
- All existing vegetation in the proposed works area to be removed and re-used.
- reinstatement works. Any excess topsoil to be used elsewhere on site as directed by the School's representatives.





Staging Notes:

57

- Contractor's Site Office shown indicatively. Exact area to be confirmed prior to construction.
 The contractor is fully responsible for Health and Safety measures during the construction phase.
 - Appropriate fencing, signage, etc. to be installed prior top the start of works. Contractor is to allow for all safety fencing for the
- during of the proposed works. The Staging Areas is to be confirmed prior to be commencement of construction.
- Contractor to obtain and maintain all traffic management requirements as per NZTA standards.

| | | | | | | 10 | 0 15 20 | | | |
|------------|--------------|------------------------------------|-----------|---------|--------|-----------------|---------|--------------|-----------|--|
| | | | | Scale 1 | :500 | (m) | | Original siz | e = A3 | |
| | | | Design: | R. de S | Swardt | Project status: | ource | Conse | nt | |
| | | | Drawn: | R. de S | Swardt | Project | | | /// | |
| | | | Approved: | N. Dou | uglas | Drawing No: | 211 | 02 | lesua/Rav | |
| ce Consent | N. D N. D | 01 Apr 2025 28 Mar 2025 Date | Scale A3: | 1:50 | 00 | Drawing No. | C100 |) | B | |
| 101011 | 740110 | Buto | | | | | | | | |

engineers • scientists



General Notes:

- All dimensions in metres unless noted otherwise.
- Coordinates are in terms of NZGD Mt Eden 2000. 2.
- All levels are in metres to NZVD2016. Origin of levels 3. RCM XIII SO 50961 RL 11.81m.
- Survey undertaken by Donaldsons; 25 July 2022. 4.
- Existing titles Pt Allot 18A PSH OF Mangonui East 5
- Boundaries and are approximate only and are subject 6. to change. Final boundaries will be in accordance with city plan and LINZ requirements. Boundaries not for building design purposes.
- Existing building, roading, and services information may have been obtained from third party records and 8. should be verified on site prior to the commencement of any works. LDE does not in any way guarantee the accuracy of any underground service shown on this plan.
- Contractor to locate / mark / confirm and protect if 9. necessary all existing services prior to commencement of work on site. Any discrepancies to be reported to the Engineer.
- 10. Flood extent extracted from NRC Natural Hazards and shown indicatively for information only.

General Legend: Earthworks Notes:

- All works to be undertaken in accordance with FNDC Engineering Standards, GD05 Guidelines and the Health and Safety at Work Act 2015 (HSWA).
- All disturbed public areas to be made good and restored to previous condition.
- All earthworks to be undertaken during periods of fine weather.
- Earthworks are to be undertaken and completed in within the minimum possible time frame.
- The area is to be progressively finished as areas are brought to grade.
- The earthworks volumes shown is a solid measure.
- The earthworks volumes has been calculated in relation to the existing and final ground levels.
- Topsoil removal volumes is excluded in the total earthworks volumes.
- Additional box cut volumes has not been included in the earthworks volumes.

Erosion and Sediment Control Notes:

- The contractor is solely responsible for ensuring the the site has effective erosion control and sediment retention measures operating at all times.
- All erosion and sediment control measures must be operational prior to any other works commencing on site
- All sediment control measure are to comply with GD05 and FNDC requirements.

| Earthworks Cut / Fill Volumes | | | | | | | | |
|-------------------------------|----------------------|--|--|--|--|--|--|--|
| Site Area | 41,210m ² | | | | | | | |
| Earthworks Area | 2,760m² | | | | | | | |
| Cut Volume | 20m³ | | | | | | | |
| Fill Volume | 460m ³ | | | | | | | |
| Net Fill Volume | 440m ³ | | | | | | | |

| | | | | 0 5 | 10 | 15 | 20 | 25 |
|--------------|--------------|--------------------------|-----------|--------------|------------------------|-------|--------------|-----------|
| | | | | Scale 1:500 | (m) | | Original siz | e = A3 |
| | | | Design: | R. de Swardt | Project status: Res | ource | Conse | ent |
| | | | Drawn: | R. de Swardt | Project: | 044 | 00 | |
| | N. D | 28 Mar 2025 | Approved: | N. Douglas | Drawing No: | 211 | 02 | Issue/Rev |
| a + Signage) | N. D H. D | 5 Sep 2024 1 May 2024 | Scale A3: | 1:500 | | C200 | | С |



Proposed Services Notes:

- All works to be undertaken in accordance with FNDC Engineering Standards and the Health and Safety at Work Act 2015 (HSWA).
- 2. All dimensions in metres unless noted otherwise.
- Contractor to locate / ml dimensionark / confirm and protect if necessary all existing services prior to commencement of work on site.
- . All disturbed public areas to be made good and restored to existing condition.
- Manholes to be reinforced concrete chambers with a precast base. Min Ø1050mm manhole unless otherwise specified.
- 6. Catchpits to be 675 x 450mm reinforced concrete chambers, with trafficable frame & grid inlet lid.
- All PVC stormwater pipes to be uPVC SN16.
- 3. All concrete stormwater pipes to be RCRRJ Class 4.
- 9. Pipe bedding and installation to comply with relevant council details.
- All lane and parking markings as per MOTSAM.
 Parking bays as per Appendix 3D of the Far North
- District Plan (FNDP).
- 12. All kerbing to be machine extruded, unless specified otherwise by Engineer.

Reference Notes:

- 13. Refer to C301,C302, C303 for Parking Area Layouts.
- 14. Refer to C310 for Parking Area and Accessway Longitudinal Sections Layout.
- 15. Refer to C311 to C314 for Cross Section Layouts.
- 16. Refer to C320 & C321 for Roading details.
- 17. Refer to C400 for Stormwater Layout.
- 18. Refer to C410 to C411 Stormwater Longitudinal Section Layouts.
- 19. Refer to C420 & C421 for Stormwater details.

Existing Services Legend:



| | | | Design: | R. de Swardt | Project status: Resource Consent |
|---------------|--------------|--------------------------|-----------|--------------|-------------------------------------|
| | | | Drawn: | R. de Swardt | Project: |
| ce Consent | N. D | 01 Apr 2025 | Approved: | N. Douglas | 21102 |
| ea + Signage) | N. D H. D | 5 Sep 2024 1 May 2024 | Scale A3: | 1:500 | C300 Drawing No: |
| /ision | Aprvd | Date | | | |

20





| Proposed Services Notes: | Existing Services Legend: | Proposed Services Legend: |
|---|--|---|
| Proposed Services Notes: All works to be undertaken in accordance with FNDC Engineering Standards and the Health and Safety at Work Act 2015 (HSWA). All dimensions in metres unless noted otherwise. Contractor to locate / ml dimensionark / confirm and protect if necessary all existing services prior to commencement of work on site. All disturbed public areas to be made good and restored to existing condition. Manholes to be reinforced concrete chambers with a precast base. Min Ø1050mm manhole unless otherwise specified. Catchpits to be 675 x 450mm reinforced concrete chambers, with trafficable frame & grid inlet lid. All lane and parking markings as per MOTSAM. | Existing Services Legend: — — Boundary (Surveyed) — — Edge of Road Sw Stormwater Pipe Image: Stormwater Manhole, Wingwall, Scruffy Dome, Catchpit > >>>> Open Drain Image: Proper Proper Pole & Over Head Lines Image: Proper Proper Pole & Over Head Lines Image: Proper Proper Pole & Over Head Lines Image: Proper Proper Pole Bank Image: Proper Pole Bank | Proposed Services Legend: Prop. Kerb Block Prop. Kerb & Channel Prop. Kerb & Channel Prop. Flush Edge Beam Standard Concrete Surface Exposed Aggregate Concrete Gravel Surface Rock Filled Traffic Island Grassed Area Image: P SW Perforated Pipe & Rodding Point Image: SW Manhole SW Catchpit SW Swale |
| Parking bays as per Appendix 3D of the FNDP. All kerbing to be machine extruded, unless specified otherwise by Engineer. | | →→→→ Fence |

Copyright: LDE All rights reserved / Do not scale off drawings / Confirm all dimensions on site prior to work Client

Oruaiti School Board of Trustees c/o Avail Pacific

2554 State Highway 10, Mangonui Proposed Carpark Upgrades

Project

Drawing Title Parking Area & Accessway Design Plan Sheet 2 of 3



| | | | | 0 2 | 5 | 5 | 7.5 | 10 | 12.5 |
|---------------|------------|----------------------------|-----------|---------|--------|-----------------|------------------------|--------------------------------|-------------------|
| | | | | Scale 1 | :250 | (m) | | Original siz | ze = A3 |
| | | | Design: | R. de S | Swardt | Project status: | ource | - Conse | -nt |
| | | | Drawn: | R. de S | Swardt | Project | | | 2111 |
| ce Consent | N. D | 01 Apr 2025 28 Mar 2025 | Approved: | N. Do | uglas | | 21 | 102 | |
| ea + Signage) | N.D H.D | 5 Sep 2024 1 May 2024 | Scale A3: | 1:2 | 50 | Drawing No: | C30 | 2 | D D |
| lision | Aprva | Date | | | | | Drive Location C., San | ed by RaanDeSwardt, Document E | 455509, Version 3 |

| | | Proposed Services N All works to be FNDC Engineerin Safety at Work Ac All dimensions in Contractor to loc and protect if neo to commencemen All disturbed pub restored to existin Manholes to be re a precast base. otherwise specifie Catchpits to be 6 chambers, with tra All lane and parkii Parking bays as p All kerbing to specified otherwise | otes: undertaken in accordance with g Standards and the Health and t 2015 (HSWA). metres unless noted otherwise. ate / ml dimensionark / confirm tessary all existing services prior t of work on site. lic areas to be made good and g condition. einforced concrete chambers with Min Ø1050mm manhole unless id. 75 x 450mm reinforced concrete afficable frame & grid inlet lid. mg markings as per MOTSAM. er Appendix 3D of the FNDP. be machine extruded, unless ie by Engineer. | Existing Service | es Legend: Boundary (Surveyed) Edge of Road Stormwater Pipe Stormwater Manhole, Wingwall, Scruffy Dome, Catchpit Open Drain Water Pipe Underground Power Lines Power Pole & Over Head Lines Transformer Fibre Optic Lines Top of Bank Bottom of Bank Fence / Gate Retaining Wall | Proposed Services Legen Prop. Ker Prop. Ker Prop. Flux Standard Exposed Gravel St Rock Fille SW Perfor SW Manh SW Catcl SW Swall O Fence | a: b Block b & Channel c & C |
|---|---|---|--|------------------|---|--|--|
| | | | | Road Marking L | <u>egend:</u> White Zebra Crossing 2.0m Long x 0,3m Wide, 0.6m Ga White Stop Sign White Bus Sign White Straight Arrow | Road Marking Leger 1650 Yell | <u>d:</u> Jmm Rubber Wheel Stop ow Disabled Parking Sign ow No-Parking Island |
| | 6 3.6 Handrails on both sic Nosing. Refer to Det | taircase including de and Contrast ail on C321 | N | | White Left-Turn Arrow White Parking Delineation Line 100mm Wide Yellow No-Stop Lines 1.0m Long x 1.0m Gap | | |
| | | Existing Post boxes to be protected and relocated to location confirmed by client Existing Asphalt Area to be Stripped & Replaced with Topsoil & Seeded with Grass | | | | | |
| A ST | | | < | | | | |
| | SH10 | Ø150 SED Timber Bollards @ 2m c/c 1m High and Concreted 1m into ground m Hotmix Transition between g asphalt and proposed concrete | | | | 0 25 | 5 7 5 10 12 5 |
| | | | | | | | (m) |
| Copyright: LDE All rights reserved / Do not scale off drawings / Confirm all dimensions on site prior to work Client Oruaiti School Board of Trustees c/o Avail Pacific | Project 2554 State Highway 10, Mangonui Proposed Carpark Upgrades | Drawing Title Parking Area & Accessway Design Plan Sheet 3 of 3 | | | Issued for Resource Consent N. D 281 Revision (Entrance Grassed Area + Signage) N. D 5 S | Design: R. de Swardt Drawn: R. de Swardt Mar 2025 Approved: N. Douglas | Project status: Project status: Project status: Project Projeco Project Project Projec |
| | | | engineers • scienti: | STS A I | Issued for Consent H. D 1 M Issue/Revision Aprvd | ay 2024 Scale A3: 1:250 Date | |





| | - | Upgraded Carpark Upgraded Accessway | | | | | | | Existin Access | ng way | ty Boundary 🖳 | | |
|-------------------------|--------------------------|-------------------------------------|-----------|-------|-------|-------|-------|-------|-------------------|-----------|--|------|----------|
| | | | | | | | | | | | Existing Fence | | <u> </u> |
| Vertical E Datum: (| Exaggeration: 2.5 0.0 | | | | | | | | | | Vertical Exaggeration: 2.5 Datum: 0.0 | | |
| Desigr | n Levels | 4.95 | 5.17 | 5.31 | 5.37 | 5.36 | 5.27 | 5.18 | | | Design Levels | | 4.29 |
| <u>Exis</u> ti <u>n</u> | ng Levels | 5.02 | 5.16 1 | 5.26 | 5.43 | 5.32 | 5.22 | 5.08 | | | Existing Levels | 4.32 | 4.21 |
| Cut/Fil | ll Depth | -0.08 | 0.01 | 0.05 | -0.06 | 0.03 | 0.05 | 0.10 | | | Cut/Fill Depth | | 0.07 |
| Chaina | age | 5.00 | 00.01 | 15.00 | 20.00 | 25.00 | 30.00 | 35.00 | | | Chainage | 5.00 | 10.00 |

Long Section (ACCESSWAY CL)

Long Section (PARKING AREA CL) Chainage 0m to 25m

| | | | | | | | | | | | | | | | | | | F |
|---------------------------------------|------|------|----------|--------------|----------|------|------|------|------|------|------|------|------|-------|-------|------|------|---|
| | | · · | <u> </u> | | <u> </u> | | | - | | | | | ~ | | | | | |
| | | | | | | | / | | | | | | | | | | | |
| | | | | | | | | | | | | | | | | | | |
| | | | | | | | | | | | | | | | | | | |
| | | | | | | | | | | | | | | | | | | |
| | | | | | | | | | | | | | | | | | | |
| Vertical Exaggeration: 2.5 Datum: 0.0 | | | | | | | | | | | | | | | | | | |
| | 4 | 0 | ~ | | | N | 0 | | | m | ~ | 10 | 0 | m | 10 | 0 | (0) | |
| Design Levels | 4.3 | 4.3 | 4.3 | 4 | 4 .2 | 4 2 | 4.2 | 4.2 | 4.4 | 4.1 | 4.2 | 4.2 | 4.4 | 4.4 | 4.7; | 4.9 | 5.16 | |
| Existing Levels | .26 | 19 | .17 | , | .07 | 80 | .97 | 76. | .01 | .02 | .01 | 56. | 28 | .57 | 62. | 98 | .92 | |
| | 4 | 4 | 4 | 4 | 4 | m | ю | Ϋ́ | 4 | 4 | 4 | ო | 4 | 4 | 4 | 4 | 4 | |
| Cut/Fill Depth | 0.08 | 0.13 | 0.14 | 0.18 | 0.20 | 0.24 | 0.25 | 0.24 | 0.18 | 0.16 | 0.19 | 0.28 | 0.12 | -0.14 | -0.04 | 0.01 | 0.23 | |
| Chainage | 00 | 00 | 00. | E | 8 | 00. | 00 | 00. | 0 | 00. | 00. | 00 | 00. | 00. | 00. | 00.(| 00.2 | |
| | 25 | 30 | 35 | 40 | 45 | 20 | 55 | 60 | 65 | 20 | 75 | 80 | 85 | 6 | 95 | 100 | 105 | |

Long Section (PARKING AREA CL) Chainage 25m to End

Copyright: LDE All rights reserved / Do not scale off drawings / Confirm all dimensions on site prior to work Client





| | | | | | |
|-----|------|-----|------|-----|--|
| | | | | | |
| | | | | | |
| | | | | | |
| | | | | | |
| | | | | | |
| | | | | | |
| | | | | | |
| | | | | | |
| | | | | | |
| 7 | | 9 | | 4 | |
| 4.3 | | 4.3 | | 4 | |
| 24 | | 25 | | g | |
| 4.2 | | 4.2 | | 4 | |
| 13 | | 11 | | 8 | |
| 0.1 | | ò. | | 0.0 | |
| 00 | | 8 | | 8 | |
| 15. | | 20. | | 25. | |

| | | | | | Original size = A3 |
|---------------|--------------|---------------------------|-----------|---------------|-------------------------------------|
| | | | Design: | R. de Swardt | Project status: Resource Consent |
| | | | Drawn: | R de Swardt | Preiost |
| | | | Biami | rt. do ondidi | 21102 |
| | | | Approved: | N. Douglas | 21102 |
| ea + Signage) | N. D N. D | 28 Mar 2025 5 Sep 2024 | | | Drawing No: Issue/Rev |
| 5 57 | H. D | 1 May 2024 | Scale A3: | 1:250 | C310 C |
| vision | Aprvd | Date | | | |



| Design: R. de Swardt Project status: Resource Cons | ent |
|---|-----------|
| Drawn: R de Swardt | |
| Project O1400 | |
| Approved: N Develop 21102 | |
| N. D 28 Mar 2025 Approved. N. Douglas | Issue/Rev |
| ea + Signage) N. D 5 Sep 2024 | 0 |
| H. D 1 May 2024 Scale A3: 1:75 C31 | I C |
| ision Aprvd Date | |











Copyright: LDE All rights reserved / Do not scale off drawings / Confirm all dimensions on site prior to work Client

Oruaiti School Board of Trustees c/o Avail Pacific

Project 2554 State Highway 10, Mangonui Proposed Carpark Upgrades Drawing Title Parking Area & Accessway 5m Inteval Cross Sections Sheet 1 of 3





| | | | | | Original size | e = A3 |
|----------------|----------------------|--------------------------|-----------|--------------|-----------------------------------|----------|
| | | | Design: | R. de Swardt | Project status: Resource Conse | nt |
| | | | Drawn: | R. de Swardt | Project: | |
| | ND | 29 Mar 2025 | Approved: | N. Douglas | 21102 | |
| rea + Signage) | N. D N. D H. D | 5 Sep 2024 1 May 2024 | Scale A3: | 1:250 | C312 | csue/Rev |
| vision | Aprvd | Date | - | | | <u> </u> |







7.5

5

12.5

15

10





Copyright: LDE All rights reserved / Do not scale off drawings / Confirm all dimensions on site prior to work Client

Oruaiti School Board of Trustees c/o Avail Pacific

Project 2554 State Highway 10, Mangonui Proposed Carpark Upgrades Drawing Title Parking Area & Accessway 5m Inteval Cross Sections Sheet 2 of 3





| | | | | | Original | size = A3 |
|--------------|-------|-------------|-----------|--------------|-----------------|------------|
| | | | Desian: | R. de Swardt | Project status: | |
| | | | | | Resource Cons | sent |
| | | | Drawn: | R. de Swardt | Project | |
| | | | | | 21102 | |
| | ND | 28 Mar 2025 | Approved: | N. Douglas | 21102 | |
| a + Signage) | N. D | 5 Sep 2024 | | | Drawing No: | Issue/Rev: |
| | H. D | 1 May 2024 | Scale A3: | 1:250 | I C313 | |
| sion | Aprvd | Date | - | | | - |
| | | | | | | |
| | | | | | | |













C Issued for Resource Consent B Revision (Entrance Grassed Area + Signage) A Issued for Consent No. IssuedRevision







Copyright: LDE All rights reserved / Do not scale off drawings / Confirm all dimensions on site prior to work Client

Proiect



R. de Swardt Design: Resource Consent R. de Swardt Drawn: 21102 N. Douglas Approved: N. D B Revised and Issued for Resource Conse B B C321 Scale A3: As Shown

Original size = A3





Long Section (SWCP1 TO SWMH3)





Long Section (SWCP7 TO SWMH3)

Copyright: LDE All rights reserved / Do not scale off drawings / Confirm all dimensions on site prior to work Client

Oruaiti School Board of Trustees c/o Avail Pacific

Project 2554 State Highway 10, Mangonui Proposed Carpark Upgrades Drawing Title Parking Area & Accessway Stormwater Longitudinal Sections Sheet 1 of 2



ssued for Resource C

| | | | | | Original siz | e = A3 |
|---------------|-----------------------|----------------------------------|-----------|--------------|-----------------------------------|---|
| | _ | | Design: | R. de Swardt | Project status: Resource Conse | nt |
| | | | Drawn: | R. de Swardt | Project: | ,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,, |
| | N. D | 28 Mar 2025 | Approved: | N. Douglas | 21102 | Issue/Rev |
| ea + Signage) | N. D H. D Aprvd | 5 Sep 2024 1 May 2024 Date | Scale A3: | 1:250 | C410 | С |



Long Section (SPORTS FIELD CATCHPIT TO TANK)



Copyright: LDE All rights reserved / Do not scale off drawings / Confirm all dimensions on site prior to work Client

Oruaiti School Board of Trustees c/o Avail Pacific

Project 2554 State Highway 10, Mangonui Proposed Carpark Upgrades

Drawing Title Parking Area & Accessway Stormwater Longitudinal Sections Sheet 2 of 2





| | | | | | Original size = A3 |
|---------------|--------------|--------------------------|-----------|--------------|--|
| | | | Design: | R. de Swardt | Project status: Resource Consent |
| | | | Drawn: | R. de Swardt | Project. |
| | ND | 28 Mar 2025 | Approved: | N. Douglas | 21102 |
| ea + Signage) | N. D H. D | 5 Sep 2024 1 May 2024 | Scale A3: | 1:250 | C411 C |
| /ision | Aprva | Date | | | Drive Location C., Saved by ReanDeSwardt, Document 42 455509, Venion |





Sheet 39 Standard Precast Manhole 200 for pipes up to 300 dia. B 600 for pipes over 300 dia Opening to be over outlet for sanitan sewer only. Short no longer 2 than 1000mm for uP.V.C. Ceramic joints in half B -pipe channels to be filled with epoxy mortar SECTION CC NOTES: 1. This detail is applicable for pipe diameters up to 600mm & for manhole depths up to 5.0m 2. 150mm thick reinforced concrete lids with heavy duty ductile iron frames & covers to be used in driveways, carriageways & berms. 100mm thick concrete lids with light duty cast iron frames & covers may be used elsewhere. 3. Precast manhole bases shall be used in all instances with minimum sized holes cut for pipe entry. 4. No additional thin plastering of benching or benching of inverts is permitted. 5. All concrete to be 20MPa. 7. Where non-concrete pipe connections are made to concrete manholes, then a gritted starter pipe shall be installed to allow bond between manhole and pipe material. Also, a 3flexible joint should be specified as part of gritted starter pipe (as standard) 8. All manholes >1.2m in depth shall be provided with manhole step rungs. These shall follow the requirements on sheet 40 London junction Epoxy mortar around pipe Short - no longer than 1000mm Dropper pipe for sewer only (single piece) -Where the height of drop exceeds 1.5m the dropper pipe shall be rigidly attached to the wall of the manhole Mortar around pipe with approved Grade 316 SS or plastic supports at 1.5m centres max. -88° bend set in benching 50mm layer of – 20MPa concrete -Half pipe channel (ceramic or uPVC) SECTION A-A INTERNAL DROP Note 1. This detail is applicable for pipe diameters up to 250mm & for manhole depth up to 5.0m & for manhole diameters ≥ 1200mm. 2. External drops shall not be used STANDARD PRECAST MANHOLE sewer and stormwater FOR ALL ENVIRONMENTS FAR NORTH DISTRICT COUNCIL

FAR NORTH DISTRICT ENGINEERING STANDARDS

engineers • scientists

| Client |
|--|
| Oruaiti School Board of Trustees c/o Avail Pacific |

2554 State Highway 10, Mangonui Proposed Carpark Upgrades Parking Area & Accessway Typical Stormwater Details



| | | | Design: | R. de Swardt | Project status: Resource Consent |
|----------------|-------|-------------|-----------|--------------|-------------------------------------|
| | | | | | |
| | | | Drawn: | R. de Swardt | Project: |
| | | | | | 01100 |
| | | | A | N. Davialaa | 21102 |
| | N. D | 28 Mar 2025 | Approved: | N. Douglas | Denvice No. |
| rea + Signage) | N. D | 5 Sep 2024 | | | Drawing No: Issuencev |
| | H. D | 1 May 2024 | Scale A3: | As Shown | L C421 LC |
| vision | Aprvd | Date | | | • • • • • |

APPENDIX C

SUPPLIER INFORMATION AND TECHNICAL DRAWINGS





Low-cost catchpit insert for removal of litter and gross pollutants from run-off

Litter, leaves and other gross pollutants such as cigarette butts wash into our storm drains daily, and are responsible for degradation of our streams, rivers, lakes and beaches. Stormwater runoff from existing drainage network largely flows untreated into our marine environment and negatively effects the water quality, land value and marine life. LittaTrap is a low-cost solution to remove these pollutants.

The LittaTrap is an award-winning catchpit insert designed to be easily retrofitted into new and existing stormwater catchpits to specifically target litter, leaves and gross pollutants, capturing them at source and preventing them from accumulating in our waterways.

The LittaTrap is very cost effective and easy to maintain. Simply lift the filterbag out of the catchpit, empty and then replace back into the frame. This efficient solution captures and removes plastics and other pollutants before they reach the stormwater outfall.

The LittaTrap is also effective as a pre-treatment device for use in a treatment train with hydrodynamic separators, filtration devices, ponds and wetlands. In many cases, it is often the most practical solution for retrofits.



HOW DOES IT WORK?

As stormwater enters a storm grate or catchpit, it passes through the filterbag. Litter, debris, and other pollutants larger than the filterbag aperture are captured and retained.

If the filterbag is full or during high flows, the overflow is released through the overflow apertures in the frame assembly.

DESIGN AND OPERATION

The LittaTrap is fixed by brackets to the catchpit precast concrete wall below the inlet grate allowing the filter insert to be easily lifted out by hand for maintenance. Consisting of a structural bracket assembly with flow diverter seal and a removable filterbag insert which allows for high flow bypass, the LittaTrap is simple to install and remove.

Once the filter insert has been removed, open access to the base of the catchpit and outlet pipe means any accumulated sediment in the pit can easily be removed by inductor truck if required.

Standard sizes fit the common precast concrete regular and curb entry catchpits. Custom designs may be fabricated for non-standard catchpits.

CAPABILITIES

- Captures litter, leaves, debris and gross pollutants before they enter the drainage system
- Fits a range of catchpit sizes ideal for retrofits
- Easy access maintenance friendly design, can be hand maintained
- Bypasses high flows with no moveable parts
- Adjustable flow diverter allows fine-tuning during installation for a perfect fit
- Third party tested at Auckland University

BENEFITS

- Can be used to target high litter areas
- No construction means low capital costs
- Ideal pre-treatment device for filters, ponds, wetlands or other stormwater management and WSD practices
- A range of filterbag sizes to target gross pollutants
- Hand maintainable
- No confined space entry



EASY MAINTENANCE

The LittaTrap is very cost-effective and easy to maintain as the filterbag can be simply picked up and emptied by hand. As the LittaTrap is hand maintainable, it can be easily cleaned by site owners or maintenance crew without the need for vacuum inductor trucks or heavy lifting equipment. The pits can be visually inspected and accessed from grade, so there is no requirement for confined space entry, reducing maintenance costs.

LittaTrap filters should be inspected periodically to determine maintenance frequency. The frequency of maintenance services should be reviewed at the completion of each clean and modified if pollutant loadings deem this necessary. At the required maintenance interval the contaminants need to be removed from the filterbags and disposed of appropriately.

The maintenance crew is responsible for the disposal of debris in accordance with all applicable regulations and is responsible for following all Health and Safety requirements.

Contact the Stormwater360 maintenance department at **maintenance@Stormwater360.co.nz** for more information.

HAND MAINTENANCE IS AS EASY AS 'LIFT, TIP, REPLACE'

- 1. Lift LittaTrap out of pit using handles
- 3. Tip contents out of LittaTrap into suitable receptacle for contents
- 2. Replace LittaTrap securely back into surrounding frame and seal









Nothing in this brochure should be construed as an expressed warranty or an implied warranty of merchantability or fitness for any particular purpose. See the Stormwater360 standard quotation or acknowledgement for applicable warranties and other terms and conditions of sale.

STORMFILTER^M

High efficiency / low maintenance stormwater filter.

SIPHON-ACTUATED FILTRATION The stormwater management StormFilter[™] cleans stormwater through a patented passive filtration system, effectively removing pollutants to meet the most stringent regulatory requirements. Highly reliable, easy to install and maintain, and proven performance over time, StormFilter[™] products are recognised as a versatile BMP for removing a variety of pollutants, such as sediments, oil and grease, metals, organics, and nutrients. These systems come in variable configurations to match local conditions and come with prolonged maintenance periods to ensure long-term performance and reduce operating costs.

HOW DOES IT WORK?

During a storm, runoff passes through the filtration media and starts filling the cartridge center tube. Air below the hood is purged through a one-way check valve as the water rises. When water reaches the top of the float, buoyant forces pull the float free and allow filtered water to drain.

After the storm, the water level in the structure starts falling. A hanging water column remains under the cartridge hood until the water level reaches the scrubbing regulators. Air then rushes through the regulators releasing water and creating air bubbles that agitate the surface of the filter media, causing accumulated sediment to drop to the vault floor. This patented surfacecleaning mechanism helps restore the filter's permeability between storm events.

PROVEN PERFORMANCE

- First independently verified filter by Washington Department of Ecology, New Jersey Department of Environmental Protection and USEPA's Environmental Technology Verification program
- Approved and accepted by most regulatory bodies across New Zealand
- Over 16,800 x StormFilter's[™] cartridges installed throughout New Zealand
- Continual innovation Design refined and perfected over two decades of research and experience



STORMFILTER[™] VAULT



STORMFILTER BENEFITS

UNDERGROUND SYSTEMS MAXIMISE PROFITABILITY

- Save land space allowing denser developments reducing sprawl
- Add parking spaces and increase
 building size, increasing profitability
- Compact design reduces construction and installation costs by limiting excavation

RELIABLE LONGEVITY & LOWER MAINTENANCE COSTS

- Self cleaning hood prevents surface blinding, ensures use of all media and prolongs cartridge life
- 1-3 year maintenance cycles
- 20 years maintenance experience
- Minimal or no standing water. Lower disposal costs

CONTACT DETAILS

Stormwater360

FREEPHONE: 0800 STORMWATER (0800 786769)

www.stormwater360.co.nz



Nothing in this brochure should be construed as an expressed warranty or an implied warranty of merchantability or fitness for any particular purpose. See the Stormwater360 standard quotation or acknowledgement for applicable warranties and other terms and conditions of sale.





SUPERIOR HYDRAULICS

Multiple cartridge heights gives design solutions for site restraints.



Other hydraulic benefits

- Hydraulic effect as low as 350mm head loss
- Zero surcharge of inlet pipe unlike upward flowing filters
- Can be designed for tail water e.g tidal conditions
- Online and offline configurations can limit hydraulic effects

MEDIA CHOICES

Our filtration products can be customised using different filter media to target site-specific pollutants. A combination of media is often recommended to maximise pollutant removal effectiveness.



Perlite is naturally occurring puffed volcanic ash. Effective for removing TSS, oil and grease.



ZPG[™] is a multi-purpose media option approved for highly trafficked sites or sites with high metal loadings. ZPG is a mixture of Zeolite, Perlite and GAC (granular activated carbon). ZPG is ideal for removing soluble metals, TSS, oils and grease, organics and ammonium.



Zeolite is a naturally occurring mineral used to remove soluble metals, ammonium and some organics.



GAC (Granular Activated Carbon) has a micro-porous structure with an extensive surface area to provide high levels of adsorption. It is primarily used to remove oil and grease and organics such as PAHs and phthalates.

CONFIGURATION

Stormfilter's can be configured in any drainage structure. Please contact SW360 for a customised design.



PRECAST VAULT

- Treats medium sized sites
- Simple installation arrives on-site fully assembled
- Plug and play solution

PRECAST MANHOLE

- Provides a low drop, point-of-entry configuration
- Uses drop from the curb inlet to the conveyance pipe to drive the passive filtration cartridges
- No crane required (Hi-AB lifting for most sizes)
- 1050-2400mm diameter sizes available



- Combines off-line bypass and StormFilter[™] pollutant removal into one structure
- Internal weir allows high peak flows with low hydraulic head losses
- Approved and accepted as offline configuration by councils around NZ
- Eliminates cost of additional structures to bypass peak flows
- Reduces the overall footprint of the treatment system



0800 STC





HIGH FLOW

- Treats flows from large sites
- Consists of large, precast components designed for easy assembly on-site
- Several configurations available, including: Panel Vault, Box Culvert, or Cast-In-Place



SCALE : NTS

| BY CUSTOMER / CONTRACTOR | | | | | | | | |
|--------------------------|----------------------|-----------------------------|-----------------------------|-----------------------------|--|--|--|--|
| | P.O. NUMBER : | | | | | | | |
| | | | | | | | | |
| | | | | | | | | |
| | | | | | | | | |
| | | | | | | | | |
|) 0 RRJ) | Ø 350 (DN300 PVC) | Ø 400 (DN300 RRJ) | Ø 500 (DN375 RRJ) | Ø 600 (DN450 RRJ) | | | | |
| T PIPE | (IL) : | | DTI : | | | | | |
|): | | | DATE : | | | | | |
| | | | | | | | | |

TO BE COMPLETED BY SW360

| | ZPG | OTHER : | | |
|---------------------|---------------------------------|---------------------------|---------|--|
| | | PRE-INSTALLATION (Y/N) : | | |
|)) RD | 3 QTR (Ø 24.0 ID) WHITE/OPAL | HALF (Ø 19.7 ID) GREEN | OTHER : | |
| WEB-FORGE / CLASS D | | OTHER : | | |
| | | DATE : | | |
| | | | | |

NOTES

- MANHOLE UNIT FITTED WITH SWIFTLIFT ANCHOR POINTS (QTY 2). DO NOT EXCEED 60 DEGREE LIFT ANGLE. CONCRETE LID FITTED WITH SWIFTLIFT ANCHOR POINTS (QTY 4).
- UNIT SUPPLIED WITH OUTLET CORÉ DRILLED. INLET/S CORE DRILLED ON SITE BY OTHERS WITHIN RANGE SHOWN ON DRAWING. SEALING / GROUTING OF MANHOLE COMPONENTS AND PIPES BY CONTRACTOR. ENSURING LOCAL CODES AND REGULATIONS ARE
- COMPLIED WITH. ANY RISERS REQUIRED TO INCREASE THE DEPTH TO INVERT (DTI) FROM THAT AS DRAWN TO BE SUPPLIED BY THE CONTRACTOR. FOR A DTI EXCEEDING 5m PLEASE CONTACT 0800STORMWATER CONTRACTOR TO TAKE APPROPRIATE MEASURES TO PROTECT CARTRIDGES FROM CONSTRUCTION RELATED EROSION RUNOFF. BACKFILL, BEDDING AND BUOYANCY DESIGN BY ENGINEER OF RECORD
- QTY OF CARTRIDGES BY ENGINEER OF RECORD.
- CONCRETE MANHOLE RISERS ARE DESIGNED AND MANUFACTURED IN ACCORDANCE WITH AS/NZS 4058 : 2007
- 10. CONCRETE MANHOLE BASES ARE DESIGNED AND MANUFACTURED IN ACCORDANCE WITH NZS 3101 : 2006 & NZS 3109 : 1997
- 11. CONCRETE LID DESIGNED AND MANUFACTURED TO HN-HO-72 12. FOR REQUIREMENTS OUTSIDE OF DRAWING SPECIFICATIONS PLEASE
- CONTACT 0800STORMWATER.

CKD :

APPROX WEIGHTS

MANHOLE SECTION INCLUDING CARTRIDGES : 4050 Kg (AS DELIVERED, BASED ON QTY 7 ZPG CARTS) LID WEIGHT : 1850 Kg

| | | | REV | REVISION DETAIL | DATE |
|------|---------------|----------|-----|-----------------|----------|
| | | | 0 | APPROVED | 01.03.19 |
| | | | 1 | MARK-UPS | 09.04.19 |
| R.P. | R.P. 01.03.19 | | | | |
| G.S. | G.Y. | 01.03.19 | | | |



| BY C | BY CUSTOMER / CONTRACTOR | | | | | | | |
|------|--------------------------|---------------------|---------------|---|------|--|--|--|
| | | P.O. NUMBER : | | | | | | |
| | | | | | | | | |
| | | | | | | | | |
| | | | | | | | | |
| | | | | | | | | |
| | CORE DF | RILL Ø : | INLET (IL) : | | | | | |
| | CORE DF | RILL Ø : | OUTLET (IL) : | | | | | |
| | ORIENTA | TION: 180° (AS DRAV | VN) / 90° | / | 135° | | | |
| | SIGNED : | : | DATE : | | | | | |

TO BE COMPLETED BY SW360

| | ZPG | OTHER : | | |
|----------|---------------------------------|---------------------------|---------|--|
| | | PRE-INSTALLATION (Y/N) : | | |
|)) RD | 3 QTR (Ø 24.0 ID) WHITE/OPAL | HALF (Ø 19.7 ID) GREEN | OTHER : | |
| WEB | -FORGE / CLASS D | OTHER : | | |
| | | DATE : | | |

NOTES

- MANHOLE UNIT FITTED WITH 2 SWIFTLIFT ANCHOR POINTS. DO NOT EXCEED 60 DEGREE LIFT ANGLE. CONCRETE LID FITTED WITH 4 SWIFTLIFT ANCHOR POINTS.
- UNIT SUPPLIED WITH INLET & OUTLET CORE DRILLED.
- SEALING / GROUTING OF MANHOLE COMPONENTS AND PIPES BY CONTRACTOR.
- ENSURING LOCAL CODES AND REGULATIONS ARE COMPLIED WITH.
- ANY RISERS REQUIRED TO INCREASE THE DEPTH TO INVERT (DTI) FROM THAT AS DRAWN TO BE SUPPLIED BY THE CONTRACTOR.
- FOR A DTI EXCEEDING 5m PLEASE CONTACT 0800STORMWATER.
- CONTRACTOR TO TAKE APPROPRIATE MEASURES TO PROTECT CARTRIDGES FROM CONSTRUCTION RELATED EROSION RUNOFF.
- BACKFILL, BEDDING AND BUOYANCY DESIGN BY ENGINEER OF RECORD
- QTY OF CARTRIDGES BY ENGINEER OF RECORD.
- CONCRETE MANHOLE RISERS ARE DESIGNED AND MANUFACTURED IN ACCORDANCE
- 10. CONCRETE MANHOLE BASES ARE DESIGNED AND MANUFACTURED IN ACCORDANCE WITH NZS 3101 : 2006 & NZS 3109 : 1997
 - CONCRETE LID DESIGNED AND MANUFACTURED TO HN-HO-72
- 12. FOR REQUIREMENTS OUTSIDE OF DRAWING SPECIFICATIONS PLEASE CONTACT

APPROX WEIGHTS

MANHOLE SECTION INCLUDING CARTRIDGES : 3900 Kg (AS DELIVERED, BASED ON QTY 5 ZPG CARTS) LID WEIGHT : 1800 Kg

| | | REV | REVISION DETAIL | DATE |
|------|----------|-----|------------------------|----------|
| | | А | FOR APPROVAL | 22.05.19 |
| | | 0 | APPROVED | 23.05.19 |
| R.P. | 22.05.19 | | | |
| G.S. | 22.05.19 | | | |

CSF 0054; StormFilter Flow-Based Sizing - Other



| Project Name | Oruaiti School Parking area & Access way | | | Location | Mangonui 2554 State Highway 10, | | |
|--------------|--|----------|--------|----------|---------------------------------|------------|----|
| Job # | 8021 | Device # | SWMH 2 | Option # | Α | Revision # | RA |
| Author | [Choose Designer] | | | Date | 11 April 2024 | | |

| | INSTRUCTIONS | | | | | | | | | | |
|---|--|--|---|--|--|---|--|--|--|--|--------------------------------------|
| 1.0 Water Quality Design Storm Peak Runoff Flowrate (RATIONAL M | 1.0. Use the rational method to compute the water quality design storm peak runoff flow rate. Values with blue text require user input. Values in red text are automatically calculated. Values with black text remain constant. | | | | | | | | | | |
| 1.1 Runoff Co-efficients | | 1.1 Input the appropriate run | noff co-efficient for each | sub-catchment. | | | | | | | |
| Coefficient of Impervious Roof (Croof) | 0.90 | Use C=0.9 for imperious roof surfaces runoff co-efficient from NZBC E1/VM1: Table 1 | | | | | | | | | |
| Coefficient of Impervious Road (Croad) | 0.85 | Use C=0.85 for imperious paved surfaces runoff co-efficient from NZBC E1/VM1: Table 1 | | | | | | | | | |
| Coefficient of Pervious Area (Cper) | 0.25 | Use C=0.25 for pervious grassed/landscaped surfaces runoff co-efficient from NZBC E1/VM1: Table 1 | | | | | | | | | |
| 1.2 Catchment Areas | | | | | | | | | | 1 | |
| Area Impervious Roof (Aroof) | 0 m ² | 1.2 Input the appropriate cat | tchment area for each su | b-catchment. | | | | | | | |
| Area Impervious Road (Aroad) | 3000 m ² | Enter impervious roof surfac | e catchment area | | | | | | | | |
| Area Pervious Area (Aper) | 0 m ² | Enter impervious paved surfa | aces catchment area | | | | | (| | | ~ |
| Area Total Catchment (Acatch) | 3000 m ² | Enter pervious grassed/lands | scaped surfaces catchm | ent area | | | | | | | |
| Product of Area & Coefficients (CA) | 2550 m ² | Total catchment area i.e. Aca | stal catchment area /e. Acatch = Aroof + Aroad + Aper | | | | | | | | |
| 1.3 Rainfall Intensity | | Product of catchment areas | & runoff co-efficients i.e. | CA = (Croof x Aroof) + (Carof x Aroof) + (Caro | Croad x Aroad) + (Cper x | (Aper) | | | | | |
| Water Quality Rainfall Intensity (iWQ) | 10 mm/hr | | | | | | | ~ | Figu | ure 1: StormFilter Ca | tridge |
| | | 1.3 Input rainfall intensity | | | | | | | | | |
| 1.4 Water Quality Design Storm Peak Runoff F | owrate | I ne sutn percentile raintali in | ntensity is recommended | to be used for calculatin | ng the water quality flow. | wnere no statistical an | alysis of historical storm | n events nas been under | rtaken, we recommne | a iwq=10mm/nr is usea | Alternative values |
| Design Water Quality Treatment Flowrate (Qwg) | 7.083 L/s | can be checked adainst Hiki | is or intensity tables in t | ne local colincii Code of | Practice | | | | | | |
| ; | | 1.4 Compute the water qualit | v design storm peak run | off flow rate via Rational | Method | | | | | | |
| 2.0 StormFilter Peak Treatment Flowrate | | i.e. Q = f.C.i.A | | | | | | | | • • • • | |
| 2.1 Proliminary | | | | | | | | $\Omega - $ | 0 111/ | 12.06 1 10 |).5 |
| 2.1 Freiminary | Porlito | 2.0 Lise the stormfilter stars | -discharge equation to | calculate the StormFilter | neak treatment flowrate | | | Q - q | 0.1114 | $i \Delta n$ | |
| Cartridge Media (Media) | Periite | 2.0. Ose the storminer stage | e-uischarge equation to | calculate the Storini inter | peak treatment nowrate. | | | ~ | | $[O] = L \min^{-1};$ | [d]=mm; [h]=m |
| Cartridge Height (Hcart) | 69 CM | Enter cortridge filtertien | in in Darlite 700 | | | | | | _ | | |
| Diameter Disc Orifice (d) | 27.6 mm | Enter cartridge filtration med | lia i.e. Perlite or ZPG | | | | | | Figure 2: Storn | nFilter Stage Dischar | ge Equation [1] |
| Internal bypass weir height (Hweir) | 0.79 m | Enter cartridge height <i>i.e.</i> 690 | cm / 46cm / 30cm | | | | | | | | |
| Priming depth (Hprime) | 0.66 m | Enter restrictor disc size, refe | er table below for max di | sc diameter | | | | 1 | | | |
| Area of a Cartridge (Acart) | 0.181 m ² | Cart Height (cm) | Actual Height (m) | Priming Depth (m) | Max Disc Diam. (mm) | Max. Design Q (L/s) | Filter Bed Area (m ²) | Flow Rate (L/s/m ²) | Bed Depth (mm) | Media Volume (m ³) | Flow Rate (L/s/m ³) |
| | | 30 | 0.305 | 0.27 | 22.70 | 0.63 | 0.460 | 1.37 | 175 | 0.052 | 12.0 |
| 2.2 StormFilter Cartridge Peak Treatment Flo | wrate | 46 | 0.457 | 0.43 | 25.00 | 0.95 | 0.689 | 1.38 | 175 | 0.078 | 12.1 |
| StormFilter cartridge stage-discharge equation | =0.111d ^{2.06} ∆h ^{0.5} L/min | 69 | 0.686 | 0.66 | 27.60 | 1.42 | 1.034 | 1.37 | 175 | 0.118 | 12.1 |
| Peak treatment flowrate at internal bypass per cartridge (Qcart) | 1.420 L/s/cart | Table 1. StormFilter Cart | ridge Specifics [2] | | | | | | | | |
| Number (actual) of StormFilter cartridges required | 4.988 cart(s) | | | | | | | | | | |
| Number (rounded) of StormFilter cartridges required | 5 cart(s) | Compute the Stormfilter peak | k treatment flowrate at in | ternal bypass per cartrid | lge via the StormFilter st | age-discharge equation | L | | | | |
| Design StormFilter Treatment Flowrate (QSF) | 7.100 L/s | Compute the number of actu | al StormFilter cartridges | required i.e. CEILING(B3 | 35,1) = QWQ / QCART | | | | | | |
| | | Compute the number (round | ed up to whole number) | of StormFilter cartridges | required | | | | | | |
| 3.0 Estimate Sediment Mass Loading | | Compute the Stormfilter peak | k treatment flowrate at in | ternal bypass via the Sto | ormFilter stage-discharge | e equation | | | | | |
| 3.1 StormFilter Manhole/Vault Dimension | s | | | | | ooquuunon | | | | | |
| | | | | | | oquation | | | | | |
| Length Cartridge Bay (Lbay) | 1.000 m | 3.0 Estimate sediment mass | loading (Refer sheet '2. I | lass Load Calcs' for mor | re details) | | | | | | |
| Length Cartridge Bay (Lbay) Width Cartridge Bay (Whay) | 1.000 m 2.440 m | 3.0 Estimate sediment mass 3.1 Use tables 2 & 3 below to | loading (Refer sheet '2. I | Mass Load Calcs' for mor | re details) of cartridges as calculate | ed in cl 2.2 above | | | | | |
| Length Cartridge Bay (Lbay) Width Cartridge Bay (Wbay) Area Cartridge Bay (Abay) | 1.000 m 2.440 m 2.440 m ² | 3.0 Estimate sediment mass 3.1 Use tables 2 & 3 below to 3.2 Use table 3 below to f | loading (Refer sheet '2. I fill in StormFilter Dimen fill in estimated TSS c | Mass Load Calcs' for mor sions based on number o oncentration. For road | re details) of cartridges as calculate ds with ≥25.000∨pd, u | ed in cl 2.2 above use minimum 600kg/k | nalvr | | | | |
| Length Cartridge Bay (Lbay) Width Cartridge Bay (Wbay) Area Cartridge Bay (Abay) Total area of Cartridges (Acarte) | 1.000 m 2.440 m 2.440 m ² 0.905 m ² | 3.0 Estimate sediment mass 3.1 Use tables 2 & 3 below to 3.2 Use table 3 below to f 3.3 Use 0% pretreatment for y | loading (Refer sheet '2. I fill in StormFilter Dimen fill in estimated TSS c yaults/manholes with no | Mass Load Calcs' for mor sions based on number o oncentration. For road forebay, Use 10-15% pre | re details) of cartridges as calculate ds with ≥25,000vpd, u etreatmnet for vaults/mar | ed in cl 2.2 above Ise minimum 600kg/l | na/yr se 50% pretreatment for i | upstream GPT ie Enviro | Pod. Use 75%-90% sv | stem efficiency | |
| Length Cartridge Bay (Lbay) Width Cartridge Bay (Wbay) Area Cartridge Bay (Abay) Total area of Cartridge Sa(Acarts) | 1.000 m 2.440 m 2.440 m ² 0.905 m ² 1.535 m ² | 3.0 Estimate sediment mass 3.1 Use tables 2 & 3 below to 3.2 Use table 3 below to f 3.3 Use 0% pretreatment for | loading (Refer sheet '2. I fill in StormFilter Dimen fill in estimated TSS c vaults/manholes with no | Mass Load Calcs' for mor sions based on number o oncentration. For road forebay. Use 10-15% pre Without | re details) of cartridges as calculate ds with ≥25,000vpd, u treatmnet for vaults/mar t forshav | ed in cl 2.2 above Ise minimum 600kg/h Iholes with forebays. U | na/yr se 50% pretreatment for i | upstream GPT ie Enviro With for | Pod. Use 75%-90% sy | stem efficiency | |
| Length Cartridge Bay (Ubay) Width Cartridge Bay (Wbay) Area Cartridge Bay (Abay) Total area of Cartridges (Acarts) Area Lower Volume (Alows) | 1.000 m 2.440 m 2.440 m ² 0.905 m ² 1.535 m ² | 3.0 Estimate sediment mass 3.1 Use tables 2 & 3 below to 3.2 Use table 3 below to f 3.3 Use 0% pretreatment for Std Manhole Dimensions | loading (Refer sheet '2. I fill in StormFilter Dimen ill in estimated TSS c vaults/manholes with no | Asss Load Calcs' for mor sions based on number o oncentration. For road forebay. Use 10-15% pre Without | re details) of cartridges as calculate ds with ≥25,000vpd, u treatmnet for vaults/mar t forebay | ed in cl 2.2 above ise minimum 600kg/ł nholes with forebays. U | a/yr se 50% pretreatment for i | upstream GPT ie Enviro With for | Pod. Use 75%-90% sy rebay | stem efficiency | |
| Length Cartridge Bay (Lbay) Width Cartridge Bay (Wbay) Area Cartridge Bay (Abay) Total area of Cartridges (Acarts) Area Lower Volume (Alow) Volume Lower Volume (Vlow) | 1.000 m 2.440 m 2.440 m ² 0.905 m ² 1.535 m ² 1059 L | 3.0 Estimate sediment mass 3.1 Use tables 2 & 3 below to 3.2 Use table 3 below to f 3.3 Use 0% pretreatment for v Std Manhole Dimensions | loading (Refer sheet '2. I fill in StormFilter Dimen ill in estimated TSS c vaults/manholes with no Cart Bay Length (m) | Mass Load Calcs' for mor sions based on number of oncentration. For road forebay. Use 10-15% pre Without Cart Bay Width (m) 0.77 | re details) of cartridges as calculate ds with ≥25,000vpd, u treatmnet for vaults/mar forebay Cart Bay Area (m ²) | ed in cl 2.2 above ise minimum 600kg/h nholes with forebays. U: Max Number Carts | na/yr se 50% pretreatment for i Cart Bay Length (m) | upstream GPT ie Enviro With for Cart Bay Width (m) | Pod. Use 75%-90% sy rebay Cart Bay Area (m²) | stem efficiency Max Number Carts | |
| Length Cartridge Bay (Lbay) Width Cartridge Bay (Wbay) Area Cartridge Bay (Abay) Total area of Cartridges (Acarts) Area Lower Volume (Alow) Volume Lower Volume (Vlow) Area Upper Volume (Aupp) | 1.000 m 2.440 m 2.440 m ² 0.905 m ² 1.535 m ² 1.059 L 2.440 m ² | 3.0 Estimate sediment mass 3.1 Use tables 2 & 3 below to 3.2 Use table 3 below to f 3.3 Use 0% pretreatment for Std Manhole Dimensions 1050 | loading (Refer sheet '2. I fill in StormFilter Dimen iill in estimated TSS c vaults/manholes with no Cart Bay Length (m) 1.00 | Aass Load Calcs' for mor sions based on number of oncentration. For road forebay. Use 10-15% pre Without Cart Bay Width (m) 0.77 | re details) of cartridges as calculate ds with ≥25,000vpd, u treatmnet for vaults/mar forebay Cart Bay Area (m ²) 0.77 | ed in cl 2.2 above use minimum 600kg/h nholes with forebays. U: Max Number Carts 1 | na/yr se 50% pretreatment for i Cart Bay Length (m) 1.00 | upstream GPT ie Enviro With for Cart Bay Width (m) N/A | Pod. Use 75%-90% sy ebay Cart Bay Area (m²) N/A | stem efficiency Max Number Carts N/A | |
| Length Cartridge Bay (Lbay) Width Cartridge Bay (Wbay) Area Cartridge Bay (Abay) Total area of Cartridges (Acarts) Area Lower Volume (Alow) Volume Lower Volume (Vlow) Area Upper Volume (Mupp) Volume Upper Volume (Vupp) | 1.000 m 2.440 m 2.440 m ² 0.905 m ² 1.535 m ² 1059 L 2.440 m ² 244 L | 3.0 Estimate sediment mass 3.1 Use tables 2 & 3 below to 3.2 Use table 3 below to f 3.3 Use 0% pretreatment for 1 Std Manhole Dimensions 1050 1200 | loading (Refer sheet '2. 1 fill in StormFilter Dimen iil in estimated TSS c vauits/manholes with no Cart Bay Length (m) 1.00 1.00 | Ass Load Calcs' for mon sions based on number oncentration. For road forebay. Use 10-15% pre Without Cart Bay Width (m) 0.77 1.03 | re details) of cartridges as calculat ds with ≥25,000vpd, u treatmet for vaults/mar forebay Cart Bay Area (m ²) 0.77 1.03 | ed in cl 2.2 above use minimum 600kg/t nholes with forebays. U: Max Number Carts 1 3 | na/yr se 50% pretreatment for i Cart Bay Length (m) 1.00 1.00 | upstream GPT ie Enviro With for Cart Bay Width (m) N/A 0.76 | Pod. Use 75%-90% sy ebay Cart Bay Area (m²) N/A 0.76 | Stem efficiency Max Number Carts N/A 2 | |
| Length Cartridge Bay (Lbay) Width Cartridge Bay (Wbay) Area Cartridge Bay (Abay) Total area of Cartridges (Acarts) Area Lower Volume (Alow) Volume Lower Volume (Now) Area Upper Volume (Aupp) Volume Upper Volume (Vupp) Live storage volume at internal bypass (Vstor) | 1.000 m 2.440 m 2.440 m ² 0.905 m ² 1.535 m ² 1059 L 2.440 m ² 2.44 L 1303 L | 3.0 Estimate sediment mass 3.1 Use tables 2 & 3 below to 3.2 Use table 3 below to f 3.3 Use 0% pretreatment for v Std Manhole Dimensions 1050 1200 1500 | loading (Refer sheet '2. 1 fill in StormFilter Dimen ill in estimated TSS c vaults/manholes with no Cart Bay Length (m) 1.00 1.00 | Ass Load Cales' for mor sions based on number o oncentration. For road forebay. Use 10-15% pre Without Cart Bay Width (m) 0.77 1.03 1.67 | re details) of cartridges as calculat ds with 252,000ypd, u treatmet for vaults/mar forebay Cart Bay Area (m ²) 0.77 1.03 1.67 | ed in cl 2.2 above rse minimum 600kg/t holes with forebays. U: Max Number Carts 1 3 4 | halyr Se 50% pretreatment for 1 Cart Bay Length (m) 1.00 1.00 1.00 | upstream GPT ie Enviro With for Cart Bay Width (m) N/A 0.76 1.39 | Pod. Use 75%-90% sy ebay Cart Bay Area (m²) N/A 0.76 1.39 | stem efficiency Max Number Carts N/A 2 3 | |
| Length Cartridge Bay (Lbay) Width Cartridge Bay (Wbay) Area Cartridge Bay (Abay) Total area of Cartridges (Acarts) Area Lower Volume (Alow) Volume Lower Volume (Vlow) Area Upper Volume (Vupp) Volume Upper Volume (Vupp) Live storage volume at internal bypass (Vstor) | 1.000 m 2.440 m 2.440 m ² 0.905 m ² 1.535 m ² 1059 L 2.440 m ² 244 L 1303 L | 3.0 Estimate sediment mass 3.1 Use tables 2 & 3 below to 3.2 Use table 3 below to f 3.3 Use 0% pretreatment for v Std Manhole Dimensions 1050 1200 1500 1800 | loading (Refer sheet '2. 1 fill in StormFilter Dimen iil in estimated TSS c vaults/manholes with no Cart Bay Length (m) 1.00 1.00 1.00 | Ass Load Cales' for mor sions based on number of oncentration. For road forebay. Use 10-15% pre Without Cart Bay Width (m) 0.77 1.03 1.67 2.44 | re details) of cartridges as calculat ds with ≥25,000vpd, u treatment for vaults/mar t forebay Cart Bay Area (m ²) 0.77 1.03 1.67 2.44 | ed in cl 2.2 above use minimum 600kg/h hholes with forebays. U Max Number Carts 1 3 4 7 | Nalyr se 50% pretreatment for r Cart Bay Length (m) 1.00 1.00 1.00 | upstream GPT ie Enviro With for Cart Bay Width (m) N/A 0.76 1.39 1.83 | Pod. Use 75%-90% sy ebay Cart Bay Area (m ²) N/A 0.76 1.39 1.83 | stem efficiency Max Number Carts N/A 2 3 3 5 | |
| Length Cartridge Bay (Lbay) Width Cartridge Bay (Wbay) Area Cartridge Bay (Mbay) Total area of Cartridges (Acarts) Area Lower Volume (Alow) Volume Lower Volume (Vlow) Area Upper Volume (Vupp) Usine Upper Volume (Vupp) Live storage volume at internal bypass (Vstor) | 1.000 m 2.440 m 2.440 m ² 0.905 m ² 1.535 m ² 1059 L 2.440 m ² 244 L 1303 L | 3.0 Estimate sediment mass 3.1 Use tables 2 & 3 below to 3.2 Use table 3 below to f 3.3 Use 0% pretreatment for v Std Manhole Dimensions 1050 1200 1500 1500 2050 | loading (Refer sheet '2. 1 fill in StormFilter Dimen fill in estimated TSS c vaults/manholes with no Cart Bay Length (m) 1.00 1.00 1.00 1.00 | Aass Load Cales' for mor sions based on number of oncentration. For roar forebay. Use 10-15% pre Without Cart Bay Widt mo 0.77 1.03 1.67 2.44 3.20 | re details) of cartridges as calculat ds with ≥25,000vpd, u treebay Cart Bay Area (m ²) 0.77 1.03 1.67 2.44 3.20 | ed in cl 2.2 above use minimum 600kg/l hholes with forebays. U: Max Number Carts 1 3 4 7 9 | halyr se 50% pretreatment for i Cart Bay Length (m) 1.00 1.00 1.00 1.00 1.00 | upstream GPT ie Enviro With for Cart Bay Width (m) N/A 0.76 1.39 1.83 2.80 | Pod. Use 75%-90% sy ebay Cart Bay Area (m ²) N/A 0.76 1.39 1.83 2.80 | Stem efficiency Max Number Carts N/A 2 3 5 7 | |
| Length Cartridge Bay (Lbay) Width Cartridge Bay (Wbay) Area Cartridge Bay (Mbay) Total area of Cartridge Bay (Abay) Total area of Cartridges (Acarts) Area Lower Volume (Alow) Volume Lower Volume (Vlow) Volume Upper Volume (Vlow) Volume Upper Volume (Vupp) Live storage volume at internal bypass (Vstor) 3.2 Catchment Sediment Loading Estimated TSS Concentration Impervious Roof (TSSroof) | 1.000 m 2.440 m 2.440 m ² 0.905 m ² 1.535 m ² 1059 L 2.440 m ² 244 L 1303 L | 3.0 Estimate sediment mass 3.1 Use tables 2 & 3 below to 3.2 Use table 3 below to f 3.3 Use 0% pretreatment for 1 Std Manhole Dimensions 1050 1200 1500 1800 2050 | loading (Refer sheet '2. 1 fill in StormFilter Dimen ill in estimated TSS c vaults/manholes with no Cart Bay Length (m) 1.00 1.00 1.00 1.00 1.00 | Aass Load Cales' for mor sions based on number of oncentration. For road forebay, Use 10-15% pre Without Cart Bay Width (m) 0.77 1.03 1.67 2.44 3.20 | re details) of cartridges as calculat ds with 252,000 ypd, u treatment for vaults/mar forebay Cart Bay Area (m ²) 0.77 1.03 1.67 2.44 3.20 | ed in cl 2.2 above use minimum 600kg/t holes with forebays. U Max Number Carts 1 3 4 7 9 | 12/yr Se 50% pretreatment for r Cart Bay Length (m) 1.00 1.00 1.00 1.00 | upstream GPT ie Enviro With for Cart Bay Width (m) N/A 0.76 1.39 1.83 2.80 Table 2: Standard St | Pod. Use 75%-90% sy ebay Cart Bay Area (m ²) NA 0.76 1.39 1.83 2.80 ormwater360 Manh | stem efficiency Max Number Carts N/A 2 3 5 7 ole Dimensions | |
| Length Cartridge Bay (Lbay) Width Cartridge Bay (Wbay) Area Cartridge Bay (Mbay) Total area of Cartridges (Acarts) Area Lower Volume (Alow) Volume Lower Volume (Nuop) Volume Upper Volume (Vuop) Usume Upper Volume (Vuop) Live storage volume at internal bypass (Vstor) 3.2 Catchment Sediment Loading Estimated TSS Concentration Impervious Roof (TSSroof) Estimated TSS Concentration Impervious Rood (TSSrood) | 1.000 m 2.440 m 2.440 m ² 0.905 m ² 1.535 m ² 1059 L 2.440 m ² 244 L 1303 L | 3.0 Estimate sediment mass 3.1 Use tables 2 & 3 below to 3.2 Use table 3 below to f 3.3 Use 0% pretreatment for v Std Manhole Dimensions 1050 1200 1500 1600 2050 Std Yault Dimensions | loading (Refer sheet '2. 1 fill in StormFilter Dimen ill in estimated TSS c vaults/manholes with no Cart Bay Length (m) 1.00 1.00 1.00 1.00 1.00 | Ass Load Cales' for mor sions based on number oncentration. For road forebay. Use 10-15% pre Without Cart Bay Width (m) 0.77 1.03 1.67 2.44 3.20 Without | re details) of cartridges as calculat ds with ≥25,000ypd, u treatment for vaults/mar forebay Cart Bay Area (m ²) 0.77 1.03 1.67 2.44 3.20 tforebay | ed in cl 2.2 above ise minimum 600kg/h hholes with forebays. U Max Number Carts 1 3 4 7 9 | halyr Se 50% pretreatment for i Cart Bay Length (m) 1.00 1.00 1.00 1.00 1.00 | upstream GPT ie Enviro With for Cart Bay Width (m) N/A 0.76 1.39 1.83 2.80 Table 2: Standard St With for | Pod. Use 75%-90% sy ebay Cart Bay Area (m²) N/A 0.76 1.39 1.83 2.80 comwater360 Manh ebay | stem efficiency Max Number Carts N/A 2 3 5 7 ole Dimensions | |
| Length Cartridge Bay (Lbay) Width Cartridge Bay (Wbay) Area Cartridge Bay (Mbay) Total area of Cartridges (Acarts) Area Lower Volume (Alow) Volume Lower Volume (Vlow) Area Upper Volume (Vupp) Usume Upper Volume (Vupp) Live storage volume at internal bypass (Vstor) <u>3.2 Catchment Sediment Loading</u> Estimated TSS Concentration Impervious Roof (TSSroad) Estimated TSS Concentration Impervious Road (TSSroad) | 1.000 m 2.440 m 2.440 m ² 0.905 m ² 1.535 m ² 1059 L 2.440 m ² 244 L 1303 L 100 kg/ha/year 300 kg/ha/year 200 kg/ha/year | 3.0 Estimate sediment mass 3.1 Use tables 2 & 3 below to 3.2 Use table 3 below to 3.3 Use 0% pretreatment for v Std Manhole Dimensions 1050 1200 1500 1500 1800 2050 Std Vault Dimensions | loading (Refer sheet '2. I fill in StormFilter Dimen fill in estimated TSS c vaults/manholes with no Cart Bay Length (m) 1.00 1.00 1.00 Cart Bay Length (m) | Ass Load Calcs' for mor sions based on number of oncentration. For road forebay. Use 10-15% pre Without Cart Bay Width (m) 0.77 1.03 1.67 2.44 3.20 Without Cart Bay Width (m) | re details) of cartridges as calculat ds with ≥25,000vpd, u teretament for valits/mar forebay Cart Bay Area (m ²) 0.77 1.03 1.67 2.44 3.20 torebay Cart Bay Area (m ²) | ed in cl 2.2 above ise minimum 600kg/h hholes with forebays. U Max Number Carts 1 3 4 7 9 Max Number Carts | halyr se 50% pretreatment for r Cart Bay Length (m) 1.00 1.00 1.00 1.00 Cart Bay Length (m) | Upstream GPT is Enviro With for Cart Bay Width (m) N/A 0.76 1.39 1.83 2.80 Table 2: Standard St With for Cart Bay Width (m) | Pod. Use 75%-90% sy ebay Cart Bay Area (m ²) N/A 0.76 1.39 1.83 2.80 orrmwater360 Manh ebay Cart Bay Area (m ²) | stem efficiency Max Number Carts N/A 2 3 5 7 ole Dimensions Max Number Carts | |
| Length Cartridge Bay (Lbay) Width Cartridge Bay (Wbay) Area Cartridge Bay (Mbay) Total area of Cartridge Bay (Mbay) Total area of Cartridge Bay (Abay) Volume Lower Volume (Abay) Volume Lower Volume (Vlow) Volume Upper Volume (Vlow) Volume Upper Volume (Vupp) Live storage volume at internal bypass (Vstor) Live storage volume at internal bypass (Vstor) Sattimated TSS Concentration Impervious Roof (TSSroof) Estimated TSS Concentration Impervious Area (TSSper) Estimated TSS Concentration Impervious Area (TSSper) Estimated TSS Concentration Previous Area (TSSper) | 1.000 m 2.440 m 2.440 m ² 0.905 m ² 1.535 m ² 1059 L 2.440 m ² 244 L 1303 L 100 kg/ha/year 200 kg/ha/year 90 kg/year | 3.0 Estimate sediment mass 3.1 Use tables 2 & 3 below to 3.2 Use table 3 below to f 3.3 Use 0% pretreatment for v Std Manhole Dimensions 1050 1200 1500 1500 2050 Std Vault Dimensions 3.4 L x 1.5 W x 1.8 D | loading (Refer sheet '2. 1 fill in StormFilter Dimen fill in estimated TSS c vaults/manholes with no Cart Bay Length (m) 1.00 1.00 1.00 1.00 Cart Bay Length (m) 2.85 | Aass Load Cales' for mor sions based on number of oncentration. For road forebay. Use 10-15% pre Without Cart Bay Width (m) 0.77 1.03 1.67 2.44 3.20 Without Cart Bay Width (m) 1.50 | re details) of cartridges as calculati ds with ≥2,5,000 ypd, u treatment for vaults/mar forebay Cart Bay Area (m ²) 0.77 1.03 1.67 2.44 3.20 t forebay Cart Bay Area (m ²) 4.28 | ed in cl 2.2 above use minimum 600kg/h holes with forebays. U Max Number Carts 1 3 4 7 9 Max Number Carts 11 | talyr Se 50% pretreatment for i Cart Bay Length (m) 1.00 1.00 1.00 1.00 Cart Bay Length (m) 2.30 | Upstream GPT is Enviro With for Cart Bay Width (m) N/A 0.76 1.39 1.83 2.80 Table 2: Standard St With for Cart Bay Width (m) 1.80 | Pod. Use 75%-90% sy ebay Cart Bay Area (m²) NA 0.76 1.39 1.83 2.80 ornwater360 Manh ebay Cart Bay Area (m²) 4.14 | stem efficiency Max Number Carts N/A 2 3 5 7 ole Dimensions Max Number Carts 8 | |
| Length Cartridge Bay (Lbay) Width Cartridge Bay (Wbay) Area Cartridge Bay (Mbay) Total area of Cartridges (Acarts) Area Lower Volume (Alow) Volume Lower Volume (Vlow) Volume Upper Volume (Vlow) Live storage volume at internal bypass (Vstor) <u>3.2 Catchment Sediment Loading</u> Estimated TSS Concentration Impervious Road (TSSroad) Estimated TSS Concentration Impervious Road (TSSroad) Estimated TSS Concentration Pervious Area (TSSper) Estimated TSS Concentration Pervious Area (TSSper) | 1.000 m 2.440 m 2.440 m ² 0.905 m ² 1.535 m ² 1059 L 2.440 m ² 244 L 1303 L 100 kg/ha/year 200 kg/ha/year 90 kg/year | 3.0 Estimate sediment mass 3.1 Use tables 2 & 3 below to 3.2 Use table 3 below to 3.3 Use 0% pretreatment for Std Manhole Dimensions 1050 1200 1500 1800 2050 Std Vault Dimensions 3.4 L x 1.5 W x 1.8 D 4.5 L x 1.5 W x 1.8 D | loading (Refer sheet '2. 1 fill in StormFilter Dimen fill in estimated TSS c vaults/manholes with no Cart Bay Length (m) 1.00 1.00 1.00 1.00 Cart Bay Length (m) 2.85 3.95 | Ass Load Cales' for mor sions based on number of oncentration. For road forebay. Use 10-15% pre Without Cart Bay Width (m) 0.77 1.03 1.67 2.44 3.20 Without Cart Bay Width Not 1.50 | re details) of cartridges as calculat ds with 252,000 ypd, u treatment for vaults/mar forebay Cart Bay Area (m ²) 0.77 1.03 1.67 2.44 3.20 forebay Cart Bay Area (m ²) 4.28 5.93 | ed in cl 2.2 above ise minimum 600kg/t hholes with forebays. U Max Number Carts 1 3 4 7 9 Max Number Carts 11 17 | halyr Se 50% pretreatment for 1 Cart Bay Length (m) 1.00 1.00 1.00 1.00 Cart Bay Length (m) 2.30 3.40 | upstream GPT ie Enviro With for Cart Bay Width (m) N/A 0.76 1.39 1.83 2.80 Table 2: Standard St With for Cart Bay Width (m) 1.80 1.50 | Pod. Use 75%-90% sy ebay Cart Bay Area (m ²) N/A 0.76 1.39 1.33 2.80 ormwater/360 Manh ebay Cart Bay Area (m ²) 4.14 5.10 | stem efficiency Max Number Carts N/A 2 3 5 7 ole Dimensions Max Number Carts 8 14 | |
| Length Cartridge Bay (Lbay) Width Cartridge Bay (Wbay) Area Cartridge Bay (Mbay) Total area of Cartridges (Acarts) Area Lower Volume (Alow) Volume Lower Volume (Vlow) Area Upper Volume (Vlow) Usume Upper Volume (Vupp) Usume Upper Volume (Vupp) 2. Live storage volume at internal bypass (Vstor) 3.2 Catchment Sediment Loading Estimated TSS Concentration Impervious Roof (TSSroof) Estimated TSS Concentration Impervious Roof (TSSroof) Estimated TSS Concentration Pervious Area (TSSper) Estimated Total TSS Load (TSSload) 3.3 Treatment Efficiencies | 1.000 m 2.440 m 2.440 m ² 0.905 m ² 1.535 m ² 1059 L 2.440 m ² 244 L 1303 L 100 kg/ha/year 300 kg/ha/year 90 kg/year | 3.0 Estimate sediment mass 3.1 Use tables 2 & 3 below to 3.2 Use table 3 below to f 3.3 Use 0% pretreatment for v Std Manhole Dimensions 1050 1200 1500 1800 2050 Std Vault Dimensions 3.4 L x 1.5 W x 1.8 D 4.5 L x 1.5 W x 1.8 D 4.2 L x 2.0 W x 1.8 D | loading (Refer sheet '2. I fill in StormFilter Dimen fill in estimated TSS c vaults/manholes with no Cart Bay Length (m) 1.00 1.00 1.00 2.85 3.95 3.95 | Ass Load Cales' for mor sions based on number of oncentration. For road forebay. Use 10-15% pre Without Cart Bay Width (m) 0.77 1.03 1.67 2.44 3.20 Without Cart Bay Width (m) 1.50 1.50 1.55 | re details) of cartridges as calculat ds with ≥25,000 vpd, u trreatment for valits/mar forebay Cart Bay Area (m ²) 0.77 1.03 1.67 2.44 3.20 t forebay Cart Bay Area (m ²) 4.28 5.93 7.70 | ed in cl 2.2 above ise minimum 600kg/h hholes with forebays. U Max Number Carts 1 3 4 7 9 Max Number Carts 11 17 23 | halyr se 50% pretreatment for r Cart Bay Length (m) 1.00 1.00 1.00 1.00 2.00 Cart Bay Length (m) 2.30 3.40 3.40 | upstream GPT is Enviro With for Cart Bay Width (m) N/A 0.76 1.39 1.83 2.80 Table 2: Standard St With for Cart Bay Width (m) 1.80 1.50 2.10 | Pod. Use 75%-99% sy ebay Cart Bay Area (m ²) N/A 0.76 1.39 1.83 2.80 ormwater360 Manh ebay Cart Bay Area (m ²) 4.14 5.10 7.14 | stem efficiency Max Number Carts N/A 2 3 5 7 ole Dimensions Max Number Carts 8 14 14 | |
| Length Cartridge Bay (Lbay) Width Cartridge Bay (Wbay) Area Cartridge Bay (Mbay) Total area of Cartridge Bay (Mbay) Total area of Cartridge Bay (Abay) Total area of Cartridges (Acarts) Area Lower Volume (Abay) Volume Lower Volume (Viow) Area Upper Volume (Viow) Live storage volume at internal bypass (Vstor) Live storage volume at internal bypass (Vstor) Storage volume at internal bypass (Vstor) Storage volume at internal bypass (Vstor) Live storage volume at internal bypass (Vstor) Storage volume at internal bypass (Vstor) Stora | 1.000 m 2.440 m 2.440 m ² 0.905 m ² 1.535 m ² 1059 L 2.440 m ² 244 L 1303 L 100 kg/ha/year 200 kg/ha/year 90 kg/year | 3.0 Estimate sediment mass 3.1 Use tables 2 & 3 below to 3.2 Use table 3 below to f 3.3 Use 0% pretreatment for v Std Manhole Dimensions 1050 1500 1500 2050 Std Vault Dimensions 3.4 L x 1.5 W x 1.8 D 4.2 L x 2.0 W x 1.8 D 5.6 L x 2.0 W x 1.8 D | loading (Refer sheet '2. 1 fill in StormFilter Dimen fill in estimated TSS c vaults/manholes with no Cart Bay Length (m) 1.00 1.00 1.00 1.00 2.85 3.95 3.95 5.05 | Aass Load Cales' for mor sions based on number of oncentration. For road forebay. Use 10-15% pre Without Cart Bay Width (m) 0.77 1.03 1.67 2.244 3.20 Without Cart Bay Width (m) 1.50 1.50 1.95 | re details) of cartridges as calculati ds with ≥2,5,000 ypd, u treatment for vaults/mar forebay Cart Bay Area (m ²) 0.77 1.03 1.67 2.244 3.20 t forebay Cart Bay Area (m ²) 4.28 5.33 7.70 9.85 | ed in cl 2.2 above use minimum 600kg/h holes with forebays. U Max Number Carts 1 3 4 7 9 Max Number Carts 11 17 23 31 | talyr cart Bay Length (m) 1.00 1.00 1.00 1.00 1.00 Cart Bay Length (m) 2.30 3.40 4.50 | Upstream GPT is Enviro With for Cart Bay Width (m) N/A 0.76 1.39 1.83 2.80 Table 2: Standard St With for Cart Bay Width (m) 1.80 1.50 2.10 2.10 | Pod. Use 75%-90% sy ebay Cart Bay Area (m²) NA 0.76 1.39 1.83 2.80 ornwater360 Manh ebay Cart Bay Area (m²) 4.14 5.10 7.14 9.45 | stem efficiency Max Number Carts N/A 2 3 5 5 7 ole Dimensions Max Number Carts 8 14 18 26 | |
| Length Cartridge Bay (Lbay) Width Cartridge Bay (Wbay) Area Cartridge Bay (Mbay) Total area of Cartridges (Acarts) Area Lower Volume (Alow) Volume Lower Volume (Now) Volume Loper Volume (Vupp) Live storage volume at internal bypass (Vstor) 3.2 Catchment Sediment Loading Estimated TSS Concentration Impervious Road (TSSroad) Estimated TSS Concentration Impervious Road (TSSroad) Estimated TSS Concentration Pervious Area (TSSper) Estimated TSS Concentration Efficiencies Pre-treatment Efficiency (EFFpre) System Efficiency (EFFsys) | 1.000 m 2.440 m 2.440 m ² 0.905 m ² 1.535 m ² 1059 L 2.440 m ² 2.44 L 1303 L 100 kg/ha/year 300 kg/ha/year 90 kg/year | 3.0 Estimate sediment mass 3.1 Use tables 2 & 3 below to 3.2 Use table 3 below to f 3.3 Use 0% pretreatment for 1 Std Manhole Dimensions 1050 1200 1500 2050 Std Vault Dimensions 3.4 L x 1.5 W x 1.8 D 4.5 L x 1.5 W x 1.8 D 4.5 L x 1.5 W x 1.8 D 5.6 L x 2.0 W x 1.8 D 5.6 L x 2.4 W x 1.8 D | loading (Refer sheet '2. 1 fill in StormFilter Dimen fill in estimated TSS c vaults/manholes with no Cart Bay Length (m) 1.00 1.00 1.00 2.00 2.00 2.00 2.00 3.95 3.95 5.05 | Ass Load Cales' for mor sions based on number of noncentration. For road forebay. Use 10-15% pre Without Cart Bay Width (m) 0.77 1.03 1.67 2.44 3.20 Without Cart Bay Width (m) 1.50 1.50 1.55 1.95 2.40 | re details) of cartridges as calculat ds with ≥25,000 ypd, u treatment for vaults/mar forebay Cart Bay Area (m ²) 0.77 1.03 1.67 2.44 3.20 Cart Bay Area (m ²) 4.28 5.93 7.70 9.85 12.12 | ed in cl 2.2 above ise minimum 600kg/h hholes with forebays. U Max Number Carts 1 3 4 7 9 9 Max Number Carts 11 17 23 31 39 | talyr se 50% pretreatment for i Cart Bay Length (m) 1.00 1.00 1.00 1.00 2.30 3.40 3.40 4.50 | Upstream GPT ie Enviro With for Cart Bay Width (m) N/A 0.76 1.39 1.83 2.80 Table 2: Standard St With for Cart Bay Width m) 1.80 1.50 2.10 2.10 | Pod. Use 75%-90% sy ebay Cart Bay Area (m ²) NA 0.76 1.39 1.83 2.80 orntwater360 Manh ebay Cart Bay Area (m ²) 4.14 5.10 7.14 9.45 9.45 | stem efficiency Max Number Carts N/A 2 3 5 7 Ole Dimensions Max Number Carts 8 14 18 26 27 | |
| Length Cartridge Bay (Lbay) Width Cartridge Bay (Wbay) Area Cartridge Bay (Mbay) Total area of Cartridges (Acarts) Area Lower Volume (Alow) Volume Lower Volume (Now) Area Upper Volume (Vow) Area Upper Volume (Vow) Live storage volume at internal bypass (Vstor) <u>3.2 Catchment Sediment Loading</u> Estimated TSS Concentration Impervious Road (TSSroof) Estimated TSS Concentration Impervious Road (TSSroof) Estimated TSS Concentration Pervious Area (TSSper) Estimated TSS Concentration Pervious Area (TSSper) Estimated Total TSS Load (TSSload) 3.3 Treatment Efficiencies Pre-treatment Efficiency (EFFpre) System Efficiency (EFFsps) | 1.000 m 2.440 m 2.440 m ² 0.905 m ² 1.535 m ² 1059 L 2.440 m ² 244 L 1303 L 100 kg/ha/year 300 kg/ha/year 90 kg/year | 3.0 Estimate sediment mass 3.1 Use tables 2 & 3 below to 3.2 Use table 3 below to f 3.3 Use 0% pretreatment for v Std Manhole Dimensions 1050 1200 1500 1500 2050 Std Vauit Dimensions 3.4 L x 1.5 W x 1.8 D 4.5 L x 1.5 W x 1.8 D 4.2 L x 2.0 W x 1.8 D 5.6 L x 2.0 W x 1.8 D 5.6 L x 2.4 W x 1.8 D | loading (Refer sheet '2. 1 fill in StormFilter Dimen iil in estimated TSS c vaults/manholes with no Cart Bay Length (m) 1.00 1.00 1.00 2.85 3.95 3.95 3.95 5.05 5.60 | Ass Load Cales' for mor sions based on number of oncentration. For road forebay. Use 10-15% pre Without Cart Bay Width (m) 0.77 1.03 1.67 2.44 3.20 Without Cart Bay Width (m) 1.50 1.50 1.55 1.95 2.40 | re details) of cartridges as calculat ds with ≥25,000ypd, tu trreatment for vaults/mar forebay Cart Bay Area (m ²) 0.77 1.03 1.67 2.44 3.20 torebay Cart Bay Area (m ²) 4.28 5.93 7.70 9.85 12.12 13.44 | ed in cl 2.2 above ise minimum 600kg/h hholes with forebays. U Max Number Carts 1 3 4 7 9 Max Number Carts 11 17 23 31 39 44 | halyr Se 50% pretreatment for 1 Cart Bay Length (m) 1.00 1.00 1.00 1.00 Cart Bay Length (m) 2.30 3.40 3.40 4.50 4.50 | Upstream GPT is Enviro With for Cart Bay Width (m) N/A 0.76 1.39 1.83 2.80 Table 2: Standard St With for Cart Bay Width (m) 1.80 1.50 2.10 2.10 2.10 2.40 | Pod. Use 75%-90% sy ebay Cart Bay Area (m ²) N/A 0.76 1.39 1.33 2.80 ormwater/360 Manh ebay Cart Bay Area (m ²) 4.14 5.10 7.14 9.45 9.45 10.80 | stem efficiency Max Number Carts N/A 2 3 5 7 ole Dimensions Max Number Carts 8 14 18 26 27 33 | |
| Length Cartridge Bay (Lbay) Width Cartridge Bay (Wbay) Area Cartridge Bay (Mbay) Total area of Cartridge Bay (Mbay) Total area of Cartridge Bay (Abay) Total area of Cartridge Bay (Abay) Total area of Cartridge Bay (Mbay) Area Upper Volume (Abay) Volume Lower Volume (Vlow) Volume Upper Volume (Vlow) Volume Upper Volume (Vlow) Live storage volume at internal bypass (Vstor) <u>3.2 Catchment Sediment Loading</u> Estimated TSS Concentration Impervious Road (TSSroad) Estimated TSS Concentration Impervious Road (TSSper) Estimated TSS Concentration Previous Area (TSSper) Estimated TSS Concentration Previous Area (TSSper) Statemated TSS Load (TSSI) Bay (Estimated TSS Load (TSSI) Destimated TSS Concentration Previous Area (TSSper) System Efficiency (EFFsys) 3.4 Maintenance Requirements | 1.000 m 2.440 m 2.440 m ² 0.905 m ² 1.535 m ² 1059 L 2.440 m ² 244 L 1303 L 100 kg/ha/year 300 kg/ha/year 90 kg/year | 3.0 Estimate sediment mass 3.1 Use tables 2 & 3 below to 3.2 Use table 3 below to f 3.3 Use 0% pretreatment for v Std Manhole Dimensions 1050 1200 1500 1500 2050 Std Vault Dimensions 3.4 L x 1.5 W x 1.8 D 4.5 L x 1.5 W x 1.8 D 5.6 L x 2.0 W x 1.8 D 5.6 L x 2.4 W x 1.8 D 6.2 L x 2.4 W x 1.8 D | loading (Refer sheet '2. 1 fill in StormFilter Dimen fill in estimated TSS c vaults/manholes with no Cart Bay Length (m) 1.00 1.00 1.00 2.85 3.95 5.05 5.05 5.60 | Aass Load Cales' for mor sions based on number of oncentration. For road forebay, Use 10-15% pre Without Cart Bay Width (m) 0.77 1.03 1.67 2.44 3.20 Without Cart Bay Width (m) 1.50 1.50 1.95 1.95 1.95 2.40 2.40 | re details) of cartridges as calculati ds with ≥2,5,000 ypd, u treatmet for vaults/mar forebay Cart Bay Area (m ²) 0.77 1.03 1.67 2.244 3.20 t forebay Cart Bay Area (m ²) 4.28 5.93 7.70 9.85 12.12 13.44 | Max Number Carts Max Number Carts 1 3 4 7 9 Max Number Carts 11 23 31 39 | talyr se 50% pretreatment for i Cart Bay Length (m) 1.00 1.00 1.00 2.30 3.40 4.50 4.50 4.50 | Upstream GPT is Enviro With for Cart Bay Width (m) N/A 0.76 1.39 1.83 2.80 Table 2: Standard St With for Cart Bay Width (m) 1.80 1.50 2.10 2.10 2.10 2.10 2.10 2.40 Table 3: Standard | Pod. Use 75%-90% sy ebay Cart Bay Area (m²) NA 0.76 1.39 1.83 2.80 ornwater360 Manh ebay Cart Bay Area (m²) 4.14 5.10 7.14 9.45 9.45 10.80 d Stormwater360 V | stem efficiency Max Number Carts N/A 2 3 5 7 ole Dimensions Max Number Carts 8 14 18 26 27 33 ault Dimensions | |
| Length Cartridge Bay (Lbay) Width Cartridge Bay (Wbay) Area Cartridge Bay (Mbay) Total area of Cartridge Bay (Abay) Total area of Cartridges (Acarts) Area Lower Volume (Alow) Volume Lower Volume (Vlow) Volume Upper Volume (Vlow) Live storage volume at internal bypass (Vstor) 3.2 Catchment Sediment Loading Estimated TSS Concentration Impervious Road (TSSroad) Estimated T | 1.000 m 2.440 m 2.440 m ² 0.905 m ² 1.535 m ² 1059 L 2.440 m ² 2.44 L 1303 L 100 kg/ha/year 300 kg/ha/year 300 kg/ha/year 90 kg/year | 3.0 Estimate sediment mass 3.1 Use tables 2 & 3 below to 3.2 Use table 3 below to f 3.3 Use 0% pretreatment for 1 Std Manhole Dimensions 1050 1200 1500 2050 Std Vault Dimensions 3.4 L x 1.5 W x 1.8 D 4.5 L x 1.5 W x 1.8 D 4.5 L x 1.5 W x 1.8 D 5.6 L x 2.0 W x 1.8 D 5.6 L x 2.0 W x 1.8 D 5.6 L x 2.4 W x 1.8 D 4.2 L x 2.0 W x 1.8 D 5.6 L x 2.4 W x 1.8 D 4.2 L x 2.0 W x 1.8 D 5.6 L x 2.4 W x 1.8 D 4.2 L x 2.0 W x 1.8 D 5.6 L x 2.4 W x 1.8 D 4.2 L x 2.4 W x 1.8 D 4.2 L x 2.4 W x 1.8 D 4.2 L x 2.4 W x 1.8 D | loading (Refer sheet '2. 1 fill in StormFilter Dimen fill in estimated TSS c vaults/manholes with no Cart Bay Length (m) 1.00 1.00 1.00 1.00 Cart Bay Length (m) 2.85 3.95 3.95 5.05 5.05 5.05 5.05 5.05 | Aass Load Cales' for mor sions based on number of oncentration. For road forebay, Use 10-15% pre Without Cart Bay Width (n) 1.67 2.44 3.20 Without Cart Bay Width (n) 1.50 1.50 1.50 1.95 2.40 2.40 | re details) of cartridges as calculat ds with ≥25,000 ypd, u treatment for vaults/mar forebay Cart Bay Area (m ²) 0.77 1.03 1.67 2.44 3.20 Cart Bay Area (m ²) 4.28 5.93 7.70 9.85 12.12 13.44 | ed in cl 2.2 above ise minimum 600kg/l holes with forebays. U Max Number Carts 1 3 4 7 9 Max Number Carts 11 17 23 31 39 44 | talyr se 50% pretreatment for i Cart Bay Length (m) 1.00 1.00 1.00 1.00 2.30 3.40 3.40 3.40 4.50 4.50 | Upstream GPT is Enviro With for Cart Bay Width (m) N/A 0.76 1.39 1.83 2.80 Table 2: Standard St With for Cart Bay Width (m) 1.50 2.10 2.10 2.10 2.10 2.10 2.40 Table 3: Standard | Pod. Use 75%-90% sy ebay Cart Bay Area (m²) N/A 0.76 1.39 1.83 2.80 ormwater360 Manh ebay Cart Bay Area (m²) 4.14 5.10 7.14 9.45 9.45 10.80 10.80 | stem efficiency Max Number Carts N/A 2 3 5 7 ole Dimensions Max Number Carts 8 14 18 26 27 33 ault Dimensions | |
| Length Cartridge Bay (Lbay) Width Cartridge Bay (Wbay) Area Cartridge Bay (Mbay) Total area of Cartridges (Acarts) Area Lower Volume (Alow) Volume Lower Volume (Now) Area Upper Volume (Vow) Area Upper Volume (Vupp) Usine Upper Volume (Vupp) Live storage volume at internal bypass (Vstor) 3.2 Catchment Sediment Loading Estimated TSS Concentration Impervious Road (TSSroof) Estimated TSS Concentration Impervious Road (TSSroof) Estimated TSS Concentration Pervious Area (TSSper) Estimated TSS Concentration Pervious Area (TSSper) Estimated Total TSS Load (TSSioad) Signated Total TSS Load (TSSioad) System Efficiency (EFFpre) System Efficiency (EFFpre) System Efficiency (EFFpre) System Efficiency (CFFpre) Setimated number of cleans per annum (nCleans) | 1.000 m 2.440 m 2.440 m ² 0.905 m ² 1.535 m ² 1059 L 2.440 m ² 2.44 L 1303 L 100 kg/ha/year 300 kg/ha/year 200 kg/ha/year 90 kg/year 50 % 75 % | 3.0 Estimate sediment mass 3.1 Use tables 2 & 3 below to 3.2 Use table 3 below to f 3.3 Use 0% pretreatment for v Std Manhole Dimensions 1050 1200 1500 1500 2050 Std Vault Dimensions 3.4 L x 1.5 W x 1.8 D 4.5 L x 1.5 W x 1.8 D 5.6 L x 2.0 W x 1.8 D 5.6 L x 2.4 W x 1.8 D 5.6 L x 2.4 W x 1.8 D Land Use Road | loading (Refer sheet '2, 1 fill in StormFilter Dimen iil in estimated TSS c vaults/manholes with no Cart Bay Length (m) 1.00 1.00 1.00 2.85 3.95 3.95 3.95 5.05 5.05 5.60 TSS (kg/halyr) 281 + 723 | Aass Load Cales' for mor slons based on number oncentration. For road forebay. Use 10-15% pre Without Cart Bay Width (m) 0.77 1.03 1.67 2.44 3.20 Without Cart Bay Width 00 1.50 1.50 1.50 1.55 1.95 1.95 2.40 2.40 | re details) of cartridges as calculat ds with 25,000 ypd, u treatment for vaults/mar forebay Cart Bay Area (m ²) 0.77 1.03 1.67 2.44 3.20 torebay Cart Bay Area (m ²) 4.28 5.33 7.70 9.85 12.12 13.44 | ed in cl 2.2 above ise minimum 600kg/h holes with forebays. U Max Number Carts 1 4 7 9 Max Number Carts 11 17 23 31 39 44 | halyr Se 50% pretreatment for 1 1.00 1.00 1.00 1.00 2.00 2.30 3.40 4.50 4.50 | upstream GPT is Enviro With for Cart Bay Width (m) N/A 0.76 1.39 1.83 2.80 Table 2: Standard St With for Cart Bay Width (m) 1.80 1.50 2.10 2.10 2.10 2.40 Table 3: Standard | Pod. Use 75%-90% sy ebay Cart Bay Area (m ²) N/A 0.76 1.39 1.83 2.80 ornwater/360 Manh ebay Cart Bay Area (m ²) 4.14 5.10 7.14 9.45 9.45 10.80 d Stornwater/360 V | stem efficiency Max Number Carts N/A 2 3 5 7 ole Dimensions Max Number Carts 8 14 18 26 27 33 'ault Dimensions | |
| Length Cartridge Bay (Lbay) Width Cartridge Bay (Wbay) Area Cartridge Bay (Mbay) Total area of Cartridge Bay (Mbay) Total area of Cartridge Bay (Abay) Total area of Cartridge Bay (Abay) Total area of Cartridge Bay (Abay) Volume Lower Volume (Jone) Volume Lower Volume (Viow) Volume Lower Volume (Viow) Live storage volume at internal bypass (Vstor) Carter (Store) Live storage volume at internal bypass (Vstor) Setimated TSS Concentration Impervious Road (TSSroad) Estimated TSS Concentration Impervious Road (TSSroad) Estimated TSS Concentration Pervious Area (TSSper) Estimated TSS Concentration Pervious Area (TSSper) Estimated TSS Concentration Pervious Area (TSSper) Estimated TSS Concentration Pervious Area (TSSper) System Efficiency (EFFpre) System Efficiency (EFFpre) System Efficiency (EFFpre) System Efficiency (EFFsys) Cartes and number of cleans per annum (nCleans) Estimated Maintenance Frequency (Mfreq) | 1.000 m 2.440 m 2.440 m ² 0.905 m ² 1.535 m ² 1059 L 2.440 m ² 244 L 1303 L 100 kg/ha/year 300 kg/ha/year 90 kg/year 90 kg/year 50 % 75 % | 3.0 Estimate sediment mass 3.1 Use tables 2 & 3 below to 3.2 Use table 3 below to f 3.3 Use 0% pretreatment for v Std Manhole Dimensions 1050 1200 1500 1500 1500 1500 1500 1500 1500 1500 1500 1500 1500 1500 1500 1500 1500 1500 1500 1500 1500 1600 15 | loading (Refer sheet '2, 1 fill in StormFilter Dimen fill in estimated TSS c vaults/manholes with no Cart Bay Length (m) 1.00 1.00 1.00 2.85 3.95 5.05 5.05 5.05 5.60 TSS (kg/ha/yr) 281 - 723 242 - 1366 | Aass Load Cales' for mori sions based on number of oncentration. For road forebay, Use 10-15% pre- Without Cart Bay Width (m) 0.77 1.03 1.67 2.44 3.20 Without Cart Bay Width (m) 1.50 1.55 1.95 1.95 2.40 2.40 | re details) of cartridges as calculati ds with ≥2,5,000 ypd, u treatmet for vaults/mar forebay Cart Bay Area (m ²) 0.77 1.03 1.67 2.244 3.20 t forebay Cart Bay Area (m ²) 4.28 5.93 7.770 9.85 12.12 13.44 | Max Number Carts Max Number Carts 1 3 4 7 9 Max Number Carts 11 3 4 3 4 3 4 3 4 3 4 3 4 3 4 3 4 3 4 3 4 | na/yr se 50% pretreatment for i Cart Bay Length (m) 1.00 1.00 1.00 2.30 3.40 4.50 4.50 4.50 4.50 | Upstream GPT is Enviro With for Cart Bay Width MA 0.76 1.39 1.83 2.80 Table 2: Standard St With for Cart Bay Width (m) 1.80 1.50 2.10 2.10 2.10 2.10 2.10 2.10 2.10 2.1 | Pod. Use 75%-90% sy ebay Cart Bay Area (m²) NA 0.76 1.39 1.83 2.80 ormwater 360 Manh ebay Cart Bay Area (m²) 4.14 5.10 7.14 9.45 9.45 10.80 d Stormwater 360 V mwater Managemen | stem efficiency Max Number Carts N/A 2 3 5 7 ole Dimensions Max Number Carts 8 14 18 26 27 33 ault Dimensions tt Inc., Technical Public | sation PD-04- |
| Length Cartridge Bay (Lbay) Width Cartridge Bay (Wbay) Area Cartridge Bay (Mbay) Total area of Cartridge Bay (Mbay) Total area of Cartridge Bay (Mbay) Narea Lower Volume (Acarts) Area Lower Volume (Vlow) Volume Lower Volume (Vlow) Live storage volume at internal bypass (Vstor) Solume Upper Volume (Vupp) Live storage volume at internal bypass (Vstor) Estimated TSS Concentration Impervious Road (TSSroof) Estimated TSS Concentration Impervious Road (TSSIcad) System Efficiency (EFFpre) System Efficiency (EFFpre) Estimated number of cleans per annum (nCleans) Estimated number of cleans per annum (nCleans) | 1.000 m 2.440 m 2.440 m ² 0.905 m ² 1.535 m ² 1059 L 2.440 m ² 2.44 L 1303 L 100 kg/ha/year 300 kg/ha/year 200 kg/ha/year 90 kg/year 90 kg/year | 3.0 Estimate sediment mass 3.1 Use tables 2 & 3 below to 3.2 Use table 3 below to f 3.3 Use 0% pretreatment for v Std Manhole Dimensions 1050 1500 1500 2050 Std Vault Dimensions 3.4 L x 1.5 W x 1.8 D 4.2 L x 2.0 W x 1.8 D 5.6 L x 2.4 W x 1.8 D 1.5 L x 1.5 W x 1.8 D 1.5 L x 2.0 W x 1.8 D 1.5 L x 2.4 W x 1.8 D 1.5 L x 2.5 W x 2. | loading (Refer sheet '2, 1 fill in StormFilter Dimen fill in estimated TSS c vaults/manholes with no Cart Bay Length (m) 1.00 1.00 1.00 2.85 3.95 3.95 5.05 5.05 5.05 5.05 5.05 5.05 5.05 5 | Ass Load Cales' for mor sions based on number of oncentration. For road forebay. Use 10-15% pre Without Cart Bay Width (m) 1.57 2.44 3.20 Without Cart Bay Width (m) 1.50 1.55 1.95 1.95 2.40 2.40 | re details) of cartridges as calculati ds with ≥2,5,000 ypd, u treatment for vaults/mar forebay Cart Bay Area (m ²) 0.77 1.03 1.67 2.44 3.20 Cart Bay Area (m ²) 4.28 5.93 7.70 9.85 12.12 13.44 | ed in cl 2.2 above use minimum 600kg/h holes with forebays. U Max Number Carts 1 3 4 7 9 Max Number Carts 11 17 23 31 39 44 | nalyr se 50% pretreatment for u Cart Bay Length (m) 1.00 1.00 1.00 1.00 1.00 2.30 3.40 3.40 3.40 3.40 3.40 3.40 3.40 3 | Upstream GPT is Enviro With for Cart Bay Widt (m) N/A 0.76 1.39 1.83 2.80 Table 2: Standard St With for Cart Bay Width (m) 1.50 2.10 2.10 2.10 2.10 2.10 2.10 2.10 2.10 2.10 2.20 Table 3: Standard Nore of the Stormward from Stor 002.0 2. Contech Stormward | Pod. Use 75%-90% sy ebay Cart Bay Area (m²) NA 0.76 1.39 1.83 2.80 ormwater360 Manh ebay Cart Bay Area (m²) 4.14 5.10 7.14 9.45 10.80 d Stormwater360 V mwater Managemen ter Solutions. Strem | stem efficiency Max Number Carts N/A 2 3 5 7 ole Dimensions Max Number Carts 8 14 18 26 27 33 rault Dimensions tt Inc., Technical Public Eitter Product Design I | zation PD-04- |
| Length Cartridge Bay (Lbay) Width Cartridge Bay (Wbay) Area Cartridge Bay (Mbay) Total area of Cartridges (Acarts) Area Lower Volume (Alow) Volume Lower Volume (Vlow) Volume Upper Volume (Vlop) Live storage volume at internal bypass (Vstor) 3.2 Catchment Sediment Loading Estimated TSS Concentration Impervious Road (TSSroof) Estimated TSS Concentration Impervious Road (TSSroof) Estimated TSS Concentration Pervious Road (TSSroof) Estimated TSS Concentration Pervious Road (TSSroof) Estimated TSS Concentration Pervious Area (TSSper) Estimated TSS Concentration Pervious Area (TSSper) Summer (SSS Concentration Pervious Area (TSSPer) Estimated TSS Concentration Pervious Area (TSSPer) Summer (Estimated Total TSS Load (TSSIoad) 3.3 Treatment Efficiencies Pre-treatment Efficiency (EFFpre) System Efficiency (EFFpre) System Efficiency (EFFpre) Estimated number of cleans per annum (nCleans) Estimated Maintenance Frequency (Mfreq) AD Design Summary | 1.000 m 2.440 m 2.440 m ² 0.905 m ² 1.535 m ² 1059 L 2.440 m ² 2.44 L 1303 L 100 kg/ha/year 300 kg/ha/year 200 kg/ha/year 90 kg/year 50 % 75 % | 3.0 Estimate sediment mass 3.1 Use tables 2 & 3 below to 3.2 Use table 3 below to f 3.3 Use 0% pretreatment for v Std Manhole Dimensions 1050 1200 1500 1500 2050 Std Vault Dimensions 3.4 L x 1.5 W x 1.8 D 4.5 L x 1.5 W x 1.8 D 5.6 L x 2.0 W x 1.8 D 5.6 L x 2.0 W x 1.8 D 5.6 L x 2.0 W x 1.8 D 5.6 L x 2.4 W x 1.8 D Call Land Use Road Commercial Residential (low) | loading (Refer sheet '2. 1 fill in StormFilter Dimen fill in estimated TSS c vaults/manholes with no Cart Bay Length (m) 1.00 1.00 1.00 1.00 2.85 3.95 5.05 5.05 5.05 5.05 5.05 5.05 5.05 5 | Ass Load Cales' for mor sions based on number of oncentration. For road forebay. Use 10-15% pre Without Cart Bay Width (m) 1.67 2.44 3.20 Without Cart Bay Width (m) 1.50 1.50 1.55 1.95 2.40 2.40 | re details) of cartridges as calculat ds with 225,000ypd, u treatment for vaults/mar forebay Cart Bay Area (m ²) 0.77 1.03 1.67 2.44 3.20 Cart Bay Area (m ²) 4.28 5.93 7.70 9.85 12.12 13.44 | ed in cl 2.2 above ise minimum 600kg/h hholes with forebays. U Max Number Carts 1 4 7 9 Max Number Carts 11 17 23 31 39 44 | na/yr se 50% pretreatment for (Cart Bay Length (m) 1.00 1.00 1.00 1.00 1.00 Cart Bay Length (m) 2.30 3.40 4.50 4.50 4.50 1. | upstream GPT is Enviro With for Cart Bay Width (m) NA 0.76 1.39 1.83 2.80 Table 2: Standard St With for Cart Bay Width (m) 1.80 1.50 2.10 2.10 2.10 2.10 2.10 1.bei/ved from Stor 7 Table 3: Standard St 8 1.Derived from Stor 002.0 2. Contech Stormwa 3. Table 4.4. Techni 4.4. Techni | Pod. Use 75%-90% sy ebay Cart Bay Area (m ²) N/A 0.75 1.39 1.83 2.80 ormwater360 Manh ebay Cart Bay Area (m ²) 4.14 5.10 7.14 9.45 9.45 10.80 d Stormwater360 V mwater Managemen ter Solutions, Storm | stem efficiency Max Number Carts N/A 2 3 5 7 Ole Dimensions Max Number Carts 8 14 18 26 27 33 ault Dimensions tt Inc., Technical Publik Filter Product Design I de Edition, May 2003. | sation PD-04- Manual. Auckland |
| Length Cartridge Bay (Lbay) Width Cartridge Bay (Wbay) Area Cartridge Bay (Mbay) Total area of Cartridges (Acarts) Area Lower Volume (Alow) Volume Lower Volume (Now) Area Upper Volume (Vow) Area Upper Volume (Vupp) Use the storage volume at internal bypass (Vstor) 3.2 Catchment Sediment Loading Estimated TSS Concentration Impervious Road (TSSroof) Estimated TSS Concentration Impervious Road (TSSroof) Estimated TSS Concentration Pervious Area (TSSper) Estimated TSS Concentration Pervious Area (TSSper) Estimated TSS Concentration Pervious Area (TSSper) Estimated TSS Concentration Pervious Area (TSSper) System Efficiencies Pre-treatment Efficiency (EFFpre) System Efficiency (EFFpre) System Efficiency (EFFsys) 3.4 Maintenance Requirements Estimated number of cleans per annum (nCleans) Estimated number of cleans per annum (nCleans) Estimated Maintenance Frequency (Mfreq) Design Water Quality Treatment Flowrate (Qwq) Steve Efficiencies (VCP) | 1.000 m 2.440 m 2.440 m ² 0.905 m ² 1.535 m ² 1059 L 2.440 m ² 244 L 1303 L 100 kg/ha/year 300 kg/ha/year 300 kg/ha/year 90 kg/year 90 kg/year 90 kg/sear 90 kg/s | 3.0 Estimate sediment mass 3.1 Use tables 2 & 3 below to 3.2 Use table 3 below to f 3.3 Use 0% pretreatment for v Std Manhole Dimensions 1050 1200 1500 1500 1500 2050 Std Vault Dimensions 3.4 L x 1.5 W x 1.8 D 4.5 L x 1.5 W x 1.8 D 5.6 L x 2.0 W x 1.8 D 5.6 L x 2.0 W x 1.8 D 5.6 L x 2.4 W x 1.8 D 6.2 L x 2.4 W x 1.8 D Land Use Road Commercial Residential (low) Residential (low) | loading (Refer sheet '2, 1 fill in StormFilter Dimen fill in estimated TSS c vaults/manholes with no Cart Bay Length (m) 1.00 1.00 1.00 2.85 3.95 5.05 5.05 5.05 5.60 TSS (kg/halyr) 2.81 - 723 2.42 - 1369 6.0 - 340 9.7 - 547 4.32 - 755 | Aass Load Cales' for more sions based on number of oncentration. For roar forebay, Use 10-15% pre- Without Cart Bay Width (m) 0.77 1.03 1.67 2.44 3.20 Without Cart Bay Width (m) 1.50 1.55 1.95 2.40 2.40 | re details) of cartridges as calculati ds with ≥2,5,000 ypd, u treatment for vaults/mar forebay Cart Bay Area (m ²) 0.77 1.03 1.67 2.244 3.20 torebay Cart Bay Area (m ²) 4.28 5.93 7.70 9.85 12.12 13.44 | ed in cl 2.2 above ise minimum 600kg/h hholes with forebays. U Max Number Carts 1 4 7 9 Max Number Carts 11 17 23 31 39 44 | halyr Se 50% pretreatment for 1 Cart Bay Length (m) 1.00 1.00 1.00 1.00 1.00 Cart Bay Length (m) 2.30 3.40 4.50 4.50 4.50 | with for With for Cart Bay Width (m) N/A 0.76 1.39 1.33 2.80 Table 2: Standard St With for Cart Bay Width (m) 1.80 1.50 2.10 2.10 2.10 2.10 2.10 3. Table 3: Standard St References 1. Derived from Stor Model 3. Table 4-4, Technil | Pod. Use 75%-90% sy ebay Cart Bay Area (m ²) N/A 0.76 1.39 1.33 2.80 comwater 360 Manh ebay Cart Bay Area (m ²) 4.14 5.10 7.14 9.45 10.80 rd Stormwater 360 V mwater Managemen ter Solutions, Storm cal Publication 10, 2 | stem efficiency Max Number Carts N/A 2 3 5 7 ole Dimensions Max Number Carts 8 14 18 26 27 33 'ault Dimensions tt Inc., Technical Public Filter Product Design I defition, May 2003, | cation PD-04- Manual Auckland |
| Length Cartridge Bay (Lbay) Width Cartridge Bay (Wbay) Area Cartridge Bay (Mbay) Total area of Cartridge Bay (Mbay) Total area of Cartridge Bay (Abay) Total area of Cartridge Bay (Abay) Total area of Cartridge Bay (Abay) Area Lower Volume (Alow) Volume Lower Volume (Vlow) Volume Lower Volume (Vlow) Live storage volume at internal bypass (Vstor) Estimated TSS Concentration Impervious Roof (TSSroof) Estimated TSS Concentration Impervious Roof (TSSroof) Estimated TSS Concentration Impervious Area (TSSper) Estimated TSS Concentration Impervious Area (TSSper) Estimated TSS Concentration Pervious Area (TSSper) Estimated TSS Concentration Pervious Area (TSSper) Estimated TSS Concentration Pervious Area (TSSper) System Efficiency (EFFsys) 3.4 Maintenance Requirements Estimated number of cleans per annum (nCleans) Estimated number of cleans per annum (nCleans) Estimated Maintenance Frequency (Mfreq) Besign Water Quality Treatment Flowrate (Qsr) StormFilter Design WQ Treatment flowrate (QSF) | 1.000 m 2.440 m 2.440 m ² 0.905 m ² 1.535 m ² 1059 L 2.440 m ² 2.44 L 1303 L 100 kg/ha/year 300 kg/ha/year 300 kg/ha/year 90 kg/ya/year 90 kg/ya/year 90 kg/year 90 kg/sear 90 kg/sear | 3.0 Estimate sediment mass 3.1 Use tables 2 & 3 below to 3.2 Use table 3 below to f 3.3 Use 0% pretreatment for Std Manhole Dimensions 1050 1200 1500 1500 2050 Std Vault Dimensions 3.4 L x 1.5 W x 1.8 D 4.5 L x 1.5 W x 1.8 D 5.6 L x 2.4 W x 1.8 D 6.2 L x 2.4 W x 1.8 D Commercial Residential (low) Residential (low) | loading (Refer sheet '2, 1 fill in StormFilter Dimen fill in estimated TSS c vaults/manholes with no 1.00 1.00 1.00 1.00 2.85 3.95 5.05 5.05 5.05 5.05 5.05 5.05 5.05 5 | Ass Load Cales' for mor sions based on number of oncentration. For road forebay. Use 10-15% pre- Vithout Cart Bay Width (m) 1.67 2.44 3.20 Without Cart Bay Width (m) 1.50 1.55 1.95 1.95 2.40 2.40 2.40 | re details) of cartridges as calculat ds with 225,000 ypd, u treatment for vaults/mar forebay Cart Bay Area (m ²) 2.44 3.20 t forebay Cart Bay Area (m ²) 4.28 5.93 7.70 9.85 12.12 13.44 | ed in cl 2.2 above use minimum 600kg/h holes with forebays. U Max Number Carts 1 3 4 7 9 Max Number Carts 11 17 23 31 39 44 | 12/yr Se 50% pretreatment for o Cart Bay Length (m) 1.00 1.00 1.00 1.00 1.00 Cart Bay Length (m) 2.30 3.40 3.40 3.40 4.50 4.50 4.50 4.50 | Upstream GPT is Enviro With for Cart Bay Width (m) 0.76 1.33 1.83 2.80 Table 2: Standard ST Cart Bay Width (m) 1.80 1.50 2.10 2.10 2.40 Z.40 Table 3: Standard ST Table 3: Standard ST Contech Stormwa 3. Table 4.4, Techni Regional Council | Pod. Use 75%-90% sy ebay Cart Bay Area (m²) NA 0.76 1.39 1.83 2.80 ornwater360 Manh ebay Cart Bay Area (m²) 4.14 5.10 7.14 9.45 9.45 10.80 d Stornwater360 V mwater Managemen ter Solutions, Storm cal Publication 10, 21 | stem efficiency Max Number Carts N/A 2 3 5 7 ole Dimensions Max Number Carts 8 14 18 26 27 33 'ault Dimensions tt Inc., Technical Public Filter Product Design I nd Edition, May 2003, | sation PD-04- Manual. Auckland |
| Length Cartridge Bay (Lbay) Width Cartridge Bay (Wbay) Area Cartridge Bay (Mbay) Total area of Cartridge Bay (Mbay) Total area of Cartridges (Acarts) Area Lower Volume (Alow) Volume Lower Volume (Vlow) Volume Lower Volume (Vlow) Live storage volume at internal bypass (Vstor) <u>3.2 Catchment Sediment Loading</u> Estimated TSS Concentration Impervious Road (TSSroad) Estimated TSS Concentration Impervious Road (TSSroad) System Efficiency (EFFpre) System Efficiency (EFFpre) System Efficiency (IFFpre) StormFilter Design Water Quality Treatment Flowrate (Qwq) StormFilter Design How Treatment Flowrate (QSF) StormFilter Design flowrate at internal bypass (Qbypass) | 1.000 m 2.440 m 2.440 m ² 0.905 m ² 1.535 m ² 1059 L 2.440 m ² 2.440 m ² 2.44 L 1303 L 100 kg/ha/year 300 kg/ha/year 200 kg/ha/year 300 kg/h | 3.0 Estimate sediment mass 3.1 Use tables 2 & 3 below to 3.2 Use table 3 below to f 3.3 Use 0% pretreatment for v Std Manhole Dimensions 1050 1200 1500 2050 Std Vault Dimensions 3.4 L x 1.5 W x 1.8 D 4.5 L x 1.5 W x 1.8 D 4.2 L x 2.0 W x 1.8 D 5.6 L x 2.4 W x 1.8 D 5.6 L x 2.4 W x 1.8 D Land Use Road Commercial Residential (low) Residential (low) Terraced Bush | loading (Refer sheet '2, 1 fill in StormFilter Dimen fill in estimated TSS c vaults/manholes with no Cart Bay Length (m) 1.00 1.00 1.00 1.00 2.85 3.95 3.95 5.05 5.05 5.05 5.05 5.50 TSS (kg/halyr) 2.81 - 723 2.42 - 1369 6.0 - 340 9.7 - 547 1.33 - 755 2.66 - 146 | Ass Load Cales' for more soncentration. For road forebay. Use 10-15% pre Without Cart Bay Width (m) Cart Bay Width (m) 1.67 2.44 3.20 Without Cart Bay Width (m) 1.50 1.50 1.50 1.50 1.50 1.95 2.40 2.40 | re details) of cartridges as calculat ds with 25,000 ypd, u treatment for vaults/may forebay Cart Bay Area (m ²) 0.77 1.03 1.67 2.44 3.20 Cart Bay Area (m ²) 4.28 5.93 7.70 9.85 12.12 13.44 | ed in cl 2.2 above ise minimum 600kg/h hholes with forebays. U Max Number Carts 1 3 4 7 9 Max Number Carts 11 17 23 31 39 44 | talyr se 50% pretreatment for i Cart Bay Length (m) 1.00 1.00 1.00 1.00 2.30 3.40 3.40 3.40 4.50 4.50 4.50 | Upstream GPT is Enviro With for Cart Bay Width (m) N/A 0.76 1.39 1.83 2.80 Table 2: Standard St With for Cart Bay Width (m) 1.80 1.50 2.10 2.10 2.10 2.10 2.10 2.10 3. Table 3: Standard St References 3. Table 4-4, Techni Regional Council | Pod. Use 75%-90% sy ebay Cart Bay Area (m ²) NA 0.76 1.39 1.83 2.80 ormwater360 Manh ebay Cart Bay Area (m ²) 4.14 5.10 7.14 9.45 9.45 10.80 tormwater Managemen ter Solutions, Storm cal Publication 10, 20 | stem efficiency Max Number Carts N/A 2 3 5 7 Ole Dimensions Max Number Carts 8 14 18 26 27 33 fault Dimensions tt Inc., Technical Public Filter Product Design I nd Edition, May 2003, | sation PD-04- Janual. Auckland |
| Length Cartridge Bay (Lbay) Width Cartridge Bay (Wbay) Area Cartridge Bay (Mbay) Total area of Cartridges (Acarts) Area Lower Volume (Alow) Volume Lower Volume (Vlow) Area Upper Volume (Vlow) Usume Upper Volume (Vupp) Live storage volume at internal bypass (Vstor) <u>3.2 Catchment Sediment Loading</u> Estimated TSS Concentration Impervious Road (TSSroof) Estimated TSS Concentration Impervious Road (TSSroof) Estimated TSS Concentration Pervious Area (TSSper) Estimated TSS Concentration Pervious Area (TSSper) Estimated TSS Concentration Pervious Area (TSSper) System Efficiency (EFFpre) System Efficiency (EFFpre) System Efficiency (EFFpre) System Efficiency (Mfreq) Estimated number of cleans per annum (nCleans) Estimated number of cleans per annum (nCleans) Estimated Maintenance Frequency (Mfreq) StormFilter Design Water Quality Treatment Flowrate (QsP) StormFilter Design Ware Cartinges required (nTOTAL) | 1.000 m 2.440 m 2.440 m ² 0.905 m ² 1.535 m ² 1059 L 2.440 m ² 2.44 L 1303 L 100 kg/ha/year 300 kg/ha/year 200 kg/ha/year 200 kg/ha/year 300 kg/year 300 kg/year 300 kg/year 300 kg/ha/year 300 kg/ha/ha/ha 300 kg/ha/ha/ha 300 kg/ha/ha 300 kg/ha/ha 30 | 3.0 Estimate sediment mass 3.1 Use tables 2 & 3 below to 3.2 Use table 3 below to f 3.3 Use 0% pretreatment for v Std Manhole Dimensions 1050 1200 1500 1800 2050 Std Vauit Dimensions 3.4 L x 1.5 W x 1.8 D 4.5 L x 1.5 W x 1.8 D 5.6 L x 2.0 W x 1.8 D 5.6 L x 2.0 W x 1.8 D 5.6 L x 2.4 W x 1.8 D 5.6 L x 2.4 W x 1.8 D Commercial Residential (low) Residential | loading (Refer sheet '2, 1 fill in StormFilter Dimen fill in estimated TSS c vaults/manholes with no Cart Bay Length (m) 1.00 1.00 1.00 2.85 3.95 5.05 5.05 5.05 5.60 TSS (kg/ha/yr) 281 - 723 242 - 1369 60 - 340 97 - 547 133 - 755 2.6 - 146 80 - 588 | Aass Load Cales' for more sions based on number of oncentration. For roar forebay, Use 10-15% pre Without Cart Bay Width (m) 0.77 1.03 1.67 2.44 3.20 Without Cart Bay Width (m) 1.50 1.55 1.95 2.40 2.40 | re details) of cartridges as calculate de with 225,000 ypd, u threatment for vaults/mar forebay Cart Bay Area (m ²) 0.77 1.03 1.67 2.44 3.20 torebay Cart Bay Area (m ²) 4.28 5.93 7.70 9.85 12.12 13.44 | ed in cl 2.2 above ise minimum 600kg/h hholes with forebays. U Max Number Carts 1 4 7 9 Max Number Carts 11 17 23 31 44 | na/yr se 50% pretreatment for 1 1.00 1.00 1.00 1.00 2.30 3.40 4.50 4.50 4.50 | upstream GPT is Enviro With for Cart Bay Width (m) N/A 0.76 1.39 1.33 2.80 Table 2: Standard St With for Cart Bay Width (m) 1.80 1.50 2.10 2.10 2.10 2.10 2.10 3. Table 3: Standard St N.Derived from Stor 0.2.0 2. Contech Stormwa 3. Table 4.4, Techni Regional Council | Pod. Use 75%-90% sy ebay Cart Bay Area (m ²) N/A 0.76 1.39 1.33 2.80 ornwater/360 Manh ebay Cart Bay Area (m ²) 4.14 5.10 7.14 9.45 9.45 10.80 d Stornwater/360 V mwater Managemen ter Solutions, Storm cal Publication 10, 21 | stem efficiency Max Number Carts N/A 2 3 5 7 ole Dimensions Max Number Carts 8 14 18 26 27 33 'ault Dimensions tl Inc., Technical Publik Filter Product Design I nd Edition, May 2003, | sation PD-04- Manual. Auckland |
| Length Cartridge Bay (Lbay) Width Cartridge Bay (Mbay) Area Cartridge Bay (Mbay) Total area of Cartridge Bay (Mbay) Total area of Cartridge Bay (Mbay) Total area of Cartridge Bay (Mbay) Volume Lower Volume (Abay) Volume Lower Volume (Vlow) Volume Lower Volume (Vlow) Live storage volume at internal bypass (Vstor) Estimated TSS Concentration Impervious Roof (TSSroof) Estimated TSS Concentration Impervious Roof (TSSroof) Estimated TSS Concentration Impervious Road (TSSroad) Estimated TSS Concentration Previous Area (TSSper) Estimated TSS Concentration Previous Area (TSSper) Estimated TSS Concentration Previous Area (TSSper) Estimated TSS Concentration Previous Area (TSSper) System Efficiency (EFFsys) 3.3 Treatment Efficiency (EFFsys) System Efficiency (EFFsys) System Efficiency (Mfreq) Estimated number of cleans per annum (nCleans) Estimated Maintenance Frequency (Mfreq) StormFilter Design W0 Treatment Flowrate (Qwq) StormFilter Design W0 Treatment flowrate (Qsp) StormFilter Design flowrate at internal bypass (Qbypass) Number of Storm Filter Cartridges required (nTOTAL) Treatment Flux per cartridge (FLUX) | 1.000 m 2.440 m 2.440 m ² 0.905 m ² 1.535 m ² 1059 L 2.440 m ² 244 L 1303 L 100 kg/halyear 300 kg/halyear 200 kg/halyear 300 k | 3.0 Estimate sediment mass 3.1 Use tables 2 & 3 below to 3.2 Use table 3 below to f 3.3 Use 0% pretreatment for Std Manhole Dimensions 1050 1200 1500 | loading (Refer sheet '2, 1 fill in StormFilter Dimen fill in estimated TSS c vaults/manholes with no 1.00 1.00 1.00 1.00 2.85 3.95 5.05 5.05 5.05 5.05 5.60 TSS (kg/ha/yr) 281 - 723 242 - 1369 60 - 340 97 - 547 133 - 755 26 - 146 80 - 588 50-110 (7) | Aass Load Cales' for mori sions based on number of oncentration. For road forebay. Use 10-15% pre- Without Cart Bay Width (m) 0.77 1.03 1.67 2.44 3.20 Without Cart Bay Width (m) 1.50 1.55 1.95 1.95 1.95 1.95 2.40 2.50 2 | re details) of cartridges as calculate ds with 225,000 ypd, u treatment for vaults/mar forebay Cart Bay Area (m ²) 2.244 3.20 Cart Bay Area (m ²) 4.28 5.33 7.70 9.85 12.12 13.44 | Max Number Carts Max Number Carts 1 3 4 7 9 Max Number Carts 11 17 9 | nalyr se 50% pretreatment for u Cart Bay Length (m) 1.00 1.00 1.00 1.00 1.00 2.30 3.40 4.50 4.50 4.50 4.50 | Upstream GPT is Enviro With for Cart Bay Width (m) 0.76 1.33 2.80 Table 2: Standard St With for Cart Bay Width (m) 1.80 1.50 2.10 2.20 2. Contech Stornwa 3. Table 4-4, Techni Regional Council | Pod. Use 75%-90% sy ebay Cart Bay Area (m²) NA 0.76 1.39 1.83 2.80 ornwater360 Manh ebay Cart Bay Area (m²) 4.14 5.10 7.14 9.45 9.45 10.80 d Stormwater360 V mwater Managemen ter Solutions, Storm cal Publication 10, 21 | stem efficiency Max Number Carts N/A 2 3 5 7 ole Dimensions Max Number Carts 8 14 18 26 27 33 aut Dimensions ti Inc., Technical Public Filter Product Design I nd Edition, May 2003, | sation PD-04- Manual. Auckland |
| Length Cartridge Bay (Lbay) Width Cartridge Bay (Wbay) Area Cartidge Bay (Mbay) Total area of Cartridge Bay (Mbay) Total area of Cartridges ReCarts) Area Lower Volume (Alow) Volume Lower Volume (Vlow) Volume Upper Volume (Vlow) Live storage volume at internal bypass (Vstor) <u>3.2 Catchment Sediment Loading</u> Estimated TSS Concentration Impervious Road (TSSroof) Estimated TSS Concentration Pervious Area (TSSroof) Estimated TSS Concentration Impervious Road (TSSroof) Estimated TSS Concentration Impervious Road (TSSroof) Estimated TSS Concentration Ervices Area (TSSIcad) Estimated TSS Concentration Ervices Area (TSSIcad) Estimated TSS Concentration Efficiency (EFFpre) System Efficiency (EFFpre) System Efficiency (EFFpre) System Efficiency (EFFpre) StormFilter Design Water Quality Treatment Flowrate (Qwq) StormFilter Design Howrate at internal bypass (Qbypass) Number of StormFilter Cartridges required (nTOTAL) Treatment Flux per cartridge (FLUX) Restrictor Disc Size (d) | 1.000 m 2.440 m 2.440 m ² 0.905 m ² 1.535 m ² 1059 L 2.440 m ² 2.440 m ² 2.441 m ² 2.440 m ² 2.441 m ² 2.441 m ² 2.441 m ² 2.441 m ² 2.00 kg/ha/year 300 kg/ha/y | 3.0 Estimate sediment mass 3.1 Use tables 2 & 3 below to 3.2 Use table 3 below to f 3.3 Use 0% pretreatment for ' Std Manhole Dimensions 1050 1500 1500 2050 Std Vault Dimensions 3.4 L x 1.5 W x 1.8 D 4.2 L x 2.0 W x 1.8 D 5.6 L x 2.4 W x 1.8 D Commercial Residential (high) Residential (high) Terraced Bush Grass Roof Pasture | loading (Refer sheet '2, 1 fill in StormFilter Dimen fill in estimated TSS c vaults/manholes with no Cart Bay Length (m) 1.00 1.00 1.00 2.85 3.95 3.95 5.05 5.05 5.05 5.05 5.05 5.05 5.05 5 | Aass Load Cales' for more soins based on number of oncentration. For road forebay. Use 10-15% pre Without Cart Bay Width (m) 1.03 1.67 2.44 3.20 Without Cart Bay Width (m) 1.50 1.50 1.95 1.95 2.40 2.40 2.40 | re details) of cartridges as calculat ds with 25,000 ypd, u treatment for vaults/mar forebay Cart Bay Area (m ²) 0.77 1.03 1.67 2.44 3.20 Cart Bay Area (m ²) 4.28 5.93 7.70 9.85 12.12 13.44 | ed in cl 2.2 above use minimum 600kg/h holes with forebays. U Max Number Carts 1 3 4 7 9 Max Number Carts 11 17 23 31 39 44 Vertices of the second | Ralyr se 50% pretreatment for u Cart Bay Length (m) 1.00 1.00 1.00 1.00 2.30 3.40 3.40 3.40 3.40 4.50 4.50 4.50 4.50 | Upstream GPT is Enviro With for Cart Bay Width (m) 0.76 1.39 1.83 2.80 Table 2: Standard St With for Cart Bay Width (m) 1.80 1.50 2.10 2.10 2.10 2.10 2.10 2.10 2.10 2.1 | Pod. Use 75%-90% sy ebay Cart Bay Area (m²) N/A 0.76 1.39 1.83 2.80 ormwater360 Manh ebay Cart Bay Area (m²) 4.14 5.10 7.14 9.45 10.80 d Stormwater360 V mwater Managemen ter Solutions, Storm cal Publication 10, 2 | stem efficiency Max Number Carts N/A 2 3 5 7 ole Dimensions Max Number Carts 8 14 18 26 27 33 rault Dimensions tt Inc., Technical Public Filter Product Design I de Edition, May 2003, | ation PD-04- Manual Auckland |
| Length Cartridge Bay (Lbay) Width Cartridge Bay (Wbay) Area Cartridge Bay (Mbay) Total area of Cartridges (Acarts) Area Lower Volume (Alow) Volume Lower Volume (Vupp) Usume Lower Volume (Vupp) Usume Loper Volume (Vupp) Current Upper Volume (Vupp) Live storage volume at internal bypass (Vstor) 3.2 Catchment Sediment Loading Estimated TSS Concentration Impervious Road (TSSroof) Estimated TSS Concentration Impervious Road (TSSroof) Estimated TSS Concentration Impervious Road (TSSroof) Estimated TSS Concentration Pervious Area (TSSper) Stimated TSS Concentration Impervious Road (TSSroof) Estimated TSS Concentration Pervious Area (TSSper) Estimated TSS Concentration Pervious Area (TSSper) System Efficiency (EFFpre) System Efficiency (EFFpre) System Efficiency (EFFsys) 3.4 Maintenance Requirements Estimated number of cleans per annum (nCleans) Estimated Maintenance Frequency (Mfreq) Design Water Quality Treatment Flowrate (QsP) StormFilter Design flowrate at internal bypass (Upypass) Number of StormFilter Cartridges required (nTOTAL) Treatment Flix per cartridge (FLUX) Restrictor Disc Size (d) | 1.000 m 2.440 m 2.440 m ² 0.905 m ² 1.535 m ² 1059 L 2.440 m ² 2.44 L 1303 L 100 kg/ha/year 200 kg/ha/year 200 kg/ha/year 200 kg/ha/year 200 kg/ha/year 300 kg/year 200 kg/ha/year 300 kg/year 200 kg/ha/year 200 kg/ha/ha/year 200 kg/ha/year 200 kg/ha/year 200 kg/ha/ha/ha/ha/ha/ha/ha/ha/ha/ha/ha/ha/ha/ | 3.0 Estimate sediment mass 3.1 Use tables 2 & 3 below to 3.2 Use table 3 below to f 3.3 Use 0% pretreatment for v Std Manhole Dimensions 1050 1200 1500 1500 2050 Std Vault Dimensions 3.4 L x 1.5 W x 1.8 D 4.5 L x 1.5 W x 1.8 D 5.6 L x 2.0 W x 1.8 D 5.6 L x 2.4 W x 1.8 D 5.6 L x 2.4 W x 1.8 D Commercial Residential (high) Terraced Bush Grass Roof Pasture Table 4: Suggested TSS | loading (Refer sheet '2, 1 fill in StormFilter Dimen iil in estimated TSS c vaults/manholes with no 1.00 1.00 1.00 1.00 2.85 3.95 3.95 5.05 5.60 TSS (kg/halyr) 2.81 · 723 2.42 · 1369 6.0 · 340 9.7 · 547 133 · 755 2.6 · 146 8.0 · 588 5.0 · 110 (1) 103 · 583 10ads [3] | Ass Load Cales' for mor sions based on number of oncentration. For road forebay, Use 10-15% pre Without Cart Bay Width (m) 1.67 2.44 3.20 Without Cart Bay Width (m) 1.50 1.50 1.50 1.50 1.55 2.40 2.40 | re details) of cartridges as calculat ds with 225,000 ypd, u treatment for vaults/mar forebay Cart Bay Area (m ²) 0.77 1.03 1.67 2.44 3.20 Cart Bay Area (m ²) Cart Bay Area (m ²) 4.28 5.93 7.70 9.85 12.12 13.44 | ed in cl 2.2 above ise minimum 600kg/h holes with forebays. U Max Number Carts 1 1 1 1 1 1 23 31 1 1 1 23 31 4 4 1 1 23 31 4 4 1 1 23 31 4 4 1 1 1 23 31 4 4 1 23 31 4 4 1 1 1 1 2 3 3 1 1 1 1 1 2 3 3 1 1 1 1 1 2 3 3 1 1 1 1 1 1 1 1 1 1 1 1 1 | na/yr se 50% pretreatment for (1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.0 | upstream GPT is Enviro With for Cart Bay Width (m) N/A 0.76 1.39 1.33 2.80 Table 2: Standard St With for Cart Bay Width (m) 1.80 1.50 2.10 2.10 2.10 2.10 3.1 Sale 4.1 Formore 5.1 1. Derived from Stor 002.0 2. Contech Stormwa 3. Table 4.4, Techni Regional Council 1. | Pod. Use 75%-90% sy ebay Cart Bay Area (m ²) NA 0.76 1.39 1.83 2.80 ornwater360 Manh ebay Cart Bay Area (m ²) 4.14 5.10 7.14 9.45 9.45 10.80 d Stornwater360 V mwater Managemen ter Solutions, Storm cal Publication 10, 21 | stem efficiency Max Number Carts N/A 2 3 5 7 Ole Dimensions Max Number Carts 8 14 18 26 27 33 ault Dimensions It Inc., Technical Public Filter Product Design I nd Edition, May 2003, | sation PD-04- Manual. Auckland |

Maximum Hydraulic Effect (hmax) Estimated Maintenance Frequency (Mfreq) 43 months

CSF 0054; StormFilter Flow-Based Sizing - Other - Revision 2.2 - Updated 10th December 2018



| То: | Waka Kotahi New Zealand Transport Agency | | | | | |
|------------------|---|--|--|--|--|--|
| From: | Riaan de Swardt | | | | | |
| Сору: | Neil Douglas; Sven Wright c/o Avail Pacific | | | | | |
| Subject: | Flooding Volume Assessment in Relation to the Oruaiti School Carpark Upgrades | | | | | |
| Date: | 5/11/2024 | | | | | |
| Project Ref: | 21102 | | | | | |
| Document ID: | 531680 | | | | | |
| Revision Status: | 01 | | | | | |
| Reviewer: | Neil Douglas | | | | | |

1 BACKGROUND

It is proposed to undertake various upgrades to the existing carpark at Oruaiti School, which is located at 2554 State Highway 10, Mangonui. The upgrades are proposed to provide additional carparks, a dedicated bus lane to facilitate the safe drop off and pick up of children from the school, and to address various safety concerns of the existing carpark. It is intended to not only upgrade the carpark, but to improve the existing stormwater system by introducing devices that will treat stormwater run-off prior to discharge into the lower laying Oruaiti School Environmental Wetlands and Oruaiti River.

This memorandum has been prepared to address concerns from Waka Kotahi New Zealand Transport Agency (WK NZTA) about the increase in stormwater run-off volume from the increased impervious area which will be created as part of the carpark upgrades. These concerns are required to be addressed, prior to WK NZTA issuing a license to occupy, due to a portion of the proposed works being within their property boundaries. The site is located within the 10-Year, 50-Year, and 100-Year flood hazard zones. WK has indicated that they are concerned about the increase in stormwater run-off volume being generated, and how this will affect the flood levels during the various storm events mentioned above.

WK NZTA have requested an assessment of the effects of the increased impervious area on flood depths, in particular if the increase in flood depth across the State Highway corridor is negligible or if there is a significant adverse impact.

In addition to the information that will be provided below, this memorandum should be read in conjunction with all other relevant documents relating to the proposed carpark upgrades. These include, but is not limited to:

- Stormwater Neutrality Report by LDE Ltd
- Detailed Design Set of Engineering Drawings by LDE Ltd
2 SITE LOCATION

Oruaiti School is a primary and intermediate school, catering for students from year one to year eight, and is located at 2554 State Highway 10, Mangonui. The legal description of the site is Pt Allot 18A PSH OF Mangonui East with an area of 41,600 m2. The site is located approximately 8km east of the Mangonui township. See figures below.



Figure 1: Aerial photo of the subject site (indicative boundary in red) sourced from NRC Hazards Maps showing the school and parking area, Oruaiti River, and State Highway 10.



Figure 2: 10-Year, 50-Year and 100-Year River Flood Hazard Zones - Regionwide Models. (NRC Hazards Maps)



3 PROPOSED DEVELOPEMENT

The school's carpark currently faces numerous health and safety risk for anyone visiting the school. The carpark is located along the frontage of the school and SH10. Vehicles leaving the carpark are at times forced to back into the state highway with traffic approaching from the east and west at speeds of around 100km/h.

The development proposal, as discussed and agreed to by the school, is to upgrade the carpark in order to make it safer for vehicles and pedestrians. The design approach is to provide a dedicated entrance to and exit from the school, with a separate dedicated lane for cars and busses. There will also be a turning lane incorporated for vehicles coming from the west and turning left off SH10 into the carpark.

Stormwater run-off from the school is currently collected and conveyed to the south via a series of existing pipes and structures prior to discharge to the 900Ø concrete pipe culvert beneath SH10. This culvert then discharges into the Oruaiti School Environmental Wetlands.



Figure 1: Proposed layout of upgraded carpark and wetland location. Source: LDE Drawings

4 DESKTOP STUDY SUMMARY

4.1 Existing Flood Levels and Flow Depths

At the request of WK NZTA we have undertaken additional assessment of the flooding within the State Highway corridor outside of Oruaiti School. The desired result of the flooding volume assessment will be to determine the potential flood level increase during a 10-Year storm event, due to the increased stormwater run-off volume being generated from the proposed upgrades.

Prior to undertaking this assessment, the level and depth information for the different flood scenarios were sourced from Northland Regional Council's (NRC) River and Natural Hazards department. A flood level report was issued by the NRC, which provided the flood levels for during 10-Year, 50-Year, and 100-Year storm events. A summary of the levels is provided below, with the detailed report appended to this memo.

- 10-Year: RL 4.93m
- 50-Year: RL 5.99m
- 100-Year: RL 7.38m

The flood level information was used and incorporated into LDE's modelling software in order to generate a cross section through the site to determine the potential flow depths during each storm event. The results are shown below and are also appended to this memo.

- 10-Year Flow Depth: 270mm
- 50-Year Flow Depth: 1330mm
- 100-Year Flow Depth: 2720mm



Figure 4: Cross Sectional Elevation showing the EGL, FGL, and Flood Levels. Source: LDE Drawings

4.2 Proposed Flood Levels and Flow Depth Post Development

The appended Stormwater Neutrality Report by LDE Ltd, outlines the pre- and post-development peak run-off volume analysis for the entire catchment area, as well as flow volumes form the proposed development. A summary of these volumes is provided below.

| Catchment Assessment | | | | | | |
|----------------------|--|-------|---|--------------------|--|--|
| Storm Event | Drainage Area (ha) Pre-Development Peak Discharge (m³/s) | | Post-Development Peak Discharge (m³/s) | Increase (m³/s) | | |
| 10-Year* | 10.0 | 0.563 | 0.738 | 0.175 | | |
| 100-Year | 10.0 | 1.058 | 1.355 | 0.277 | | |

| Table 1: Pre- and Post Develo | pment catchment areas ar | d peak run-off flow rates |
|-------------------------------|--------------------------|---------------------------|
| | | |

*Additional modelling was undertaken to determine the 10-Year flows using HEC-HMS software.

It is important to note that the above-mentioned flow volumes are relative to the entire 10ha catchment area, including a 20% climate change factor, and not just the increase in flows form the upgraded carpark. The volumes were determined by conservatively estimating pervious and impervious areas within the catchment to be 90% pervious and 10% impervious. Of the total catchment, only 3% (0.3ha) is impervious area associated with the proposed carpark. Therefore, it is considered that the extended carparks contribution to the flows is negligible, and that any affect will be less than minor.

In order to determine the potential change in flow depth during the 10-Year and 100-Year storm events within the SH10 corridor itself. As a result of the significant size of the flood hazard zone, a conservative area up to the southern boundary of SH10 adjacent to the school has been selected to calculate the depth.

Given the size of the flood zone and the proximity to the Oruaiti River, we have assumed that once floodwaters breach SH10 any additional water entering the 'system' from the upstream catchment will immediately disperse downstream as there are no natural or man-made features to prevent it from doing so.



Figure 5: Flooding Area (above red line) Selected for Flow Depth Determinisation. Source: LDE Drawings

In order to determine the level increase of flood water, we have taken the additional volume of run-off within the flood plain as a result of the development (and climate change) and divided by the area of the flood plain.

- 10-Year: 175m³ ÷ 12,800m² = **13.6 mm ≈ 14mm**
- 100-Year: 277m³ ÷ 21,400m² = **12.9 mm ≈ 13mm**

In summary, the potential change in flow depth during a 10-Year storm event for the entire catchment area (conservatively assumed to be bounded by SH10) including climate change factors, is only <u>14mm</u>. If a larger catchment area was utilised, the depth of flooding would be even less significant. With the additional 14mm included on top of the existing flow depth determined by the NRC, the total flow depth will be <u>284mm</u>. At our most recent meeting on the 4th October 2024, WK NZTA stated that provided the depth of floodwater during the 10-year event did not increase above <u>300mm</u> that the proposal would be supported and deemed acceptable.

In conclusion, it is the opinion of LDE Ltd that the proposed development and increase in flow depth is negligible and in accordance with what was deemed acceptable by WK NZTA.

5 LIMITATIONS

This report should be read and reproduced in its entirety including the limitations to understand the context of the opinions and recommendations given.

This report has been prepared exclusively for Oruaiti School Board of Trustees c/o Avail Pacific in accordance with the brief given to us or the agreed scope and they will be deemed the exclusive owner on full and final payment of the invoice. Information, opinions, and recommendations contained within this report can only be used for the purposes with which it was intended. LDE accepts no liability or responsibility whatsoever for any use or reliance on the report by any party other than the owner or parties working for or on behalf of the owner, such as local authorities, and for purposes beyond those for which it was intended.

This report was prepared in general accordance with current standards, codes and best practice at the time of this report. These may be subject to change.

For and on Behalf of Land Development and Engineering Ltd

Prepared by:

MATSWART

Riaan de Swardt *Civil Designer*

Reviewed by:

Neil Douglas Civil Manager – Rodney/Kaipara/Northland

APPENDICES

APPENDIX A: LDE ENGINEERING DRAWINGS APPENDIX B: NRC FLOOD REPORT (REGIONWIDE MODEL) APPENDIX C: LDE STORMWATER REPORT

Project Reference: 21102 2554 State Highway 10, Mangonui Document ID: 531680

APPENDIX A LDE ENGINEERING DRAWINGS

LDE



General Notes:

- All dimensions in metres unless noted otherwise.
- Coordinates are in terms of NZGD Mt Eden 2000. 2. All levels are in metres to NZVD2016. Origin of 3
- levels RCM XIII SO 50961 RL 11.81m.
- 4
- levels RCM XIII SO 50961 RL 11.81m. Survey undertaken by Donaldsons; 25 July 2022. Existing titles Pt Allot 18A PSH OF Mangonui East Boundaries and are approximate only and are subject to change. Final boundaries will be in accordance with city plan and LINZ requirements. Boundaries not for building design purposes. Existing building, roading, and services information may have been obtained from third party records. 5. 6.
- 7.
- 8. may have been obtained from third party records and should be verified on site prior to the commencement of any works. LDE does not in any way guarantee the accuracy of any underground service shown on this plan.
- 9. Contractor to locate / mark / confirm and protect if necessary all existing services prior to commencement of work on site. Any discrepancies

to be reported to the Engineer.10. Flood extent extracted from NRC Natural Hazards and shown indicatively for information only.

Im

| | | - | | | Original size = A3 |
|--------|-------|------------|-----------|--------------|---|
| | | | Design | D de Swordt | Project status: |
| | | | Design. | R. de Swaldt | For Consent |
| | | | Descure | D de Oureett | T OF CONCOR |
| | | | Drawn: | R. de Swardt | Project: |
| | | | | | 21102 |
| | | | Approved: | N. Douglas | Drawing No: |
| | | | | | |
| | H. D | 1 Nov 2024 | Scale A3: | 1:500 | C430 IA |
| rision | Aprvd | Date | | | • |

SH10



Long Section - FLOODING SECTION A-A

Project

Copyright: LDE All rights reserved / Do not scale off drawings / Confirm all dimensions on site prior to work Client

2554 State Highway 10, Mangonui Proposed Carpark Upgrades

Flooding Section

Drawing Title



| | | | | | Original size = A | 13 |
|--------|---------------|--------------------|-----------|--------------|--------------------------------|-----|
| | | | Design: | R. de Swardt | Project status: For Consent | |
| | | | Drawn: | R. de Swardt | Project | - |
| | | | Approved: | N. Douglas | 21102 | |
| wision | H. D Apryd | 1 Nov 2024 Date | Scale A3: | 1:500 | C430 | Re' |

| 7 38 | 202 | 7.38 |
|--------|-----|--------|
| 00 011 | 200 | 115.00 |

Oruaiti School Environmental Wetlands

Project Reference: 21102 2554 State Highway 10, Mangonui Document ID: 531680

APPENDIX B NRC FLOOD REPORT (REGIONWIDE MODEL)

Flood Level Report





Parcel ID: 4901722

Title:

Appellation: Part Allot 18A PSH OF Mangonui East

Survey Area: 53,497 m²





Useful Flood Information Definitions

Annual Exceedance Probability (AEP) - The probability of a flood event of a given size occurring in any one year, usually expressed as a percentage annual chance.

1% AEP - A flood of this size or larger has a 1 in 100 chance or a 1% probability of occurring in any year.
2% AEP - A flood of this size or larger has a 1 in 50 chance or a 2% probability of occurring in any year.
5% AEP - A flood of this size or larger has a 1 in 20 chance or a 5% probability of occurring in any year.
10% AEP - A flood of this size or larger has a 1 in 10 chance or a 10% probability of occurring in any year.

NZVD2016 - New Zealand Vertical Datum - The reference level used in our flood models to define ground level. Flood Levels - Flood levels are used from our modelled flood level rasters. The flood levels are calculated above NZVD 2016 Datum.

Climate Change (CC) - NZCPS (2010) requires that the identification of coastal hazards includes consideration of sea level rise over at least a 100-year planning period. Climate change impacts, such as increased rain intensity, have been included in the flood scenarios. You can read more about the Climate Change forecasts included in each flood model in the technical reports on the NRC website.

Mean high water spring (MHWS) - describes the highest level that spring tides reach, on average.

Coastal Flood Hazard Zones (CFHZ)

Coastal flood hazard zones are derived using a range of data including tide gauge analysis, wind and wave data and models, and use empirical calculations to estimate extreme water levels around the coastline. The calculations include projected sea level rise scenarios based on the latest Ministry for the Environment guidance.

CFHZ 0 Coastal Flood Hazard Zone 0 - area currently susceptible to coastal inundation (flooding by the sea) in a 1-in-100 year storm event

CFHZ 1 Coastal Flood Hazard Zone 1 - an area susceptible to coastal inundation (flooding by the sea) in a 1-in-50 year storm event, taking into account a projected sea-level rise of 0.6m over the next 50 years **CFHZ 2** Coastal Flood Hazard Zone 2 - an area susceptible to coastal inundation (flooding by the sea) in a 1in-100 year storm event, taking into account a projected sea-level rise of 1.2m over the next 100 years **CFHZ 3** Coastal Flood Hazard Zone 3 - an area susceptible to coastal inundation (flooding by the sea) in a 1in-100 year storm event, taking into account a projected sea-level rise of 1.2m over the next 100 years **CFHZ 3** Coastal Flood Hazard Zone 3 - an area susceptible to coastal inundation (flooding by the sea) in a 1in-100 year storm event, taking into account a projected sea-level rise of 1.5m over the next 100 years (rapid sea level rise scenario)

REGIONWIDE and PRIORITY - RIVER FLOOD HAZARD ZONES (RFHZ)

River flood hazard zones are created to raise awareness of where flood hazard areas are identified, inform decision-making and to support the minimisation of the impacts of flooding in our region. The river flood hazard zones have been created using an assessment of best current available information, engaging national and international experts in the field, using national standards and guidelines and has been peer reviewed. This will provide a good indication of the areas at potential risk of flooding from a regional perspective. However, flood mapping is a complex process which involves some approximation of the natural features and processes associated with flooding.

River Flood Hazard Zone 1 – 10% AEP flood extent: an area with a 10% chance of flooding annually **River Flood Hazard Zone 2** – 2% AEP flood extent: an area with a 2% chance of flooding annually **River Flood Hazard Zone 3** – 1% AEP flood extent: an area with a 1% chance of flooding annually with the inclusion of potential Climate Change (CC) impact



& www.nrc.govt.nz



Private Bag 9021, Te Mai, Whangarei 0143



12.5

0

25

50



75

l m







12.5

25

50

100 Year + CC



75 l m 7.38 m

6

Disclaimers

Our modelling disclaimers are linked below:

https://www.nrc.govt.nz/media/ko2dkgxn/coastal-hazard-maps-disclaimer-june-2017.pdf https://www.nrc.govt.nz/media/cqnnw12y/flood-map-disclaimer-2021.pdf

Our regionwide modelling reports are linked below:

https://www.nrc.govt.nz/environment/river-flooding-and-coastal-hazards/river-flooding/river-flood-hazard-maps/regionwide-river-catchments-analysis-technical-reports

| ARE YOU FLOOD READY? | |
|----------------------------|--|
| | Know your risk |
| 01 | Check what potential flood risks and other hazards that may impact your property. |
| | The Natural Hazards Portal is a great place to start. It's a 'one-stop-shop' of information related to natural hazards within our region: www.nrc.govt.nz/environment/natural-hazards-portal |
| | The Environmental Data Hub provides river level and flow data, as well as warning levels, rainfall data, water quality, and more: www.nrc.govt.nz/environment/environmental-data/environmental-data- hub |
| | Have a plan |
| 02 | Make sure you have an evacuation plan, emergency kit and important phone numbers ready. Check out: <u>https://getready.govt.nz/en/prepared/</u> for tips on how to get ready. |
| | Stay up to date |
| 03 | In a civil defence emergency situation, follow the updates on the Northland CDEM Group's Facebook page: www.facebook.com/civildefencenorthland |
| | Or follow updates from the embedded feed on the regional council website: www.nrc.govt.nz/civildefence |
| | In an emergency |
| | Remember, if life is threatened dial 111 to contact emergency services. |











2554 State Highway 10, Mangonui 10 Year River Flood (Regionwide Model) Depth Results in 0.4m intervals Scale: 1:900

| Lege | nd |
|-------|------------|
| 10 yr | Depth |
| m | |
| 0 | .101 - 0.4 |
| 0 | .401 - 0.8 |
| 0 | .801 - 1.2 |
| 1 | .201 - 1.6 |
| 1 | .601 - 2 |
| 2 | .001 - 2.4 |
| 2 | .401 - 2.8 |
| 2 | .801 - 3.2 |
| 3 | .201 - 3.6 |
| 3 | .601 - 4 |
| | |

Crown Copyright Reserved Projection NZTM. Datum NZTM2000. DISCLAIMER: The Northland Regional Council cannot guarantee that the information shown is accurate and should not be reused in any manner without proper consultation with its owner.

17.5

35



70 Meters

October 14 2024





2554 State Highway 10, Mangonui 50 Year River Flood (Regionwide Model) Depth Results in 0.4m intervals Scale: 1:900

| and the second division of the second divisio |
|--|
| jend |
| r Depth |
| |
| 0.101 - 0.4 |
| 0.401 - 0.8 |
| 0.801 - 1.2 |
| 1.201 - 1.6 |
| 1.601 - 2 |
| 2.001 - 2.4 |
| 2.401 - 2.8 |
| 2.801 - 3.2 |
| 3.201 - 3.6 |
| 3.601 - 4 |
| 4.001 - 4.4 |
| |

17.5

Crown Copyright Reserved Projection NZTM. Datum NZTM2000. DISCLAIMER: The Northland Regional Council cannot guarantee that the information shown is accurate and should not be reused in any manner without proper consultation with its owner.

35



Ν

70 Meters





2554 State Highway 10, Mangonui 100 Year CC River Flood (Regionwide Model) Depth Results in 0.4m intervals Scale: 1:900

Legend

| 100 | yr CC Dept |
|-----|-------------|
| m | |
| | 0.101 - 0.4 |
| | 0.401 - 0.8 |
| | 0.801 - 1.2 |
| | 1.201 - 1.6 |
| | 1.601 - 2 |
| | 2.001 - 2.4 |
| | 2.401 - 2.8 |
| | 2.801 - 3.2 |
| | 3.201 - 3.6 |
| | 3.601 - 4 |
| | 4.001 - 4.4 |
| | 4.401 - 4.8 |
| | 4.801 - 5.2 |
| | 5.201 - 5.6 |
| | |

17.5

Ex-1

Crown Copyright Reserved Projection NZTM. Datum NZTM2000. DISCLAIMER: The Northland Regional Council cannot guarantee that the information shown is accurate and should not be reused in any manner without proper consultation with its owner.

35



Ν

70 Meters

Project Reference: 21102 2554 State Highway 10, Mangonui Document ID: 531680

APPENDIX C LDE STORMWATER REPORT





Oruaiti School Board of Trustees c/o Avail Pacific STORMWATER NEUTRALITY REPORT

2554 State Highway 10, Mangonui

Project Reference: 21102 June 10, 2024

DOCUMENT CONTROL

| Version | Date | Comments |
|---------|--------------|---|
| A | 1 May 2024 | Issued for Consent |
| В | 10 June 2024 | Compliance Assessment Included – Issued for Consent |

| Version | Issued For | Prepared By | Reviewed & Authorised By |
|---------|-----------------------|--|--|
| В | Issued for Consent | Marsware | Dubinko |
| | | Riaan De Swardt Civil Designer <i>MEngNZ</i> | Helena Dubinko Senior Civil Engineer <i>MEng (Civil), MEngNZ</i> |



CONTENTS

| 1 | PRO | DJECT DESCRIPTION | 1 |
|---|--------------|--|---|
| 2 | SITE | E DESCRIPTION | 1 |
| 3 | PRO | POSED DEVELOPMENT. | 2 |
| 4 | EXIS | STING FLOW REGIME & DOWNSTREAM FLOODING | 3 |
| 5 | ATT | ENUATION REQUIREMENT | 7 |
| 6 | STO | RMWATER | 9 |
| | 6.1 🖇 | Stormwater Analysis | 9 |
| | 6.2 <i>i</i> | Analysis Parameters | 9 |
| | 6.3 I | Pre-development Peak Runoff | 0 |
| | 6.4 I | Post-development Peak Runoff | 0 |
| | 6.5 <i>i</i> | Analysis Results | 0 |
| | 6.6 9 | Stormwater Treatment Devices Recommendation1 | 1 |
| | 6.6.1 | 1 Sheet Flows from State Highway – New Traffic Island1 | 1 |
| | 6.6.2 | 2 Stormwater Catchpits | 1 |
| | 6.6.3 | 3 Stormwater Filtration System | 2 |
| 7 | COM | IPLIANCE ASSESSMENT | 2 |
| 8 | LIMI | TATIONS | 8 |
| | | | |

APPENDIX A: HEC-HMS MODEL AND RESULTS APPENDIX B: ENGINEERING DRAWINGS APPENDIX C: SUPPLIER INFORMATION AND TECHNICAL DRAWINGS



1 PROJECT DESCRIPTION

LDE Ltd was engaged by Avail Pacific on behalf of the Oruaiti School Board of Trustees to undertake a stormwater neutrality design related to upgrade of the existing carpark located at 2554 State Highway 10, Mangonui.

The purpose of this report is to determine appropriate stormwater management devices and treatment of the first flush relating to the upgrades. This report has been prepared to support a resource and building consent application.



Figure 1: Site location in relation to the surrounding Mangonui suburbs. Aerial sourced from NRC Hazard Maps

2 SITE DESCRIPTION

Oruaiti School is a primary and intermediate school, catering for student from year one to year eight, and is located at 2554 State Highway 10, Mangonui. The legal description of the site is Pt Allot 18A PSH OF Mangonui East with an area of 41,600 m². The site is located approximately 8km east of the Mangonui township.

The school site contains an admin office, classrooms, a gym, playground, and pool, with existing gravel and asphalt accessways. An existing carpark is located along the frontage of the school on the northern side of the SH10. The school currently has approximately >190 students and 15 staff members. The students are dropped off and picked-up from the carpark by busses and parents in vehicles. See figure 2 below.





Figure 2: Aerial photo of the subject site (indicative boundary in red) sourced from NRC Hazards Maps showing the school and parking area, Oruaiti River, and State Highway 10.

3 PROPOSED DEVELOPMENT

The school's carpark currently faces numerous health and safety risk for anyone visiting the school. The carpark is located along the frontage of the school and SH10. Vehicles leaving the carpark are at times forced to back into the state highway with traffic approaching from the east and west at speeds of around 100km/h.

The development proposal as discussed and agreed to by the school, is to upgrade the carpark in order to make it safer for vehicles and pedestrians. The design approach is to provide a dedicated entrance to the school and exit from the school, with a separate dedicated lane for cars and busses. There will also be a turning lane incorporated for vehicles coming from the west and turning left into the carpark.

There is currently some existing stormwater pipes and structures collecting and conveying runoff to the south via a 900Ø concrete pipe culvert, discharging into the Oruaiti School Environmental Wetlands.





Figure 3: Proposed layout of upgraded carpark and wetland location. Source: LDE Drawings

4 EXISTING FLOW REGIME & DOWNSTREAM FLOODING

A desktop study was undertaken to determine and identify a catchment area of approximately 10ha, flowing from the north towards the south. 3D Modelling software was used to process LINZ DEM data and build an existing ground surface to determine approximate flow paths as indicated below. By utilizing the existing surface data and aerial imaging it was determined that the existing flows are conveyed by both natural and man-made channels throughout the catchment area. The channels connect into pipe culverts at various locations around the perimeter of the school, where the flows are conveyed to the discharge point.

During a site walkover undertaken by an Engineer from LDE in December 2023, some of the existing inlet structures and pipe culvert were identified. It appears that most of the over land flows are captured and conveyed to the discharge point via 2 x existing pipe culverts underneath the school's sport field. One Ø375mm PVC pipe culvert could be identified on site where it starts at a scruffy dome manhole and flows from the north-east towards the south-west. The second culvert's exact location and size is unknown, but from survey and historic site information, it is estimated that a Ø900mm concrete pipe originates from the neighbouring property to the west of the school and flows towards the south-east.



The culvert from the north-east connects into a concrete tank located in the southern edge of the sports field. Information from a drainlayer who installed the tank say that it is a 25,000L Underground Duracrete Tank. From here the tank is fitted with what is assumed to be a Ø900mm concrete outlet pipe, which connects to a second large underground structure directly downstream of the tank. The exact size and depth of this structure is unknown, but it is assumed that it is also a 25,000L underground tank. The second culvert from the north-west, connects into this second tank. From this tank it is assumed that a Ø900mm concrete pipe culvert crosses the State Highway and discharges into a downstream wetland, which continues to flow into the into the tidal zone of the Oruaiti River.



Figure 4: 10ha Catchment Area (red), tanks and culverts (green) and flow directions (blue arrows) and discharge to the wetland. Not to scale.





Photo 1: Scruffy dome and pipe culvert flowing from the north-east. (LDE Site Photo)



Photo 2: First underground concrete tank with inlet from the north-east cuvlvert. (LDE Site Photo)



Photo 3: First underground concrete tank flowing into second underground structure. (LDE Site Photo)





Figure 5: Historic site data indicating both culvert sizes. (Provided by the school)

As per Northland Regional Council Hazard Maps, the risk and extent of the river and coastal flooding is shown to occur downstream of the subject catchment area. The flooding maps can be seen in the figures below.



Figure 6: 10-Year, 50-Year and 100-Year River Flood Hazard Zones - Regionwide Models. (NRC Hazards Maps)





Figure 7: Current, 50 Years, 100Years and 100 Years + Rapid Sea Level Rise Scenario Coastal Flood Hazard Zones. (NRC Hazards Maps)

5 ATTENUATION REQUIREMENT

Due to the discharge of stormwater occurring into the tidal zone of the Oruaiti River (<RL 2.0m), the FNDC Engineering Standards; Dated May 2023, Table 4-1 of the standards is shown below and states that "*If the proposed stormwater discharge is into a tidal zone, then no attenuation is required.*"

| Criteria De | esign Parameter | When required |
|---|---|--|
| Flood Control D (1% AEP event) ra | Detention required, limiting the post-development 1% AEP event flow rates to 80% of the pre-development 1% AEP event flow rates. | Where downstream flooding hazard has been identified. Where there is no CMP or site-specific SMP[.] 1. Refer to Flood Hazard Areas in the <u>District Plan</u> and any known downstream restrictions causing flooding. |
| Flow attenuation (Attenuation of the 50% and 20% AEP events) | Limit the post-development 50% and 20% AEP event flow rates to 80% of the pre-development flows through controlled attenuation and release. | Where there is no CMP or site-specific SMP [.] Catchment location dependent. Typically, always required in the upper catchment and sometimes not required where development site is located in proximity to the catchment outlet, discharging to a watercourse with sufficient network capacity, and where flow attenuation may worsen flooding hazards due to relative timing of peak flows. This is subject to assessment demonstrating no negative impacts would occur. If the proposed stormwater discharge is into a tidal zone, then no attenuation is required. |



We have utilised HEC-HMS software to analyse the pre- and post-development runoff flows generated from the catchment area to determine if the existing Ø900mm concrete culvert under the road discharging into the wetland area has sufficient capacity to accommodate the increase in impervious area generated from the carpark upgrades. It was assumed that 10% of the upstream catchment area is impervious and 90% is pervious, as shown in Table 1 below. The impervious allocation includes the upgraded carpark areas in.

Table 1. Dra, and Dast Davidenment established areas and pack run off flow rates

| Table 1. Fie- and Fost Development calcriment areas and peak run-on now rates | | | | |
|---|----------------------------------|--|--|--|
| Pre-Development Catchment Assessment | | | | |
| | Drainage Area (ha) | Peak Discharge (m³/s) | | |
| Pervious | 9.0 | 0.909 | | |
| Impervious | 1.0 | 0.151 | | |
| Total | 10.0 | 1.059 | | |
| Post-Development Catchment Assessment | | | | |
| Post-De | velopment Catchment Assess | ment | | |
| Post-De | Drainage Area (ha) | ment Peak Discharge (m³/s) | | |
| Post-De | Drainage Area (ha) 9.0 | ment Peak Discharge (m ³ /s) 1.175 | | |
| Post-De Pervious Impervious | Drainage Area (ha) 9.0 1.0 | ment Peak Discharge (m ³ /s) 1.175 0.181 | | |

Table below shows the parameters used in the HEC-HMS hydrological model of the catchment area using a 24-hr duration rainfall event with a SCS Type 1A storm profile and 20% Climate Change factor applied. A hydrological soil group, Class C, was adopted for the HEC-HMS model. The hydrological soil group was inferred based on the parameters set out in the FNDC engineering standards Table 4-3, with a time of concentration of 10 minutes.

| Land use, group C soils | Runoff curve Number | Initial rainfall abstraction (mm) |
|--------------------------|---------------------|-----------------------------------|
| Permeable (Grass/Garden) | 74 | 5 |
| Impervious Pavement Area | 98 | 0 |

Table 2: HEC-HMS model parameters

The pre- and post-development peak runoff rate for a 1% AEP rainfall event amounts to 1.059 m³/s and 1.356 m³/s respectively for the entire catchment area, as shown in the HEC-HMS outputs in the Appendix. Hydraflow Express software was used to calculate the capacity of the existing culvert. The calculation parameters used was for a concrete pipe with an n-value or 0.013, laid at a grade of 0.75%, and flow rates of 1.059 m³/s and 1.356m³/s.

The results illustrate that the existing culvert has a pre-development flow depth of approximately 540mm, and a post-development flow depth of approximately 620mm, which results in an increased flow depth of 80mm. This indicates that the existing culvert will have sufficient capacity during a post-development 1% AEP rainfall event. Analyses results in the Appendix.

We consider that there are no downstream effects on other buildings or property as the flows are shown to be able to be entirely conveyed by the pipe network in the 1% AEP storm event. Therefore, there will be no effect from the increase in impermeable areas on the State Highway. Once flows reach the main river, the 15L/s increase is



negligible compared to the expected overall flows from the river catchment. Accordingly, we consider that no attenuation of new impermeable areas is required.

6 STORMWATER

6.1 Stormwater Analysis

We have utilised HEC-HMS software to analyse both the pre- and post-development scenarios. As stated in section 5 above, stormwater attenuation for the increase in impervious areas is not a requirement and therefore only stormwater treatment devices will be considered to minimise any increased adverse effects on the downstream wetland.

6.2 Analysis Parameters

The pre-development rainfall depths used in the stormwater analysis have been taken from the historical data from NIWA HIRDS V4. For post-development the rainfall has been increased by 20% to take climate change into account.

| Rainfall depths (mm): | | | | | | | | | |
|-----------------------|-------|------|------|------|------|------|------|------|------|
| ARI | AEP | 10m | 20m | 30m | 1h | 2h | 6h | 12h | 24h |
| 1.58 | 0.633 | 8.71 | 13.4 | 16.9 | 24.5 | 34.4 | 54.8 | 70.4 | 87.4 |
| 2 | 0.5 | 9.54 | 14.6 | 18.5 | 26.9 | 37.7 | 60.2 | 77.3 | 96 |
| 5 | 0.2 | 12.4 | 19 | 24.1 | 35 | 49.2 | 78.7 | 101 | 126 |
| 10 | 0.1 | 14.5 | 22.3 | 28.2 | 41.1 | 57.8 | 92.5 | 119 | 148 |
| 20 | 0.05 | 16.7 | 25.7 | 32.5 | 47.3 | 66.7 | 107 | 138 | 172 |
| 30 | 0.033 | 18 | 27.7 | 35.1 | 51.1 | 72 | 116 | 149 | 186 |
| 40 | 0.025 | 18.9 | 29.1 | 36.9 | 53.8 | 75.9 | 122 | 157 | 196 |
| 50 | 0.02 | 19.6 | 30.3 | 38.4 | 56 | 78.9 | 127 | 163 | 204 |
| 60 | 0.017 | 20.3 | 31.2 | 39.5 | 57.7 | 81.4 | 131 | 169 | 210 |
| 80 | 0.013 | 21.2 | 32.7 | 41.4 | 60.5 | 85.3 | 137 | 177 | 221 |
| 100 | 0.01 | 22 | 33.8 | 42.9 | 62.7 | 88.4 | 142 | 184 | 229 |
| 250 | 0.004 | 25 | 38.6 | 49 | 71.6 | 101 | 163 | 210 | 263 |

Table 3: NIWA HIRDS V4 Rainfall Data

The pre- and post-development rainfall parameters used in HEC-HMS is shown in the table below.

| Table 4: Rainfall Parameters | | | | |
|------------------------------|-------------------------------|---------------------------|--|--|
| Rainfall Event | Pre-development Rainfall (mm) | Post-development Rainfall | | |
| 50% AEP 24-hour | 96 | 115.2 | | |
| 20% AEP 24-hour | 126 | 151.2 | | |
| 1% AEP 24-hour | 229 | 274.8 | | |

The table below shows the parameters used in the HEC-HMS hydrological model of the site. A hydrological soil group, Class C, was adopted for the HEC-HMS model. The hydrological soil group was inferred based on the parameters set out in the FNDC ES Table 4-3, with a time of concentration of 10 minutes.



| Table 5: HEC-HMS model parameters | | | | |
|-----------------------------------|---------------------|--|--|--|
| Land use, group C soils | Runoff curve Number | Initial rainfall abstraction (mm) | | |
| , 5 1 | | , , , , , , , , , , , , , , , , , , , | | |
| | | | | |
| | | | | |
| Permeable (Grass/Garden) | 74 | 5 | | |
| | | | | |
| Impervious Pavement Area | 98 | 0 | | |
| | ••• | , and the second s | | |

A summary of pervious and impervious areas for the pre- and post- development scenarios relevant to the modelling are shown below.

| Description | Pre-development | Post-development |
|---------------------|--------------------|--------------------|
| Pervious Area | 1240m ² | 110m ² |
| Impervious Area | 1860m ² | 2990m ² |
| Total Analyzed Area | 310 | 0m ² |

6.3 Pre-development Peak Runoff

Using the parameters above and 24-hr duration rainfall event with a SCS Type 1A storm profile, the peak discharges are calculated as follows.

Table 7: Pre-development peak runoff flow rates for the 50%, 20% and 1% AEP rainfall events

| 50% AEP (L/s) | 20% AEP (L/s) | 1% AEP (L/s) | |
|---------------|---------------|--------------|--|
| 14.8 | 20.3 | 40.6 | |

6.4 Post-development Peak Runoff

Using the parameters above for the development of new carpark, a 24-hr duration rainfall event with 20% increase allowance for climate change and SCS Type 1A storm profile, the peak discharges are calculated as follows.

| Table 8: Post-development | peak runoff flow rates for the 50%, 2 | 0% and 1% AEP rainfall events |
|---------------------------|---------------------------------------|-------------------------------|
| | | |

| 50% AEP (L/s) | 20% AEP (L/s) | 1% AEP (L/s) |
|---------------|---------------|--------------|
| 22.9 | 30.3 | 55.7 |

6.5 Analysis Results

Table below shows a summary of the pre- and post-development peak flow rates and difference generated by the proposed development. The full output tables from the HEC-HMS modelling are attached in Appendix A.



| Table 9: Peak runoff flow rates for the 50%, 20% and 1% AEP rainfall events | | | | | |
|---|-----------------------|------------------------|----------------|--|--|
| AEP Rainfall Event | Pre-development (L/s) | Post-development (L/s) | Increase (L/s) | | |
| 50% AEP | 14.8 | 22.9 | 8.1 | | |
| 20% AEP | 20.3 | 30.3 | 10.0 | | |
| 1% AEP | 40.6 | 55.7 | 15.1 | | |

The post-development flow results shown, are then input into the capacity assessment of the existing Ø900mm concrete culvert outlet from the site to the wetland. The results show that the culvert will be 77% full in the 1% AEP event and has sufficient capacity to convey the peak flows from the proposed development.

If impermeable areas greater than those analysed in this design are proposed, then a revision of the design outlined in this report will be required and we should be contacted.

6.6 Stormwater Treatment Devices Recommendation

6.6.1 Sheet Flows from State Highway – New Traffic Island

Some of the stormwater runoff from the state highway, will be captured and pass through a new traffic island constructed to serve as delineation between the school carpark and the turning lane for vehicles traveling from the west and entering the carpark.

It is proposed that the island be constructed with 150mm high block kerbs, with 0.5m wide gaps formed at 3.0m spacings. This will allow runoff to sheet flow into the island area. The Island is intended to be filled with Ø100-150mm aggregate at a depth of 300mm. Underneath this a 300mm wide x 500mm min deep subsoil drain trench will be constructed. The trench will be filled with clean drainage metal, wrapped in geotextile with a Ø110mm perforated drain coil. At both ends of the island a Ø675mm x 1.0m deep chamber with a scruffy dome will be installed and serve as a connection point for the drain coil. The chamber will be connected to the proposed stormwater system.

This will serve as a collection device for the runoff from the state highway. See the appended Engineering Drawings for details.

6.6.2 Stormwater Catchpits

To serve as a gross pollutant trap, the Stormwater 360 LittaTrap is to be installed in all catchpits for the collection of litter, leaves, debris, and other sediment from the carpark areas. Refer to the operation manual appended to this report.



6.6.3 Stormwater Filtration System

It is intended that stormwater runoff from the carpark will sheet flow towards low points and be collected via catchpits. From here all run-off will flow via a pipe network and it will be connected to the existing outlet structure, before discharging downstream into the wetland.

A Stormwater 360 Stormfilter is a cartridge filtration system that will be installed upstream of the connection point to the existing outlet structure. This passive filtration system consists of a precast concrete manhole with cartridges installed to provide treatment of the 'first flush' through filtration media to effectively remove contaminants generated by the movement of vehicles through the carpark. Refer to the supplier information and LDE engineering drawings appended to this report.

7 COMPLIANCE ASSESSMENT

The Northland Regional Council's Proposed Regional Plan for Northland; Dated February 2024, has rules in relation to wetlands. Section C.4.1.9 Land drainage and flood control general conditions, and Section C.6.4.2 Other stormwater discharges, has been assessed to determine if it is a permitted activity.

The design intent for the proposed works was to introduce treatment devices to improve the existing conditions, with a less than minor increase in adverse effects. The treatment devices selected provides a reasonable amount of treatment to mitigate the effects on the receiving environment.

"General conditions apply to activities when referred to in the rules of C.4.1 Land drainage and floor control."

| | Section C.4.1.9 Land drainage and flood control general conditions | | |
|---|---|---|--|
| 1 | There are no adverse flooding, erosion or over-drainage effects on other property. | Refer to Planning Assessment and/or Ecological Memo | |
| 2 | The activity does not alter the course of a lake or continually or intermittently flowing river. | Refer to Planning Assessment and/or Ecological Memo | |
| 3 | New land drainage does not occur within 50 metres of any natural wetland. | Refer to Planning Assessment and/or Ecological Memo | |
| 4 | Drainage does not cause any change to the seasonal or annual range in water level of a natural wetland to an extent that may adversely affect the wetland's natural ecosystem. | Refer to Planning Assessment and/or Ecological Memo | |
| 5 | No vegetation, soil or other debris generated from the activity is placed in a position where it may be carried into a river or natural wetland, lake or the coastal marine area. | Refer to Planning Assessment and/or Ecological Memo | |



| 6 | There is no damage to a flood defence or any other authorised structure. | n/a |
|----|--|--|
| 7 | Fish passage is maintained, unless an existing authorisation provides otherwise, or temporary works to enable repair and replacement works are being carried out. | Refer to Planning Assessment and/or Ecological Memo |
| 8 | Eels, fish (other than pest fish), kōura (freshwater crayfish) and kākahi (freshwater mussels) unintentionally removed during mechanical clearing of drainage channels are returned to the drainage channel as soon as practicable, but no later than one hour after their removal. | Refer to Planning Assessment and/or Ecological Memo |
| 9 | Refuelling of machinery does not take place in the bed of a river or lake. | n/a |
| 10 | Any discharge of drainage water does not contain concentrations of contaminants which have or are likely to have significant adverse effects on aquatic life in any river, wetland, or the coastal marine area. | Refer to Planning Assessment and/or Ecological Memo |
| 11 | The discharge to the water body or coastal marine area does not, beyond the zone of reasonable mixing: | |
| a) | result in any conspicuous oil or grease films, scums or foams, or floatable or suspended material except where caused by natural events in the receiving water, and | LittaTraps shall be installed in all catchpits, and each catchpit shall have a 300mm min sump which will remove gross pollutants from stormwater flows. A Stormfilter device shall be installed within the proposed carpark. The stormfilter is effective at removing 80% TSS as well as oil and grease which meets water quality requirements. The stormfilter operates like a siphon through engineered media, so these pollutants will remain trapped inside the treatment chamber. The chamber will be subject to maintenance at intervals as directed by the manufacturer. Therefore, with the treatment train provided, we consider that there will be no increase in contaminants discharged from the proposed carpark construction. |
| b) | cause the pH of the receiving water to fall outside the range of 6.5 to 9.0 (except where caused by natural events, or when natural background levels fall outside that range), and | The site is being used as a school carpark, and is not expected to have any significant impact on runoff PH levels. |
| c) | cause any emission of objectionable odour in the receiving water, and | The site use being a school carpark is not expected to have any contaminants which would cause objectionable odour of stormwater runoff. |
| d) | cause any conspicuous change in colour or visual clarity of the receiving water, and | The carpark is proposed to be sealed reducing the likelihood of suspended sediments. All runoff from the proposed carpark is treated prior to discharge to further remove suspended solids so we consider there to be no change to the colour or clarity of stormwater runoff from the subject area. |


| e) | cause the natural temperature of the receiving water body to be changed by more than three degrees Celsius, except in an Outstanding Freshwater Body where it must not be changed by more than one degree Celsius, and | It is not anticipated that the proposed works will have an effect on the temperature of the receiving water due to the short time of concentration being less than 10 minutes. |
|----|--|--|
| f) | cause contamination which may render freshwater taken from a mapped priority drinking water abstraction point (refer I Maps Ngā mahere matawhenua) unsuitable for human consumption after existing treatment. | According to NRC Proposed Regional Plan Maps, the closest drinking water abstraction point is located approximately 6km "as the crow flies" from the area of proposed works. This point is also located inland with a radius of approximately 600m to the closest body of water. See Figure below. |
| 12 | Any discharge of sediment associated with repair and maintenance activities does not occur for more than five consecutive days and must not occur for more than 12 hours on any one day. | n/a |
| 13 | Where in-river works involve bed disturbance from mechanical vegetation clearance or sediment removal: | |
| a) | if undertaken between 1 August and 31 December, a visual inspection of the works area must be undertaken, immediately prior to in-river work starting. If a shoal of whitebait is present, no in-river works shall be undertaken until the shoal passes; and | n/a |
| b) | the works shall not occur more than once in any area between 1 August and 31 December of any year. | n/a |
| 14 | Riverbank disturbance is limited to one side of the waterway, at any one time. | Refer to Planning Assessment and/or Ecological Memo |
| 15 | When mechanically clearing aquatic vegetation, a weed bucket shall be used with a curved flat base and a slatted back. | n/a |
| 16 | The activity does not take place in an īnanga spawning site between 1 March and 30 September. | Refer to Planning Assessment and/or Ecological Memo |





Figure 9: Subject Site and Nearest Drinking Water Abstraction Point (NRC Proposed Regional Plan Maps)

"The diversion and discharge of stormwater into water of onto or into land where it may enter water from an impervious area or by way of a stormwater collection system, is a permitted activity, provided:"

| Section C.6.4.2 Other stormwater discharges | | | | | | |
|---|--|---|--|--|--|--|
| 1 | the discharge or diversion is not from: | | | | | |
| a) | a public stormwater network, or | n/a | | | | |
| b) | a high-risk industrial or trade premises, and | n/a | | | | |
| 2 | the diversion and discharge does not cause or increase flooding of land on another property in a storm event of up to and including a 10 percent annual exceedance probability, or flooding of buildings on another property in a storm event of up to and including a one percent annual exceedance probability, and | Refer to Planning Assessment and/or Ecological Memo | | | | |
| 3 | where the diversion or discharge is from a hazardous substance storage or handling area: | | | | | |
| a) | the stormwater collection system is designed and operated to prevent hazardous substances stored or used on the site from entering the stormwater system, or | n/a | | | | |



| b) | there is a secondary containment system in place to intercept any spillage of hazardous substances and either discharges that spillage to a trade waste system or stores it for removal and treatment, or | n/a |
|----|---|---|
| c) | if the stormwater contains oil contaminants, the stormwater is passed through a stormwater treatment system designed in accordance with the Environmental Guidelines for Water Discharges from Petroleum Industry Sites in New Zealand (Ministry for the Environment, 1998) prior to discharge, and | n/a |
| 4 | where the diversion or discharge is from an industrial or trade premises: | |
| a) | the stormwater collection system is designed and operated to prevent any contaminants stored or used on the site, other than those already controlled by condition 3) above, from entering stormwater unless the stormwater is discharged through a stormwater treatment system, and | n/a |
| b) | any process water or liquid waste stream on the site is bunded, or otherwise contained, within an area of sufficient capacity to provide secondary containment equivalent to 100 percent of the quantity of any process water or liquid waste that has the potential to spill into a stormwater collection system, in order to prevent trade waste entering the stormwater collection system, and | n/a |
| 5 | the diversion or discharge is not into potentially contaminated land, or onto potentially contaminated land that is not covered by an impervious area, and | The discharge is to the existing road culvert which then discharges to the watercourse. No change to the flow regime is proposed as part of this development. |
| 6 | the diversion and discharge does not cause permanent scouring or erosion of the bed of a water body at the point of discharge, and | The proposed works will slightly increase the pre- development flows by 15l/s during a 1% AEP storm event, with increases of 8.1l/s and 10l/s for the 50%- and 20% AEP events. It is not anticipated that this will cause scouring or erosion to the receiving bed of water as the existing culvert is shown to be capable of discharging these flows as existing so there will be no increase in risk of erosion and scour. |
| 7 | the discharge does not contain more than 15 milligrams per litre of total petroleum hydrocarbons, and | High volumes of hydrocarbons are not expected from the carpark area. Any hydrocarbons that are collected in stormwater runoff will generally be trapped in the stormfilter device due to its syphonic function. Information received from the supplier of the Stormfilter Treatment device, states that the device can achieve greater than 80% TSS removal and meet water quality requirements. It is therefore anticipated that the Stormfilter, in conjunction with other treatment methods, will provide sufficient treatment of the run-off from the existing and proposed work. |



| 8 | the discharge does not cause any of the following effects in the receiving waters beyond the zone of reasonable mixing: | |
|----|--|--|
| a) | the production of conspicuous oil or grease films, scums or foams, of floatable or suspended materials, or | LittaTraps shall be installed in all catchpits, and each catchpit shall have a 300mm min sump which will remove gross pollutants from stormwater flows. A Stormfilter device shall be installed within the proposed carpark. The stormfilter is effective at removing 80% TSS as well as oil and grease which meets water quality requirements. The stormfilter operates like a siphon through engineered media, so these pollutants will remain trapped inside the treatment chamber. The chamber will be subject to maintenance at intervals as directed by the manufacturer. Therefore, with the treatment train provided, we consider that there will be no increase in contaminants discharged from the proposed carpark construction. |
| b) | a conspicuous change in the colour or visual clarity, or | The carpark is proposed to be sealed reducing the likelihood of suspended sediments. All runoff from the proposed carpark is treated prior to discharge to further remove suspended solids so we consider there to be no change to the colour or clarity of stormwater runoff from the subject area. |
| c) | an emission of objectionable odour, or | The site use being a school carpark is not expected to have any contaminants which would cause objectionable odour of stormwater runoff. |
| d) | the rendering of freshwater unsuitable for consumption by farm animals, or | Run-off from the existing and proposed impervious areas will be collected in catchpits with a 300mm min sump, from where it will be treated by the Stormfilter device, prior to discharging into an existing structure. From this structure the run-off will discharge into a wetland where further treatment will take place, before discharging into the Oruaiti River, from where farm animals would have access to the freshwater for consumption. The proposed works have been designed to mitigate this and enhance the existing conditions. |
| e) | the rendering of freshwater taken from a mapped priority drinking water abstraction point (refer I Maps Ngā mahere matawhenua) unsuitable for human consumption after existing treatment. | According to NRC Proposed Regional Plan Maps, the closest drinking water abstraction point is located approximately 6km "as the crow flies" from the area of proposed works. This point is also located inland with a radius of approximately 600m to the closest body of water. |



8 LIMITATIONS

This report should be read and reproduced in its entirety including the limitations to understand the context of the opinions and recommendations given.

This report has been prepared exclusively for Oruaiti School Board of Trustees c/o Avail Pacific in accordance with the brief given to us or the agreed scope and they will be deemed the exclusive owner on full and final payment of the invoice. Information, opinions, and recommendations contained within this report can only be used for the purposes with which it was intended. LDE accepts no liability or responsibility whatsoever for any use or reliance on the report by any party other than the owner or parties working for or on behalf of the owner, such as local authorities, and for purposes beyond those for which it was intended.

This report was prepared in general accordance with current standards, codes, and best practice at the time of this report. These may be subject to change.



APPENDIX A

HEC-HMS MODEL AND RESULTS







PRE-DEVELOPMENT CATCHMENT ASSESSMENT

| Project | | ject: 21102 - Carpark Stormwater | | | Simulation Run: Catchment 100-Year Pre | | | | |
|---|--|---|--------------|------------------------------------|--|-----------------------|----------|---------------------|--|
| Start of Run: 01Jan2000, 0 End of Run: 02Jan2000, 0 Compute Time:01May2024, | | 0:00 Basin Model: Catchr 0:00 Meteorologic Model: 100-Yo 16:22:06 Control Specifications:Contro | | Catchme : 100-Yea ns:Control | ent Pre ar Pre | | | | |
| Show Elements: All Elements > Volume Units: C | | | MM 🔾 10 | 00 M3 | Sorting: | Watershed Explorer $$ | | | |
| Hydrologic Element | | Drainag (KM | e Area 2) | Peak D (M | ischarge 3/S) | Time | of Peak | Volume (1000 M3) | |
| Grass Area | | 0.09 | 00 | 0.9 | 085 | 1 January | 2000, 08 | . 13.3053 | |
| Impervious | | 0.0100 0. | | 0.1 | 507 | 1 January | 2000, 07 | . 2.2330 | |
| Outlet | | 0.10 | 00 | 1.0 | 587 | 1 January | 2000, 08 | . 15.5382 | |

EXISTING CULVERT PRE-DEVELOPMENT CAPACITY ASSESSMENT

1

| | input |
|-------------------|----------|
| Section Type = | Circular |
| Btm Width (m) | -0- |
| Side Slope, z:1 = | -0- |
| Diameter(m) = | 0.9000 |
| Inv Elev(m) = | 5.0000 |
| Slope (%) = | 0.7500 |
| n-value = | 0.013 |
| Compute by = | Known Q |
| Q (cms) = | 1.0587 |
| | Run |
| | |
| | |
| | |





POST-DEVELOPMENT CATCHMENT ASSESSMENT

| F | Project: 21102 - Carpark Stormwate | | mwater | Simulation R | 'ear Post | | | | |
|--------------------------------------|---|--------------------------------------|--------------------------|------------------------------------|-------------------------------------|---------------------------------|--------------------|---------|--|
| | of Run: 01J f Run: 02J ute Time:01N | an2000, 0 an2000, 0 1ay2024, : | 0:00 0:00 16:22:06 | Basin Mo Meteorolo Control S | del: ogic Model: pecification | Catchme 100-Yea s:Control | ent +CC Ir Post | | |
| Show Elements: All Elements > Volume | | | e Units: 🔇 |) MM 🔾 10 | 00 M3 | Sorting: | Watershed Explorer | \sim | |
| Hydrologic | | Drainage Area | | Peak [| Discharge | Time | of Peak | Volume | |
| Element | | (KM2) (I | | 13/S) | | | (1000 M3) | | |
| Grass Area | | 0.0900 | | 1.1753 | | 1 January | 2000, 08 | 17.0019 | |
| Impervious | | 0.0100 | | 0. | 1810 | 1 January | 2000, 07 | 2.6895 | |
| Outlet | | 0.100 | 00 | 1. | 3557 | 1 January | 2000, 08. | 19.6914 | |

EXISTING CULVERT POST-DEVELOPMENT CAPACITY ASSESSMENT

| Section Type = Circular Btm Width (m) -0- Side Slope, z:1 = -0- Diameter(m) = 0.9000 Inv Elev(m) = 5.0000 Slope (%) = 0.7500 n-value = 0.013 Compute by = Known Q | Section Type = Circular Btm Width (m) -0- de Slope, z:1 = -0- Diameter(m) = 0.9000 Inv Elev(m) = 5.0000 Slope (%) = 0.7500 n-value = 0.013 Compute by = Known Q Q (cms) = 1.3560 Run | | |
|---|--|-------------------|----------|
| Btm Width (m) -0- Side Siope, z:1 = -0- Diameter (m) = 0.9000 Inv Elev (m) = 5.0000 Siope (%) = 0.7500 n-value = 0.013 Compute by = Known Q | Btm Width (m) -0- de Slope, z.1 = -0- Diameter(m) = 0.9000 Inv Elev(m) = 5.0000 Slope (%) = 0.7500 n-value = 0.013 Compute by = Known Q Q (cms) = 1.3580 | Section Type = | Circular |
| Side Slope, z:1 = 0- Diameter(m) = 0.9000 Inv Elev(m) = 5.0000 Slope (%) = 0.7500 n-value = 0.013 Compute by = Known Q | de Slope, z:1 =0- Diameter(m) = 0.9000 Inv Elev(m) = 5.0000 Slope (%) = 0.7500 n-value = 0.013 Compute by = Known Ω Q (cms) = 1.3560 Run | Btm Width (m) | -0- |
| Diameter(m) = 0.9000 Inv Elev(m) = 5.0000 Slope (%) = 0.7500 n-value = 0.013 Compute by = Known Q | Diameter(m) = 0.9000 Inv Elev(m) = 5.0000 Slope (%) = 0.7500 n-value = 0.013 Compute by = Known Q Q (cms) = 1.3560 Run | Side Slope, z:1 = | -0- |
| Inv Elev(m) = 5.0000 Slope (%) = 0.7500 n-value = 0.013 Compute by = Known Q | Inv Elev(m) = 5.0000 Slope (%) = 0.7500 n-value = 0.013 Compute by = Known Q Q (cms) = 1.3560 | Diameter(m) = | 0.9000 |
| Slope (%) = 0.7500 n-value = 0.013 Compute by = Known Q | Skipe (%) = 0.7500 n-value = 0.013 Compute by = Known Q Q (cms) = 1.3560 | Inv Elev(m) = | 5.0000 |
| n-value = 0.013 | n-value = 0.013 Compute by = Known Q Q (cms) = 1.3560 Run | Slope (%) = | 0.7500 |
| Compute by = Known O | Compute by = Known Q Q (cms) = 1.3560 Run | n-value = | 0.013 |
| | Q (cms) = 1.3560 Run | Compute by = | Known Q |
| Q (cms) = 1.3560 | Run | Q (cms) = | 1.3560 |
| Run | | | Run |
| | | | |
| | | | |
| | | | |
| | | | |
| | | | |

Input





PRE-DEVELOPMENT PEAK RUN-OFF - 50% AEP

| | ark Stormwater | Simu | lation Run | :2-Year Pr | e | | |
|---|---------------------|-------------------------------------|--------------------------|--|---------------------------------|---------------------|---|
| Start of Run: 01Jan2000, End of Run: 02Jan2000, Compute Time:01May2024, | | 0, 00:00 0, 00:00 4, 16:22:09 | Basin Meteo Contro | Model: rologic Moo ol Specificat | PreD del: 2-Ye tions:Cont | ev ar Pre rol | |
| Show Elements: A | All Elements \vee | Volum | e Units: 🔿 MM | 0 10 | 00 M3 | Sorting: | Watershed Explorer $\ \!$ |
| Hydrologic Element | Drainage (KM | e Area 2) | Peak Discha (M3/S) | arge | Time | of Peak | Volume (1000 M3) |
| Grass | 0.00 | 12 | 0.0031 | | 1 January | 2000, 08 | 0.0498 |
| Gravel | 0.00 | 0.0008 | | | 1 January | 2000, 07 | 0.0727 |
| Parking Asphalt | 0.00 | 0.0005 | | | 1 January | 2000, 07 | 0.0427 |
| Existing Road Asph | alt 0.00 | 0.0005 | | 0.0029 | | 2000, 07 | 0.0427 |
| Concrete | 0.00 | 0.0001 | | 0.0007 | | 2000, 07 | 0.0109 |
| Underground Tanks | s 0.00 | 31 | 0.0148 | | 1 January | 2000, 07 | 0.2608 |
| Outlet | 0.00 | 31 | 0.0148 | | 1 January | 2000, 07 | 0.2608 |

PRE-DEVELOPMENT PEAK RUN-OFF – 20% AEP

| | Project: 21102 | Carpark Stormwate | r Simulation F | Run: 5-Year Pre | 9 |
|--|----------------------|--|--|--|-----------------------|
| Start of Run: 01Jan End of Run: 02Jan Compute Time:01Mar | | n2000, 00:00 n2000, 00:00 ay2024, 16:22:11 | Basin Model: Meteorologic Control Specif | PreDe Model: 5-Yea fications:Contr | ev Ir Pre ol |
| Show Elements: A | ll Elements 🖂 | Volume Units: 🔿 MI | M 🔾 1000 M3 | Sorting: | Watershed Explorer $$ |
| Hydrologic Element | Drainage Ar (KM2) | ea Peak Disc (M3/S | narge Ti) | me of Peak | Volume (1000 M3) |
| Grass | 0.0012 | 0.004 | 9 1 Janu | ary 2000, 08. | 0.0769 |
| Gravel | 0.0008 | 0.006 | 6 1 Janu | ary 2000, 07. | 0.0965 |
| Parking Asphalt | 0.0005 | 0.003 | 9 1 Janu | ary 2000, 07. | 0.0567 |
| Existing Road Aspha | alt 0.0005 | 0.003 | 9 1 Janu | ary 2000, 07. | 0.0567 |
| Concrete | 0.0001 | 0.001 | 0 1 Janu | ary 2000, 07. | 0.0145 |
| Underground Tanks | 0.0031 | 0.020 | 3 1 Janu | ary 2000, 08. | 0.3355 |
| Outlet | 0.0031 | 0.020 | 3 1 Janu | ary 2000, 08. | 0.3355 |

PRE-DEVELOPMENT PEAK RUN-OFF – 1% AEP

Project: 21102 - Carpark Stormwater Simulation Run: 100-Year Pre

| S E C | tart of Run: 0 nd of Run: 0 compute Time:0 | 1Jan2000, 2Jan2000, 1May2024, | 00:00 00:00 16:22:07 | Basin M Meteoro Control | lodel: ologic Mode Specificatic | PreDev l: 100-Ye ons:Contro | ear Pre I |
|-----------------------|--|-------------------------------------|----------------------------|-------------------------------|---------------------------------------|-----------------------------------|-----------------------|
| Show Elements: A | All Elements \vee | Volume | e Units: 🔿 MM | 1 🔾 10 | 00 M3 | Sorting: | Watershed Explorer $$ |
| Hydrologic Element | Drainag (KM | Drainage Area (KM2) | | arge) | Time | of Peak | Volume (1000 M3) |
| Grass | 0.00 | 12 | 0.0125 | 5 | 1 January | 2000, 08. | 0.1833 |
| Gravel | 0.00 | 0.0008 | | L | 1 January | 2000, 07. | 0.1786 |
| Parking Asphalt | 0.00 | 0.0005 | | 0.0071 | | 2000, 07. | 0.1049 |
| Existing Road Asph | alt 0.00 | 0.0005 | | 0.0071 | | 2000, 07. | 0.1049 |
| Concrete | 0.00 | 0.0001 | | 0.0018 | | 2000, 07. | 0.0268 |
| Underground Tanks | s 0.00 | 31 | 0.0406 | 5 | 1 January | 2000, 07. | 0.6198 |
| Outlet | 0.00 | 31 | 0.0406 | 5 | 1 January | 2000, 07. | 0.6198 |



POST-DEVELOPMENT PEAK RUN-OFF - 50% AEP

| | Project: 21102 - Carp | ark Stormwater Simu | lation Run: 2-Year Pos | t |
|-----------------------|---|---|--|-----------------------|
| S E C | Start of Run: 01Jan2000 End of Run: 02Jan2000 Compute Time:01May202 | 0, 00:00 Basin I 0, 00:00 Meteor 4, 16:22:09 Contro | Model: PostDe ologic Model: 2-Year I Specifications:Contro | ev Post I |
| Show Elements: A | Il Elements Volun | ne Units: 🔿 MM 🔇 10 | 000 M3 Sorting: 1 | Watershed Explorer $$ |
| Hydrologic Element | Drainage Area (KM2) | Peak Discharge (M3/S) | Time of Peak | Volume (1000 M3) |
| Asphalt Carpark | 0.0020 | 0.0150 | 1 January 2000, 07 | . 0.2198 |
| Existing Road Asph | alt 0.0005 | 0.0035 | 1 January 2000, 07 | . 0.0517 |
| Gravel | 0.0003 | 0.0024 | 1 January 2000, 07 | . 0.0352 |
| Asphalt Walkway | 0.0002 | 0.0015 | 1 January 2000, 07 | . 0.0220 |
| Grass | 0.0001 | 0.0004 | 1 January 2000, 08 | . 0.0059 |
| Underground Tanks | 0.0031 | 0.0229 | 1 January 2000, 07 | . 0.3579 |
| Outlet | 0.0031 | 0.0229 | 1 January 2000, 07 | . 0.3579 |

POST-DEVELOPMENT PEAK RUN-OFF – 20% AEP

| Project: 21102 - Carpark Stormwater Simulation Run: 5-Year Post | | | | | | | | | | |
|--|------------------------|--------------------------|---------------------|---|--|--|--|--|--|--|
| Start of Run: 01Jan2000, 00:00 Basin Model: PostDev End of Run: 02Jan2000, 00:00 Meteorologic Model: 5-Year Post Compute Time:01May2024, 16:22:10 Control Specifications:Control | | | | | | | | | | |
| Show Elements: A | l Elements 🖂 Volum | ne Units: 🔿 MM 🗿 10 | 000 M3 Sorting: | Watershed Explorer ${\scriptstyle 	imes}$ | | | | | | |
| Hydrologic Element | Drainage Area (KM2) | Peak Discharge (M3/S) | Time of Peak | Volume (1000 M3) | | | | | | |
| Asphalt Carpark | 0.0020 | 0.0198 | 1 January 2000, 07. | . 0.2915 | | | | | | |
| Existing Road Aspha | lt 0.0005 | 0.0047 | 1 January 2000, 07. | . 0.0685 | | | | | | |
| Gravel | 0.0003 | 0.0032 | 1 January 2000, 07. | . 0.0466 | | | | | | |
| Asphalt Walkway | 0.0002 | 0.0020 | 1 January 2000, 07. | . 0.0292 | | | | | | |
| Grass | 0.0001 | 0.0006 | 1 January 2000, 08. | . 0.0090 | | | | | | |
| Underground Tanks | 0.0031 | 0.0303 | 1 January 2000, 07 | . 0.4641 | | | | | | |
| Outlet | 0.0031 | 0.0303 | 1 January 2000, 07. | . 0.4641 | | | | | | |

POST-DEVELOPMENT PEAK RUN-OFF – 1% AEP

Project: 21102 - Carpark Stormwater Simulation Run: 100-Year Post

| Stari End Com | Start of Run:01Jan2000, 00:00Basin Model:PostDevEnd of Run:02Jan2000, 00:00Meteorologic Model:100-YearCompute Time:01May2024, 16:22:06Control Specifications:Control | | | | | | | | | |
|-----------------------|--|--------------------------|--------------------|-----------------------|--|--|--|--|--|--|
| Show Elements: All | Elements 🖂 Volum | e Units: 🔵 MM 🗿 10 | 000 M3 Sorting: V | Vatershed Explorer $$ | | | | | | |
| Hydrologic Element | Drainage Area (KM2) | Peak Discharge (M3/S) | Time of Peak | Volume (1000 M3) | | | | | | |
| Asphalt Carpark | 0.0020 | 0.0362 | 1 January 2000, 07 | 0.5379 | | | | | | |
| Existing Road Asphalt | 0.0005 | 0.0085 | 1 January 2000, 07 | 0.1264 | | | | | | |
| Gravel | 0.0003 | 0.0058 | 1 January 2000, 07 | 0.0861 | | | | | | |
| Asphalt Walkway | 0.0002 | 0.0036 | 1 January 2000, 07 | 0.0538 | | | | | | |
| Grass | 0.0001 | 0.0014 | 1 January 2000, 08 | 0.0208 | | | | | | |
| Underground Tanks | 0.0031 | 0.0557 | 1 January 2000, 07 | 0.8406 | | | | | | |
| Outlet | 0.0031 | 0.0557 | 1 January 2000, 07 | 0.8406 | | | | | | |



APPENDIX B

ENGINEERING DRAWINGS





Project Number:

Project Office: Project Manager: Issue Date:

Drawing Status: For Consent

Oruaiti School Board of Trustees c/o Avail Pacific 2554 State Highway 10, Mangonui Proposed Carpark Upgrades

| | Drawing Index | | | | | | | | | |
|-------|--|-------------|---------------------|--|--|--|--|--|--|--|
| Sheet | Description | Status | Revision / Issue | | | | | | | |
| C001 | Existing Site Plan | For Consent | А | | | | | | | |
| C200 | Earthworks Layout | For Consent | А | | | | | | | |
| C300 | Overall Site Layout | For Consent | А | | | | | | | |
| C301 | Parking Area & Accessway Design Plan Sheet 1 of 2 | For Consent | А | | | | | | | |
| C302 | Parking Area & Accessway Design Plan Sheet 2 of 2 | For Consent | А | | | | | | | |
| C310 | Parking Area & Accessway Longitudinal Sections | For Consent | А | | | | | | | |
| C311 | Parking Area & Accessway Typical Cross Sections | For Consent | А | | | | | | | |
| C312 | Parking Area & Accessway 5m Interval Cross Sections Sheet 1 of 3 | For Consent | А | | | | | | | |
| C313 | Parking Area & Accessway 5m Interval Cross Sections Sheet 2 of 3 | For Consent | А | | | | | | | |
| C314 | Parking Area & Accessway 5m Interval Cross Sections Sheet 3 of 3 | For Consent | А | | | | | | | |
| C320 | Parking Area & Accessway Typical Details | For Consent | А | | | | | | | |
| C400 | Parking Area & Accessway Stormwater Management Plan | For Consent | А | | | | | | | |
| C410 | Parking Area & Accessway Stormwater Longitudinal Sections Sheet 1 of 2 | For Consent | А | | | | | | | |
| C411 | Parking Area & Accessway Stormwater Longitudinal Sections Sheet 2 of 2 | For Consent | А | | | | | | | |
| C420 | Parking Area & Accessway Typical Stormwater Device Details | For Consent | А | | | | | | | |
| C421 | Parking Area & Accessway Typical Stormwater Details | For Consent | A | | | | | | | |



21102

Warkworth Riaan de Swardt 1 May 2024



General Notes:

4

Library

Pool

- All dimensions in metres unless noted otherwise.
- Coordinates are in terms of NZGD Mt Eden 2000. 2. All levels are in metres to NZVD2016. Origin of 3
 - levels RCM XIII SO 50961 RL 11.81m.
- Survey undertaken by Donaldsons; 25 July 2022. Existing titles Pt Allot 18A PSH OF Mangonui East
- 5. Boundaries and are approximate only and are subject to change. Final boundaries will be in 6. accordance with city plan and LINZ requirements.
- Boundaries not for building design purposes. 7 Existing building, roading, and services information 8. may have been obtained from third party records and should be verified on site prior to the commencement of any works. LDE does not in any way guarantee the accuracy of any underground service shown on this plan.
- 9. Contractor to locate / mark / confirm and protect if necessary all existing services prior to commencement of work on site. Any discrepancies to be reported to the Engineer.
- 10. Flood extent extracted from NRC Natural Hazards and shown indicatively for information only.

General Legend: ____

 Boundary (Surveyed) - 1m Major Contour 5m Minor Contour

NRC River Flood Hazard Zone - Regionwide Models

| 10 Year Exent |
|------------------------|
| 50 Year Extent |
| 100 Year CC Extent |

Existing Services Legend:

| | | | | _ | — |
|---|---------------|---------------|---------------|---------------|--------------|
| 8 | **** | *** | **** | *** | \bigotimes |
| | SW | | | sw — | _ |
| | SW | \langle | \$ | | |
| | | | | | |
| ⇒ | \rightarrow | \rightarrow | \rightarrow | \rightarrow | _ |
| | -ww | | | ww— | |
| | — он | | | он — | |
| | | | Γ | | |
| _ | | | | | _ |
| | | - | | - | |

Edge of Road Asphalt Surface Stormwater Pipe Stormwater Manhole, Wingwall, Scruffy Dome, Catchpit Open Drain Wastewater Pipe Power Pole & Over Head Lines Transformer Fence **Retaining Wall**

| 2/ | | | | ° | 5 | 10 | 15 | 5 2 | 20 | 25 |
|----|-------|------------|-----------|-------|----------|------|-------------------|----------------------------|------------------------|-------------------|
| | | | | Scale | 1:500 | | (m) | (| Original siz | e = A3 |
| | | | Design: | R. d | e Swardt | Proj | ect status: Fo | r Con | sent | |
| | | | Drawn: | R. d | e Swardt | Proj | ect | | 00111 | |
| | | | Approved: | J. \$ | Simson | | | 2110 | 2 | |
| | J. S | 1 May 2024 | Scale A3: | 1 | 1:500 | Dra | | 2001 | | A |
| | Aprvd | Date | | | | | Drive | Location C., Saved by Rise | nDeSwardt, Document ID | 362062, Vension 6 |



General Notes:

- . All dimensions in metres unless noted otherwise.
- Coordinates are in terms of NZGD Mt Eden 2000.
 All levels are in metres to NZVD2016. Origin of
- levels RCM XIII SO 50961 RL 11.81m. Survey undertaken by Donaldsons: 25
- Survey undertaken by Donaldsons; 25 July 2022.
 Existing titles Pt Allot 18A PSH OF Mangonui East
- Existing trues Pt Allot 18A PSH OF Mangonul Ear
 Boundaries and are approximate only and are subject to change. Final boundaries will be in coordenee with city plan and UNZ
- accordance with city plan and LINZ requirements.7. Boundaries not for building design purposes.9. Eviding building and bui
- Existing building, roading, and services information may have been obtained from third party records and should be verified on site prior to the commencement of any works. LDE does not in any way guarantee the accuracy of any underground service shown on this plan.
- Contractor to locate / mark / confirm and protect if necessary all existing services prior to commencement of work on site. Any discrepancies to be reported to the Engineer.
- Flood extent extracted from NRC Natural Hazards and shown indicatively for information only.

Earthworks Notes:

- All works to be undertaken in accordance with FNDC Engineering Standards, GD05 Guidelines and the Health and Safety at Work Act 2015 (HSWA).
- All disturbed public areas to be made good and restored to previous condition.
- All earthworks to be undertaken during periods of fine weather.
- Earthworks are to be undertaken and completed in within the minimum possible time frame.
- The area is to be progressively finished as areas are brought to grade.
- The earthworks volumes shown is a solid measure.
- The earthworks volumes has been calculated in relation to the existing and final ground levels.
- Topsoil removal volumes is excluded in the total earthworks volumes.
- Additional box cut volumes has not been included in the earthworks volumes.

Erosion and Sediment Control Notes:

- The contractor is solely responsible for ensuring the the site has effective erosion control and sediment retention measures operating at all times.
- All erosion and sediment control measures must be operational prior to any other works commencing on site.
- All sediment control measure are to comply with GD05 and FNDC requirements.

| Earthworks Cut / Fill Volumes | | | | | | | | |
|-------------------------------|----------------------|--|--|--|--|--|--|--|
| Site Area | 41,210m ² | | | | | | | |
| Earthworks Area | 2,525m² | | | | | | | |
| Cut Volume | 18m³ | | | | | | | |
| Fill Volume | 230m³ | | | | | | | |
| Net Fill Volume | 212m³ | | | | | | | |

| | | | | 0 5 | 10 | 15 | 20 | 25 |
|------|---------------|--------------------|-----------|--------------|---------------|------|-------------|------------|
| / | | | | Scale 1:500 | (m | ı) | Original si | ze = A3 |
| | | | Design: | R. de Swardt | | | Consent | |
| | | | Drawn: | R. de Swardt | Project: | 1010 | | |
| | | | Approved: | H. Dubinko | . Dubinko 211 | | 102 | Icoue/Rour |
| sion | H. D Aprvd | 1 May 2024 Date | Scale A3: | 1:500 | Drawing iv | C20 | 00 | A |



Proposed Services Notes:

- All works to be undertaken in accordance with FNDC Engineering Standards and the Health and Safety at Work Act 2015 (HSWA).
- All dimensions in metres unless noted otherwise. 2.
- Contractor to locate / ml dimensionark / confirm and 3. protect if necessary all existing services prior to commencement of work on site.
- 4 All disturbed public areas to be made good and restored to existing condition.
- Manholes to be reinforced concrete chambers with a 5. precast base. Min Ø1050mm manhole unless otherwise specified.
- 6. Catchpits to be 675 x 450mm reinforced concrete chambers, with trafficable frame & grid inlet lid.
- All PVC stormwater pipes to be uPVC SN16.
- All concrete stormwater pipes to be RCRRJ Class 4. 8 Pipe bedding and installation to comply with relevant 9 council details.
- 10. All lane and parking markings as per MOTSAM.
- 11. Parking bays as per Appendix 3D of the Far North District Plan (FNDP).

Reference Notes:

- Refer to C301 & C302 for Parking Area Layouts.
- Refer to C310 for Parking Area and Accessway Longitudinal Sections Layout.
- Refer to C311 to C314 for Cross Section Layouts.
- Refer to C320 for Roading details.
- Refer to C400 for Stormwater Layout.
- Refer to C410 to C411 Stormwater Longitudinal Section Layouts.
- Refer to C420 & C421 for Stormwater details.

Existing Services Legend:

| | Boundary (Surveyed) |
|---|-------------------------------|
| | Edge of Road |
| sw sw | Stormwater Pipe |
| 👓 🤇 🏶 🛛 | Stormwater Manhole, Wingwall, |
| | Scruffy Dome, Catchpit |
| $\rightarrow \rightarrow \rightarrow \rightarrow \rightarrow -$ | Open Drain |
| — он — ф он — | Power Pole & Over Head Lines |
| T | Transformer |
| | Fence |
| -0-0-0-0-0-00 | Retaining Wall |
| | |

Proposed (Prop.) Services Legend:





& ENGINEERING

- 0.5m Major Contour 0.1m Major Contour Kerb Block - Kerb & Channel Flush Edge Beam Asphalt Surface Proposed Concrete Surface Proposed Gravel Surface Rock Filled Traffic Island Stormwater Pipe Stormwater Perforated Pipe Stormwater Manhole SW Manhole with Scruffy Dome Stormwater Catchpit Stormwater Rodding Point Stormwater Swale Fence

| | | 0 5 | 10 | 15 | 20 | 25 | |
|----|--------------|-----------|--------------|----------------|------------------------|-----------------------------------|-----------------|
| / | | | Scale 1:500 | (m) | | Original siz | e = A3 |
| | _ | Design: | R. de Swardt | Project status | Eor Co | onsent | |
| | | Drawn: | R. de Swardt | Project: | Project: | | |
| | | Approved: | H. Dubinko | Drawing No: | 21102 | | losuo/Pour |
| H. | D 1 May 2024 | Scale A3: | 1:500 | Drawing No. | C300 | 0 | A |
| П | Vu Date | | | | Drive Location C: Save | of her Disso CarSecord, Company D | accord Gambra G |





| | | Pro | posed S | ervic | es Leger | nd | : | | | |
|----------------|-----|---------------------|-----------------------------|----------------|--------------|-----|-----------------|---------|----------|-----------|
| ed) | | | | | Asphalt S | Sur | face | | | |
| | | | | | Proposed | d C | Concrete | Surfa | се | |
| | | | | | Proposed | 9 6 | Gravel S | urface | | |
| ole, Wingwall, | | 62 | 9493 | <u> 9</u> 2 | Rock Fille | ed | Traffic I | sland | | |
| chpit | | | | | Stormwat | ter | Perfora | ted Pi | pe | |
| rlloodlingo | | | (ev) (A) | | Stormwai | ter | Manho | e | Domo | |
| r Head Lines | | | ₩ ₩ | | Stormwat | tor | Catchn | it | Dome | |
| | | | R | | Stormwat | ter | Roddin | a Poin | t | |
| | | → - | $\rightarrow \rightarrow -$ | → | Stormwat | ter | Swale | 5 | | |
| ır | | | | | Fence | | | | | |
| ur | | | | | | | | | | |
| | | | | | | | | | | |
| nnel | | | | | | | | | | |
| Beam | | | | | | | | | | |
| | | Roa | d Marki | nala | aand. | | | | | |
| | | Nua | | | egenu. | | | | | |
| sing | | | • •••• • | | 1650mm I | Ru | bber Wi | ieel St | top | |
| Wide, 0.6m G | iap | | Ö . | | Yellow Di | sa | bled Pa | rking S | Sign | |
| | | $\overline{\nabla}$ | \overline{III} | \overline{Z} | Yellow No | o-F | Parking | sland | | |
| | | | | | | | | | | |
| | | | | | | | | | | |
| | | | | | | | | | | |
| | | | | | | | | | | |
| W | | | | | | | | | | |
| | | | | | | | | | | |
| row | | | | | | | | | | |
| ineation Line | | | | | | | | | | |
| | | | | | | | | | | |
| nes Gap | | | | | | | | | | |
| | | | | | | | | | | |
| | | | | | | | | | | |
| | | | | | | | | | | |
| | | | | | | | | | | |
| | | | | | | | | | | |
| | | | | | | | | | | |
| | | | | | | | | | | |
| | | | | | | | | | | |
| | | | | | | | | | | |
| | | | | | | | | | | |
| | | | | | | | | | | |
| | | | | | | | | | | |
| | | | | | | | | | | |
| | | | | | | | | | | |
| | | | | | | | | | | |
| | | | | | | | | | | |
| | | | | | | | | | | |
| | | | | | | | | | | |
| | | | | | | | | | | |
| | | | | | | | | | | |
| | | | | | | | | | | |
| | | | | | | | | | | |
| | | | | | | | | | | |
| | | | | | | | | | | |
| | | | | | | | | | | |
| | | | | | | | | | | |
| | | | | | | | | | | |
| | | | | 0 | 2.5 | ļ | 5 7 | .5 | 10 | 12.5 |
| | | | | Scal | e 1:250 | | (m) | | 0 | size - ** |
| | | | Design: | | t de Swardt | | Project status: | | Uriginal | size = A3 |
| | | | D D D | | | - | F | or Co | nsen | t |
| | | | Drawn: | F | k. de Swardt | _ | Project: | 211 | 02 | |
| | | | | | LL Durkislas | - 1 | | | | |

H. Dubinko

1:250

Scale A3

Α

C302

| Vortical Exagonation: 2.5 | | | | | | | | |
|----------------------------|----|----------|--------|--------|-----|-----|--|--|
| Vertical Exaggeration. 2.5 | | | | | | | | |
| Datum: 0.0 | | | | | | | | |
| | | | | | | | | |
| | | | | | | | | |
| | | - | | | - | - | | |
| Design Levels | | 00 | 34 | 04 | 36 | 20 | 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0 | |
| Design Levels | | 5. | 5 | D | 5. | 5 | 2 | |
| | | | | | | | | |
| | | | | | | | | |
| | 9 | 2 | ດ | - - | 0 | 4 | <u>o</u> | |
| Existing Levels | 6. | <u>.</u> | N N | 4 | 2 | N N | 0 | |
| - | 4 | Ω. | ى ى | L) L) | 2 | Ω. | വ | |
| | | | | | | | | |
| | | - | | | | | | |
| Cut/Fill Depth | | 6 | 05 | 80 | 10 | 05 | 6 | |
| | | ö | ö | i õ | | ö | o. | |
| | | - | - | - | - | | | |
| | | | | | | | | |
| | | õ | | | 2 | Q | | |
| Chainage | | 0.0 | | 0.0 | 0.0 | 0.0 | | |
| - | | 5 | - ÷ | ы М | 5 | ж |) ř | |
| | l | | | | | | | |

Long Section (ACCESSWAY CL)

| | _^ | | + | | | | | | | | | | | | | | | |
|----------------------------|-----|-----|-----|-----|------|-----|-----|-----|----------|----------|------|-----|-------|-----|-----|-----|------|-----|
| | | | | | | | | | + | | | | | | | | | |
| | | | | | | | | | | | | | | | | | | |
| | | | | | | | | | | | | | | | | | | |
| | | | | | | | | | | | | | | | | | | |
| | | | | | | | | | | | | | | | | | | |
| | | | | | | | | | | | | | | | | | | |
| | | | | | | | | | | | | | | | | | | |
| | | | | | | | | | | | | | | | | | | |
| | | | | | | | | | | | | | | | | | | |
| | | | | | | | | | | | | | | | | | | |
| | | | | | | | | | | | | | | | | | | |
| Vertical Exaggeration: 2.5 | | | | | | | | | | | | | | | | | | |
| Datum: 0.0 | | | | | | | | | | | | | | | | | | |
| | | | | | | | | | | | | | | | | | | |
| | • | | | 10 | - | • | | 10 | + | | 10 | ~ | 0 | | | 10 | ~ | ~ |
| Design Levels | 5 | 30 | 5 | 5 | Ņ | ÷. | | 4 | ő | .97 | 0 | ö | Ő | | 5 | .36 | .56 | 12 |
| _ | 4 | 4 | 4 | 4 | 4 | 4 | 4 | 4 | 4 | со (| 4 | 4 | 4 | 4 | 4 | 4 | 4 | 4 |
| | | | | | | | | | | | | | | | | | | |
| Existing Levels | 29 | 30 | 27 | 52 | 5 | 19 | 17 | 15 | 2 | 67 | 02 | 08 | 60 | 12 | 17 | 35 | 58 | 77 |
| | 4 | 4 | 4 | 4 | 4 | 4 | 4 | 4 | 4 | τi C | 4 | 4 | 4 | 4 | 4 | 4 | 4 | 4 |
| | | | | | | | | | | | | | | | | | | |
| Cut/Fill Depth | 8 | 0 | | 8 | 8 | 8 | 0 | 8 | 8 | 8 | 8 | 8 | 0 | 8 | 8 | 00 | 00 | 8 |
| | o.o | 0.0 | i o | Ö.Ö | 0.0 | 0.0 | 0.0 | 0.0 | Ö.Ö | 0.0 | 0.0 | 0.0 | 0.0 | ö | 0.0 | 0.0 | 0.0 | Ö.Ö |
| | | | | | | | | | | | | | | | | | | |
| | 0 | E E | | 0 | 8 | 0 | g | 8 | 8 | g | 8 | g | 0 | | 0 | 0 | 0 | g |
| Chainage | 2.0 | | 2.0 | 0.0 | 25.0 | 0.0 | 5.0 | - O | 5.0 | 0.0 | 55.0 | 0.0 | 52 (C | 0.0 | 5.0 | 0.0 | 35.0 | 0.0 |
| | | | | N | N | ო | ო | 4 | 4 | LO LO | cu | Ű | Ű | | 2 | ω | ω | 0 |

Long Section (PARKING AREA CL)

Copyright: LDE All rights reserved / Do not scale off drawings / Confirm all dimensions on site prior to work Client

Oruaiti School Board of Trustees c/o Avail Pacific

2554 State Highway 10, Mangonui Proposed Carpark Upgrades

Project

Drawing Title Parking Area & Accessway Longitudinal Sections



| | | | | Oliginal Size - Ac |
|------|------------|------------|----------------|----------------------|
| | | Design: | R. de Swardt | For Consent |
| | | Drawni | D de Swardt | |
| | | Diawii. | R. de Swardt | Project: |
| | | Approved: | H. Dubinko | 21102 |
| | | rippiorod. | The Bradelinko | Drawing No: Issue/Re |
| H. D | 1 May 2024 | Scale A3: | 1:250 | C310 A |



| | | | | | ě |
|--------|-------|------------|-----------|--------------|--------------------------------|
| | | | Design: | R. de Swardt | Project status: For Consent |
| | | | _ | | |
| | | | Drawn: | R. de Swardt | Project: |
| | | | | | 01100 |
| | | | Approved: | H Dubinko | 21102 |
| | | | Appioreu. | TI: Dubinito | Drawing No: Issue/Rev |
| | | | | | |
| | H. D | 1 May 2024 | Scale A3: | 1:75 | I C311 IA |
| vision | Aprvd | Date | | | |











Copyright: LDE All rights reserved / Do not scale off drawings / Confirm all dimensions on site prior to work Client

Oruaiti School Board of Trustees c/o Avail Pacific

Project 2554 State Highway 10, Mangonui Proposed Carpark Upgrades

Drawing Title Parking Area & Accessway 5m Inteval Cross Sections Sheet 1 of 3



A

C312

H. D 1 Scale A3: May 2024

Approved:

1:250



Oruaiti School Board of Trustees c/o Avail Pacific

2554 State Highway 10, Mangonui Proposed Carpark Upgrades

Drawing Title Parking Area & Accessway 5m Inteval Cross Sections Sheet 2 of 3





с |RL:-4.52 0-







| | | | Design: | R. de Swardt | Project status: For Consent |
|--------|-------|------------|-----------------------|--------------|--|
| | | | | | |
| | | | Drawn: | R. de Swardt | Project: |
| | | | | | 21102 |
| | | | Approved [.] | H Dubinko | 21102 |
| | | | rippiorou. | | Drawing No: Issue/Rev: |
| | | | | | |
| | H. D | 1 May 2024 | Scale A3: | 1:250 | I 6313 IA |
| rision | Aprvd | Date | | | |
| | | | | | Drive Location C., Saved by RisanDeSvaardt, Document ID 455509, Version 14 |











Copyright: LDE All rights reserved / Do not scale off drawings / Confirm all dimensions on site prior to work Client

Oruaiti School Board of Trustees c/o Avail Pacific

2554 State Highway 10, Mangonui Proposed Carpark Upgrades

Project

Drawing Title Parking Area & Accessway 5m Inteval Cross Sections Sheet 3 of 3







| | | | | | Original size = A3 |
|--------|---------------|--------------------|-----------|--------------|--------------------------------|
| | | | Design: | R. de Swardt | Project status: For Consent |
| | | | Drawn: | R. de Swardt | Project |
| | | | Approved: | H. Dubinko | 21102 |
| rision | H. D Aprvd | 1 May 2024 Date | Scale A3: | As Shown | C320 A |

- Safety at Work Act 2015 (HSWA).
- the FNDC Engineering Standards (ES).
- Standards as stated in the FNDC ES.
- Contractor to locate / ml dimensionark / confirm and protect if necessary all existing services prior to commencement of work on site.
- 6. All disturbed public areas to be made good and restored to existing condition.
- with a precast base. Min Ø1050mm manhole unless otherwise specified.
- chambers, with trafficable frame & grid inlet lid.

- Trap", installed as per supplier specification.
- is to be re-laid to SWMH6 and connected to

- Scruffy Dome, Catchpit
 - Transformer

- 0.5m Major Contour 0.1m Major Contour ---- Prop. Kerb Block







Long Section (SWCP1 TO SWMH3)





Long Section (SWCP7 TO SWMH3)

Copyright: LDE All rights reserved / Do not scale off drawings / Confirm all dimensions on site prior to work Client

Project 2554 State Highway 10, Mangonui Proposed Carpark Upgrades Drawing Title Parking Area & Accessway Stormwater Longitudinal Sections Sheet 1 of 2



Original size = A3

| | | | Design: | R. de Swardt | Project status: For Consent |
|--------|---------------|--------------------|-----------|--------------|--------------------------------|
| | | | Drawn: | R. de Swardt | Project: |
| | | | Approved: | H. Dubinko | 21102 |
| vision | H. D Aprvd | 1 May 2024 Date | Scale A3: | 1:250 | C410 A |



Long Section (SPORTS FIELD CATCHPIT TO TANK)



Long Section (DRAINAGE TRENCH)

Copyright: LDE All rights reserved / Do not scale off drawings / Confirm all dimensions on site prior to work Client

Oruaiti School Board of Trustees c/o Avail Pacific

Project 2554 State Highway 10, Mangonui Proposed Carpark Upgrades

Drawing Title Parking Area & Accessway Stormwater Longitudinal Sections Sheet 2 of 2



| | | | | | Oliginal size – Ab |
|--------|-------|------------|-----------|--------------|--------------------------------|
| | | | Design: | R. de Swardt | Project status: For Consent |
| | | | Drawn: | R. de Swardt | Project: |
| | | | Approved: | H. Dubinko | 21102 |
| | _ | | | | Drawing No: Issue/Rev |
| islan | H. D | 1 May 2024 | Scale A3: | 1:250 | C411 A |
| rision | Aprvd | Date | | | 1 1 |





| | B 200 for pipes up to | 300 dia. |
|---|---|--|
| Opening to be over for sanitary sewer only. | outlet | 00 dia Conci 25mr |
| ME | B SECTION CC | uP.V.C. ar Fran |
| <u>NOTES:</u> 1. This de | stail is applicable for pipe diameters | riser bedo |
| up to 6 2. 150mm iron fro & bern iron fro | 300mm & for manhole depths up to 5.0m. n thick reinforced concrete lids with heavy duty d ames & covers to be used in driveways, carriage ns. 100mm thick concrete lids with light duty ca ames & covers may be used elsewhere. | Mort luctile arou eways st |
| Precast with mi No add | t manhole bases shall be used in all instances inimum sized holes cut for pipe entry. ditional thin plastering of benching or benching of | = 20 |
| 5. All con | is permitted. Increte to be 20MPa. | 00 |
| manhol bond b should 8. All mar | les, then a gritted starter pipe shall be installed between manhole and pipe material. Also, a 3flexi be specified as part of gritted starter pipe (as s nholes >1.2m in depth shall be provided with ma | to allow ble joint standard). |
| rungo. IIIt | ese shall follow the requirements on sheet 40 | innoic step |
| Tungs. The | ese shall follow the requirements on sheet 40 | innoie step |
| | 200mm London junction | innoid Step |
| | 200mm London junction Epoxy mortar around pipe | innoid Step |
| Tungs. The | 200mm London junction Epoxy mortar around pipe Short - no longer than 1000mm | itandard step at 300m |
| | 200mm London junction Epoxy mortar around pipe Short - no longer than 1000mm Dropper pipe for sewer only (single piece) | itandard step at 300m Bolt h mortar sec Sheet 40 |
| | 200mm London junction Epoxy mortar around pipe Short - no longer than 1000mm Dropper pipe for sewer only (single piece) Where the height of drop exceeds 1.5m the dropper pipe shall be rigidly attached to the wall of the manhole with approved Grade 316 SS or plastic supports at 1.5m centres | itandard step at 300m Bolt h mortar sea Sheet 40 ortar around max. |
| SECT | 200mm London junction Epoxy mortar around pipe Short - no longer than 1000mm Dropper pipe for sewer only (single piece) Where the height of drop exceeds 1.5m the dropper pipe shall be rigidly attached to the wall of the manhole with approved Grade 316 SS or plastic supports at 1.5m centres 88' bend set in benching -Half pipe channel (ceramic or uPVC) TON A-A INTERNAL DROP | itandard step at 300m Bolt h mortar sec Sheet 40 ortar around max.)mm layer o)MPa concrei |
| SECT Note: 1. This det manhole 2. External | ese shall follow the requirements on sheet 40 200mm max Epoxy mortar around pipe Short - no longer bropper pipe for sewer only (single piece) Where the height of drop exceeds 1.5m the dropper pipe shall be rigidly attached to the wall of the manhole with approved Grade 316 SS or plastic supports at 1.5m centres 88' bend set in benching -Half pipe channel (ceramic or uPVC) TON A-A INTERNAL DROP cail is applicable for pipe diameters up to 250mm depth up to 5.0m & for manhole diameters ≥ drops shall not be used | Standard step at 300m Bolt h mortar sec Sheet 40 ortar around max.)MPa concrei n & for 1200mm. |

ENGINEERING STANDARDS

Issued for Consent Issue/Rev

Copyright: LDE All rights reserved / Do not scale off drawings / Confirm all dimensions on site prior to work

Oruaiti School Board of Trustees c/o Avail Pacific

2554 State Highway 10, Mangonui Proposed Carpark Upgrades

Project

Parking Area & Accessway Typical Stormwater Details



| | | | Design: | R. de Swardt | For Consent | |
|--------|-------|------------|-----------|--------------|---------------------|-----|
| | | | Drawn: | R. de Swardt | Project: | |
| | | | Approved: | H. Dubinko | 21102 | |
| | H. D | 1 May 2024 | Scale A3: | As Shown | Drawing No: C421 | A A |
| vision | Aprvd | Date | | | ••• | |

APPENDIX C

SUPPLIER INFORMATION AND TECHNICAL DRAWINGS





Low-cost catchpit insert for removal of litter and gross pollutants from run-off

Litter, leaves and other gross pollutants such as cigarette butts wash into our storm drains daily, and are responsible for degradation of our streams, rivers, lakes and beaches. Stormwater runoff from existing drainage network largely flows untreated into our marine environment and negatively effects the water quality, land value and marine life. LittaTrap is a low-cost solution to remove these pollutants.

The LittaTrap is an award-winning catchpit insert designed to be easily retrofitted into new and existing stormwater catchpits to specifically target litter, leaves and gross pollutants, capturing them at source and preventing them from accumulating in our waterways.

The LittaTrap is very cost effective and easy to maintain. Simply lift the filterbag out of the catchpit, empty and then replace back into the frame. This efficient solution captures and removes plastics and other pollutants before they reach the stormwater outfall.

The LittaTrap is also effective as a pre-treatment device for use in a treatment train with hydrodynamic separators, filtration devices, ponds and wetlands. In many cases, it is often the most practical solution for retrofits.



HOW DOES IT WORK?

As stormwater enters a storm grate or catchpit, it passes through the filterbag. Litter, debris, and other pollutants larger than the filterbag aperture are captured and retained.

If the filterbag is full or during high flows, the overflow is released through the overflow apertures in the frame assembly.

DESIGN AND OPERATION

The LittaTrap is fixed by brackets to the catchpit precast concrete wall below the inlet grate allowing the filter insert to be easily lifted out by hand for maintenance. Consisting of a structural bracket assembly with flow diverter seal and a removable filterbag insert which allows for high flow bypass, the LittaTrap is simple to install and remove.

Once the filter insert has been removed, open access to the base of the catchpit and outlet pipe means any accumulated sediment in the pit can easily be removed by inductor truck if required.

Standard sizes fit the common precast concrete regular and curb entry catchpits. Custom designs may be fabricated for non-standard catchpits.

CAPABILITIES

- Captures litter, leaves, debris and gross pollutants before they enter the drainage system
- Fits a range of catchpit sizes ideal for retrofits
- Easy access maintenance friendly design, can be hand maintained
- Bypasses high flows with no moveable parts
- Adjustable flow diverter allows fine-tuning during installation for a perfect fit
- Third party tested at Auckland University

BENEFITS

- Can be used to target high litter areas
- No construction means low capital costs
- Ideal pre-treatment device for filters, ponds, wetlands or other stormwater management and WSD practices
- A range of filterbag sizes to target gross pollutants
- Hand maintainable
- No confined space entry



EASY MAINTENANCE

The LittaTrap is very cost-effective and easy to maintain as the filterbag can be simply picked up and emptied by hand. As the LittaTrap is hand maintainable, it can be easily cleaned by site owners or maintenance crew without the need for vacuum inductor trucks or heavy lifting equipment. The pits can be visually inspected and accessed from grade, so there is no requirement for confined space entry, reducing maintenance costs.

LittaTrap filters should be inspected periodically to determine maintenance frequency. The frequency of maintenance services should be reviewed at the completion of each clean and modified if pollutant loadings deem this necessary. At the required maintenance interval the contaminants need to be removed from the filterbags and disposed of appropriately.

The maintenance crew is responsible for the disposal of debris in accordance with all applicable regulations and is responsible for following all Health and Safety requirements.

Contact the Stormwater360 maintenance department at **maintenance@Stormwater360.co.nz** for more information.

HAND MAINTENANCE IS AS EASY AS 'LIFT, TIP, REPLACE'

- 1. Lift LittaTrap out of pit using handles
- 3. Tip contents out of LittaTrap into suitable receptacle for contents
- 2. Replace LittaTrap securely back into surrounding frame and seal









Nothing in this brochure should be construed as an expressed warranty or an implied warranty of merchantability or fitness for any particular purpose. See the Stormwater360 standard quotation or acknowledgement for applicable warranty earn do ther terms and conditions of sale.

STORMFILTER^M

High efficiency / low maintenance stormwater filter.

SIPHON-ACTUATED FILTRATION The stormwater management StormFilter[™] cleans stormwater through a patented passive filtration system, effectively removing pollutants to meet the most stringent regulatory requirements. Highly reliable, easy to install and maintain, and proven performance over time, StormFilter[™] products are recognised as a versatile BMP for removing a variety of pollutants, such as sediments, oil and grease, metals, organics, and nutrients. These systems come in variable configurations to match local conditions and come with prolonged maintenance periods to ensure long-term performance and reduce operating costs.

HOW DOES IT WORK?

During a storm, runoff passes through the filtration media and starts filling the cartridge center tube. Air below the hood is purged through a one-way check valve as the water rises. When water reaches the top of the float, buoyant forces pull the float free and allow filtered water to drain.

After the storm, the water level in the structure starts falling. A hanging water column remains under the cartridge hood until the water level reaches the scrubbing regulators. Air then rushes through the regulators releasing water and creating air bubbles that agitate the surface of the filter media, causing accumulated sediment to drop to the vault floor. This patented surfacecleaning mechanism helps restore the filter's permeability between storm events.

PROVEN PERFORMANCE

- First independently verified filter by Washington Department of Ecology, New Jersey Department of Environmental Protection and USEPA's Environmental Technology Verification program
- Approved and accepted by most regulatory bodies across New Zealand
- Over 16,800 x StormFilter's[™] cartridges installed throughout New Zealand
- Continual innovation Design refined and perfected over two decades of research and experience



STORMFILTER[™] VAULT



STORMFILTER BENEFITS

UNDERGROUND SYSTEMS MAXIMISE PROFITABILITY

- Save land space allowing denser developments reducing sprawl
- Add parking spaces and increase
 building size, increasing profitability
- Compact design reduces construction and installation costs by limiting excavation

RELIABLE LONGEVITY & LOWER MAINTENANCE COSTS

- Self cleaning hood prevents surface blinding, ensures use of all media and prolongs cartridge life
- 1-3 year maintenance cycles
- 20 years maintenance experience
- Minimal or no standing water. Lower disposal costs

CONTACT DETAILS

Stormwater360

FREEPHONE: 0800 STORMWATER (0800 786769)

www.stormwater360.co.nz



Nothing in this brochure should be construed as an expressed warranty or an implied warranty of merchantability or fitness for any particular purpose. See the Stormwater360 standard quotation or acknowledgement for applicable warranties and other terms and conditions of sale.





SUPERIOR HYDRAULICS

Multiple cartridge heights gives design solutions for site restraints.



Other hydraulic benefits

- Hydraulic effect as low as 350mm head loss
- Zero surcharge of inlet pipe unlike upward flowing filters
- Can be designed for tail water e.g tidal conditions
- Online and offline configurations can limit hydraulic effects

MEDIA CHOICES

Our filtration products can be customised using different filter media to target site-specific pollutants. A combination of media is often recommended to maximise pollutant removal effectiveness.



Perlite is naturally occurring puffed volcanic ash. Effective for removing TSS, oil and grease.



ZPG[™] is a multi-purpose media option approved for highly trafficked sites or sites with high metal loadings. ZPG is a mixture of Zeolite, Perlite and GAC (granular activated carbon). ZPG is ideal for removing soluble metals, TSS, oils and grease, organics and ammonium.



Zeolite is a naturally occurring mineral used to remove soluble metals, ammonium and some organics.



GAC (Granular Activated Carbon) has a micro-porous structure with an extensive surface area to provide high levels of adsorption. It is primarily used to remove oil and grease and organics such as PAHs and phthalates.

CONFIGURATION

Stormfilter's can be configured in any drainage structure. Please contact SW360 for a customised design.



PRECAST VAULT

- Treats medium sized sites
- Simple installation arrives on-site fully assembled
- Plug and play solution

PRECAST MANHOLE

- Provides a low drop, point-of-entry configuration
- Uses drop from the curb inlet to the conveyance pipe to drive the passive filtration cartridges
- No crane required (Hi-AB lifting for most sizes)
- 1050-2400mm diameter sizes available



- Combines off-line bypass and StormFilter[™] pollutant removal into one structure
- Internal weir allows high peak flows with low hydraulic head losses
- Approved and accepted as offline configuration by councils around NZ
- Eliminates cost of additional structures to bypass peak flows
- Reduces the overall footprint of the treatment system



0800 STC





HIGH FLOW

- Treats flows from large sites
- Consists of large, precast components designed for easy assembly on-site
- Several configurations available, including: Panel Vault, Box Culvert, or Cast-In-Place



SCALE : NTS

| 3Y CUSTOMER / CONTRACTOR | | | | | | | | | | |
|---------------------------------------|----------------------|-----------------------------|-----------------------------|-----------------------------|--|--|--|--|--|--|
| | P.O. NUMBER : | | | | | | | | | |
| · · · · · · · · · · · · · · · · · · · | | | | | | | | | | |
| | | | | | | | | | | |
| | | | | | | | | | | |
| | | | | | | | | | | |
|) 0 RRJ) | Ø 350 (DN300 PVC) | Ø 400 (DN300 RRJ) | Ø 500 (DN375 RRJ) | Ø 600 (DN450 RRJ) | | | | | | |
| T PIPE | (IL) : | | DTI : | | | | | | | |
|): | | | DATE : | | | | | | | |
| | | | | | | | | | | |

TO BE COMPLETED BY SW360

| | ZPG | OTHER : | | | | |
|----------|---------------------------------|---------------------------|------------|--|--|--|
| | | PRE-INSTALLATIO | ON (Y/N) : | | | |
|)) RD | 3 QTR (Ø 24.0 ID) WHITE/OPAL | HALF (Ø 19.7 ID) GREEN | OTHER : | | | |
| WEB | -FORGE / CLASS D | OTHER : | | | | |
| | | DATE : | | | | |
| | | | | | | |

NOTES

- MANHOLE UNIT FITTED WITH SWIFTLIFT ANCHOR POINTS (QTY 2). DO NOT EXCEED 60 DEGREE LIFT ANGLE. CONCRETE LID FITTED WITH SWIFTLIFT ANCHOR POINTS (QTY 4).
- UNIT SUPPLIED WITH OUTLET CORÉ DRILLED. INLET/S CORE DRILLED ON SITE BY OTHERS WITHIN RANGE SHOWN ON DRAWING. SEALING / GROUTING OF MANHOLE COMPONENTS AND PIPES BY CONTRACTOR. ENSURING LOCAL CODES AND REGULATIONS ARE
- COMPLIED WITH. ANY RISERS REQUIRED TO INCREASE THE DEPTH TO INVERT (DTI) FROM THAT AS DRAWN TO BE SUPPLIED BY THE CONTRACTOR. FOR A DTI EXCEEDING 5m PLEASE CONTACT 0800STORMWATER CONTRACTOR TO TAKE APPROPRIATE MEASURES TO PROTECT CARTRIDGES FROM CONSTRUCTION RELATED EROSION RUNOFF. BACKFILL, BEDDING AND BUOYANCY DESIGN BY ENGINEER OF RECORD
- QTY OF CARTRIDGES BY ENGINEER OF RECORD.
- CONCRETE MANHOLE RISERS ARE DESIGNED AND MANUFACTURED IN ACCORDANCE WITH AS/NZS 4058 : 2007
- 10. CONCRETE MANHOLE BASES ARE DESIGNED AND MANUFACTURED IN ACCORDANCE WITH NZS 3101 : 2006 & NZS 3109 : 1997
- 11. CONCRETE LID DESIGNED AND MANUFACTURED TO HN-HO-72 12. FOR REQUIREMENTS OUTSIDE OF DRAWING SPECIFICATIONS PLEASE
- CONTACT 0800STORMWATER.

CKD :

APPROX WEIGHTS

MANHOLE SECTION INCLUDING CARTRIDGES : 4050 Kg (AS DELIVERED, BASED ON QTY 7 ZPG CARTS) LID WEIGHT : 1850 Kg

| | | | REV | REVISION DETAIL | DATE |
|------|------|----------|-----|-----------------|----------|
| | | | 0 | APPROVED | 01.03.19 |
| | | | 1 | MARK-UPS | 09.04.19 |
| R.P. | | 01.03.19 | | | |
| G.S. | G.Y. | 01.03.19 | | | |



| BY CUSTOMER / CONTRACTOR | | | | | | | |
|--------------------------|---------------|---|---------------|------|--|--|--|
| | P.O. NUMBER : | | | | | | |
| | | | | | | | |
| | | | | | | | |
| | | | | | | | |
| | | | | | | | |
| | CORE DF | RILL Ø : | INLET (IL) : | | | | |
| | CORE DF | RILL Ø : | OUTLET (IL) : | | | | |
| | ORIENTA | ORIENTATION : 180° (<i>AS DRAWN</i>) / 90° / 135° | | 135° | | | |
| | SIGNED : | : | DATE : | | | | |

TO BE COMPLETED BY SW360

| | ZPG | OTHER : | | |
|---------------------|---------------------------------|---------------------------|---------|--|
| | | PRE-INSTALLATION (Y/N) : | | |
|)) RD | 3 QTR (Ø 24.0 ID) WHITE/OPAL | HALF (Ø 19.7 ID) GREEN | OTHER : | |
| WEB-FORGE / CLASS D | | OTHER : | | |
| | | DATE : | | |

NOTES

- MANHOLE UNIT FITTED WITH 2 SWIFTLIFT ANCHOR POINTS. DO NOT EXCEED 60 DEGREE LIFT ANGLE. CONCRETE LID FITTED WITH 4 SWIFTLIFT ANCHOR POINTS.
- UNIT SUPPLIED WITH INLET & OUTLET CORE DRILLED.
- SEALING / GROUTING OF MANHOLE COMPONENTS AND PIPES BY CONTRACTOR.
- ENSURING LOCAL CODES AND REGULATIONS ARE COMPLIED WITH.
- ANY RISERS REQUIRED TO INCREASE THE DEPTH TO INVERT (DTI) FROM THAT AS DRAWN TO BE SUPPLIED BY THE CONTRACTOR.
- FOR A DTI EXCEEDING 5m PLEASE CONTACT 0800STORMWATER.
- CONTRACTOR TO TAKE APPROPRIATE MEASURES TO PROTECT CARTRIDGES FROM CONSTRUCTION RELATED EROSION RUNOFF.
- BACKFILL, BEDDING AND BUOYANCY DESIGN BY ENGINEER OF RECORD
- QTY OF CARTRIDGES BY ENGINEER OF RECORD.
- CONCRETE MANHOLE RISERS ARE DESIGNED AND MANUFACTURED IN ACCORDANCE
- 10. CONCRETE MANHOLE BASES ARE DESIGNED AND MANUFACTURED IN ACCORDANCE WITH NZS 3101 : 2006 & NZS 3109 : 1997
 - CONCRETE LID DESIGNED AND MANUFACTURED TO HN-HO-72
- 12. FOR REQUIREMENTS OUTSIDE OF DRAWING SPECIFICATIONS PLEASE CONTACT

APPROX WEIGHTS

MANHOLE SECTION INCLUDING CARTRIDGES : 3900 Kg (AS DELIVERED, BASED ON QTY 5 ZPG CARTS) LID WEIGHT : 1800 Kg

| | | REV | REVISION DETAIL | DATE |
|------|----------|-----|-----------------|----------|
| | | Α | FOR APPROVAL | 22.05.19 |
| | | 0 | APPROVED | 23.05.19 |
| R.P. | 22.05.19 | | | |
| G.S. | 22.05.19 | | | |
CSF 0054; StormFilter Flow-Based Sizing - Other



| Project Name | Oruaiti School Parking area & Access way | | Location | Mangonui 2554 State Highway 10, | | | |
|--------------|--|----------|----------|---------------------------------|---------------|------------|----|
| Job # | 8021 | Device # | SWMH 2 | Option # | Α | Revision # | RA |
| Author | [Choose Designer] | | | Date | 11 April 2024 | | |

| | CALCULATIONS - Please Read Instructions First | | INSTRUCTIONS | | | | | | | | |
|---|--|--|---|--|---|--|--|--|---|--|--------------------------------------|
| 1.0 Water Quality Design Storm Peak Runoff Flowrate (RATIONAL M | 1.0. Use the rational method to compute the water quality design storm peak runoff flow rate. Values with blue text require user input. Values in red text are automatically calculated. Values with black text remain constant. | | | | | | | | | | |
| 1.1 Runoff Co-efficients | | 1 Input the appropriate runoff co-efficient for each sub-catchment. | | | | | | | | | |
| Coefficient of Impervious Roof (Croof) | 0.90 | Jee C=0.9 for imperious roof surfaces runoff co-efficient from NZBC E1/VM1: Table 1 | | | | | | | | | |
| Coefficient of Impervious Road (Croad) | 0.85 | Use C=0.85 for imperious paved surfaces runoff co-efficient from NZBC E1/VM1: Table 1 | | | | | | | | | |
| Coefficient of Pervious Area (Cper) | 0.25 | Jee C=0.25 for pervious grassed/landscaped surfaces runoff co-efficient from NZBC E1/VM1: Table 1 | | | | | | | | | |
| 1.2 Catchment Areas | | | | | | | | | | | |
| Area Impervious Roof (Aroof) | 0 m ² | 1.2 Input the appropriate cat | Input the appropriate catchment area for each sub-catchment. | | | | | | | | |
| Area Impervious Road (Aroad) | 3000 m ² | Enter impervious roof surfac | e catchment area | | | | | | | | |
| Area Pervious Area (Aper) | 0 m ² | Enter impervious paved surfa | aces catchment area | | | | | (| | | ~ |
| Area Total Catchment (Acatch) | 3000 m ² | Enter pervious grassed/lands | scaped surfaces catchm | ent area | | | | | | | |
| Product of Area & Coefficients (CA) | 2550 m ² | Total catchment area i.e. Aca | atch = Aroof + Aroad + A | per | | | | le la constante de la constante | | | |
| 1.3 Rainfall Intensity | | Product of catchment areas | & runoff co-efficients i.e. | CA = (Croof x Aroof) + (Carof x Aroof) + (Caro | Croad x Aroad) + (Cper x | Aper) | | | | | |
| Water Quality Rainfall Intensity (iWQ) | 10 mm/hr | | | | | | | ~ | Figu | ure 1: StormFilter Ca | rtridge |
| | | 1.3 Input rainfall intensity | | | | | | | | | |
| 1.4 Water Quality Design Storm Peak Runoff F | lowrate | I ne sutn percentile raintali in | ntensity is recommended | to be used for calculatin | ig the water quality flow. | wnere no statistical an | alysis of historical storm | n events nas been under | rtaken, we recommne | a iwq=10mm/nr is usea | Alternative values |
| Design Water Quality Treatment Flowrate (Qwg) | 7.083 L/s | can be checked adainst Hiki | is or intensity tables in t | ne local colincii Code of | Practice | | | | | | |
| ; | | 1.4 Compute the water qualit | v design storm peak run | off flow rate via Rational | Method | | | | | | |
| 2.0 StormFilter Peak Treatment Flowrate | | i.e. Q = f.C.i.A | | | | | | | | • • • • | - |
| 2.1 Proliminary | | | | | | | | $\Omega - $ | 0 111 | 12.06 1 10 | J.5 |
| 2.1 Freiminary | Porlito | 2.0 Lise the stormfilter stars | -discharge equation to | calculate the StormFilter | neak treatment flowrate | | | Q - q | 0.1116 | $i \Delta n$ | |
| Cartridge Media (Media) | Periite | 2.0. Ose the storminer stage | e-uischarge equation to | calculate the Storini inter | peak ueaunent nowrate. | | | ~ | | $[O] = L \min^{-1};$ | [d]=mm; [h]=m |
| Cartridge Height (Hcart) | 69 cm | Fater contrider filtration and | lin i n Deville en 700 | | | | | | | | . , , , , |
| Diameter Disc Orifice (d) | 27.6 mm | Enter cartridge filtration med | lia i.e. Perlite or ZPG | | | | | | Figure 2: Storn | nFilter Stage Dischar | ge Equation [1] |
| Internal bypass weir height (Hweir) | 0.79 m | Enter cartridge height <i>i.e.</i> 690 | cm / 46cm / 30cm | | | | | | | | |
| Priming depth (Hprime) | 0.66 m | Enter restrictor disc size, refe | er table below for max di | sc diameter | | | | 1 | | | |
| Area of a Cartridge (Acart) | 0.181 m ² | Cart Height (cm) | Actual Height (m) | Priming Depth (m) | Max Disc Diam. (mm) | Max. Design Q (L/s) | Filter Bed Area (m ²) | Flow Rate (L/s/m ²) | Bed Depth (mm) | Media Volume (m ³) | Flow Rate (L/s/m ³) |
| | | 30 | 0.305 | 0.27 | 22.70 | 0.63 | 0.460 | 1.37 | 175 | 0.052 | 12.0 |
| 2.2 StormFilter Cartridge Peak Treatment Flo | wrate | 46 | 0.457 | 0.43 | 25.00 | 0.95 | 0.689 | 1.38 | 175 | 0.078 | 12.1 |
| StormFilter cartridge stage-discharge equation | =0.111d ^{2.06} ∆h ^{0.5} L/min | 69 | 0.686 | 0.66 | 27.60 | 1.42 | 1.034 | 1.37 | 175 | 0.118 | 12.1 |
| Peak treatment flowrate at internal bypass per cartridge (Qcart) | 1.420 L/s/cart | Table 1. StormFilter Cart | ridge Specifics [2] | | | | | | | | |
| Number (actual) of StormFilter cartridges required | 4.988 cart(s) | | | | | | | | | | |
| Number (rounded) of StormFilter cartridges required | 5 cart(s) | Compute the Stormfilter peak | k treatment flowrate at in | ternal bypass per cartrid | ge via the StormFilter st | age-discharge equation | L | | | | |
| Design StormFilter Treatment Flowrate (QSF) | 7.100 L/s | Compute the number of actu | al StormFilter cartridges | required i.e. CEILING(B3 | 5,1) = QWQ / QCART | | | | | | |
| | | Compute the number (rounded up to whole number) of StormFilter cartridges required | | | | | | | | | |
| 3.0 Estimate Sediment Mass Loading | | Compute the Stormfilter peak | k treatment flowrate at in | ternal bypass via the Sto | rmFilter stage-discharg | e equation | | | | | |
| 3.1 StormFilter Manhole/Vault Dimension | s | | | | | | | | | | |
| | | | | | | | | | | | |
| Length Cartridge Bay (Lbay) | 1.000 m | 3.0 Estimate sediment mass | loading (Refer sheet '2. I | lass Load Calcs' for mor | re details) | | | | | | |
| Length Cartridge Bay (Lbay) Width Cartridge Bay (Wbay) | 1.000 m 2.440 m | 3.0 Estimate sediment mass 3.1 Use tables 2 & 3 below to | loading (Refer sheet '2. I | Mass Load Calcs' for mor | re details) of cartridges as calculate | ed in cl 2.2 above | | | | | |
| Length Cartridge Bay (Lbay) Width Cartridge Bay (Wbay) Area Cartridge Bay (Abay) | 1.000 m 2.440 m 2.440 m ² | 3.0 Estimate sediment mass 3.1 Use tables 2 & 3 below to 3.2 Use table 3 below to f | loading (Refer sheet '2. I fill in StormFilter Dimen fill in estimated TSS c | Mass Load Calcs' for mor sions based on number o oncentration. For road | re details) of cartridges as calculate ds with ≥25.000vpd, u | ed in cl 2.2 above use minimum 600kg/k | na/vr | | | | |
| Length Cartridge Bay (Lbay) Width Cartridge Bay (Wbay) Area Cartridge Bay (Nbay) Total area of Cartridge Bay (Abay) | 1.000 m 2.440 m 2.440 m ² 0.905 m ² | 3.0 Estimate sediment mass 3.1 Use tables 2 & 3 below to 3.2 Use table 3 below to f 3.3 Use 0% pretreatment for y | loading (Refer sheet '2. I fill in StormFilter Dimen fill in estimated TSS c yaults/manholes with no | Mass Load Calcs' for mor sions based on number o oncentration. For road forebay, Use 10-15% pre | re details) of cartridges as calculate ds with ≥25,000vpd, u treatmnet for vaults/mar | ed in cl 2.2 above use minimum 600kg/h pholes with forebays. U | na/yr se 50% pretreatment for i | upstream GPT ie Enviro | Pod. Use 75%-90% sv | stem efficiency | |
| Length Cartridge Bay (Lbay) Width Cartridge Bay (Wbay) Area Cartridge Bay (Abay) Total area of Cartridge G(Acarts) | 1.000 m 2.440 m 2.440 m ² 0.905 m ² 1.535 m ² | 3.0 Estimate sediment mass 3.1 Use tables 2 & 3 below to 3.2 Use table 3 below to f 3.3 Use 0% pretreatment for v | loading (Refer sheet '2. I fill in StormFilter Dimen fill in estimated TSS c vaults/manholes with no | Mass Load Calcs' for mor sions based on number o oncentration. For road forebay. Use 10-15% pre Without | re details) of cartridges as calculate ds with ≥25,000vpd, u treatmnet for vaults/mar forebay | ed in cl 2.2 above ise minimum 600kg/ł nholes with forebays. U | na/yr se 50% pretreatment for i | upstream GPT ie Enviro With for | Pod. Use 75%-90% sy | stem efficiency | |
| Length Cartridge Bay (Lbay) Width Cartridge Bay (Wbay) Area Cartridge Bay (Abay) Total area of Cartridges (Acarts) Area Lower Volume (Alow) Volume Lower Volume (Volume (Volume) | 1.000 m 2.440 m 2.440 m ² 0.905 m ² 1.535 m ² | 3.0 Estimate sediment mass 3.1 Use tables 2 & 3 below to 3.2 Use table 3 below to f 3.3 Use 0% pretreatment for Std Manhole Dimensions | loading (Refer sheet '2. I fill in StormFilter Dimen iill in estimated TSS c vaults/manholes with no | Ass Load Calcs' for mor sions based on number o oncentration. For road forebay. Use 10-15% pre Without | re details) of cartridges as calculat ds with ≥25,000vpd, u treatmnet for vaults/mar forebay | ed in cl 2.2 above ise minimum 600kg/ł nholes with forebays. U | na/yr se 50% pretreatment for i | upstream GPT ie Enviro With for | Pod. Use 75%-90% sy rebay | stem efficiency | |
| Length Cartridge Bay (Lbay) Width Cartridge Bay (Wbay) Area Cartridge Bay (Abay) Total area of Cartridges (Acarts) Area Lower Volume (Alow) Volume Lower Volume (Vlow) | 1.000 m 2.440 m 2.440 m ² 0.905 m ² 1.535 m ² 1059 L 2.440 m ² | 3.0 Estimate sediment mass i 3.1 Use tables 2 & 3 below to 3.2 Use table 3 below to f 3.3 Use 0% pretreatment for v Std Manhole Dimensions | loading (Refer sheet '2. I fill in StormFilter Dimen iill in estimated TSS c vaults/manholes with no Cart Bay Length (m) | Aass Load Calcs' for mor sions based on number of oncentration. For roar forebay. Use 10-15% pre Without Cart Bay Width (m) 0.77 | re details) of cartridges as calculate ds with ≥25,000vpd, u treatmnet for vaults/mar forebay Cart Bay Area (m ²) 0,77 | ed in cl 2.2 above ise minimum 600kg/ł hholes with forebays. U Max Number Carts | na/yr se 50% pretreatment for t Cart Bay Length (m) | upstream GPT ie Enviro With for Cart Bay Width (m) | Pod. Use 75%-90% sy rebay Cart Bay Area (m²) | stem efficiency Max Number Carts | |
| Length Cartridge Bay (Lbay) Width Cartridge Bay (Wbay) Area Cartridge Bay (Abay) Total area of Cartridges (Acarts) Area Lower Volume (Alow) Volume Lower Volume (Vlow) Area Upper Volume (Alow) | 1.000 m 2.440 m 2.440 m ² 0.905 m ² 1.535 m ² 1059 L 2.440 m ² | 3.0 Estimate sediment mass 3.1 Use tables 2 & 3 below to 3.2 Use table 3 below to f 3.3 Use 0% pretreatment for Std Manhole Dimensions 1050 1200 | loading (Refer sheet '2. I fill in StormFilter Dimen iill in estimated TSS c vaults/manholes with no Cart Bay Length (m) 1.00 | Aass Load Calcs' for mor sions based on number of oncentration. For road forebay. Use 10-15% pre Without Cart Bay Width (m) 0.77 | re details) of cartridges as calculate ds with ≥25,000vpd, u treatmnet for vaults/mar forebay Cart Bay Area (m ²) 0.77 | ed in cl 2.2 above ise minimum 600kg/h hholes with forebays. U: Max Number Carts 1 | na/yr se 50% pretreatment for i Cart Bay Length (m) 1.00 | upstream GPT ie Enviro With for Cart Bay Width (m) N/A | Pod. Use 75%-90% sy rebay Cart Bay Area (m²) N/A | Stem efficiency Max Number Carts N/A | |
| Length Cartridge Bay (Lbay) Width Cartridge Bay (Wbay) Area Cartridge Bay (Nbay) Total area of Cartridges (Acarts) Area Lower Volume (Alow) Volume Lower Volume (Vlow) Area Upper Volume (Nop) Volume Upper Volume (Vupp) | 1.000 m 2.440 m 2.440 m ² 0.905 m ² 1.535 m ² 1059 L 2.440 m ² 244 L | 3.0 Estimate sediment mass 3.1 Use tables 2 & 3 below to 3.2 Use table 3 below to f 3.3 Use 0% pretreatment for v Std Manhole Dimensions 1050 1200 | loading (Refer sheet '2. 1 fill in StormFilter Dimen iil in estimated TSS c vauits/manholes with no Cart Bay Length (m) 1.00 1.00 | Ass Load Calcs' for mor sions based on number of oncentration. For road forebay. Use 10-15% pre Without Cart Bay Width (m) 0.77 1.03 | e details) of cartridges as calculat ds with ≥25,000vpd, u treatmet for vaults/mar forebay Cart Bay Area (m²) 0.77 1.03 | ed in cl 2.2 above ise minimum 600kg/t nholes with forebays. U: Max Number Carts 1 3 | halyr se 50% pretreatment for i Cart Bay Length (m) 1.00 1.00 | upstream GPT ie Enviro With for Cart Bay Width (m) N/A 0.76 | Pod. Use 75%-90% sy rebay Cart Bay Area (m ²) N/A 0.76 | stem efficiency Max Number Carts N/A 2 | |
| Length Cartridge Bay (Lbay) Width Cartridge Bay (Wbay) Area Cartridge Bay (Abay) Total area of Cartridges (Acarts) Area Lower Volume (Alow) Volume Lower Volume (Nump) Area Upper Volume (Aupp) Volume Upper Volume (Vupp) Live storage volume at internal bypass (Vstor) | 1.000 m 2.440 m 2.440 m ² 0.905 m ² 1.535 m ² 1059 L 2.440 m ² 2.44 L 1303 L | 3.0 Estimate sediment mass 3.1 Use tables 2 & 3 below to 3.2 Use table 3 below to f 3.3 Use 0% pretreatment for v Std Manhole Dimensions 1050 1200 1500 | loading (Refer sheet '2. 1 fill in StormFilter Dimen fill in estimated TSS c vaults/manholes with no Cart Bay Length (m) 1.00 1.00 | Aass Load Cales' for mor sions based on number of oncentration. For road forebay. Use 10-15% pre Without Cart Bay Width (m) 0.77 1.03 1.67 | e details) of cartridges as calculati ds with ≥25,000ypd, u treatmnet for vaults/mar forebay Cart Bay Area (m ²) 0.77 1.03 1.67 | ed in cl 2.2 above see minimum 600kg/l holes with forebays. U Max Number Carts 1 3 4 | alyr se 50% pretreatment for i Cart Bay Length (m) 1.00 1.00 1.00 | upstream GPT ie Enviro With for Cart Bay Width (m) N/A 0.76 1.39 | Pod. Use 75%-90% sy rebay Cart Bay Area (m ²) N/A 0.76 1.39 | Stem efficiency Max Number Carts N/A 2 3 | |
| Length Cartridge Bay (Lbay) Width Cartridge Bay (Wbay) Area Cartridge Bay (Abay) Total area of Cartridges (Acarts) Area Lower Volume (Alow) Volume Lower Volume (Vlow) Area Upper Volume (Vupp) Volume Upper Volume (Vupp) Live storage volume at internal bypass (Vstor) | 1.000 m 2.440 m 2.440 m ² 0.905 m ² 1.535 m ² 1059 L 2.440 m ² 244 L 1303 L | 3.0 Estimate sediment mass 3.1 Use tables 2 & 3 below to 3.2 Use table 3 below to f 3.3 Use 0% pretreatment for v Std Manhole Dimensions 1050 1200 1500 | loading (Refer sheet '2. 1 fill in StormFilter Dimen ill in estimated TSS c vaults/manholes with no Cart Bay Length (m) 1.00 1.00 1.00 | Aass Load Cales' for mor sions based on number of oncentration. For road forebay. Use 10-15% pre Without Cart Bay Width (m) 0.77 1.03 1.67 2.44 | e details) of cartridges as calculat ds with ≥25,000ypd, u treatment for vaults/mar forebay Cart Bay Area (m ²) 0.77 1.03 1.67 2.44 | ed in cl 2.2 above ise minimum 600kg/h sholes with forebays. U Max Number Carts 1 3 4 7 | talyr se 50% pretreatment for r Cart Bay Length (m) 1.00 1.00 1.00 1.00 | upstream GPT ie Enviro With for Cart Bay Width (m) N/A 0.76 1.39 1.83 | Pod. Use 75%-90% sy rebay Cart Bay Area (m ²) N/A 0.76 1.39 1.83 | stem efficiency Max Number Carts N/A 2 3 5 | |
| Length Cartridge Bay (Lbay) Width Cartridge Bay (Wbay) Area Cartridge Bay (Abay) Total area of Cartridges Bay (Abay) Area Lower Volume (Alow) Volume Lower Volume (Vlow) Area Upper Volume (Vupp) Usine Upper Volume (Vupp) Live storage volume at internal bypass (Vstor) | 1.000 m 2.440 m 2.440 m ² 0.905 m ² 1.535 m ² 1059 L 2.440 m ² 244 L 1303 L | 3.0 Estimate sediment mass 3.1 Use tables 2 & 3 below to 3.2 Use table 3 below to f 3.3 Use 0% pretreatment for Std Manhole Dimensions 1050 1200 1500 1500 2050 | loading (Refer sheet '2. I fill in StormFilter Dimen iill in estimated TSS c vaults/manholes with no Cart Bay Length (m) 1.00 1.00 1.00 1.00 | Aass Load Cales' for mor sions based on number oncentration. For road forebay. Use 10-15% pro- Without Cart Bay Width (m) 0.77 1.03 1.67 2.44 3.20 | e details) of cartridges as calculati ds with ≥25,000vpd, u treatmnet for vaults/mar forebay 0.77 1.03 1.67 2.44 3.20 | ed in cl 2.2 above rise minimum 600kg/h hholes with forebays. U: Max Number Carts 1 3 4 7 9 | halyr se 50% pretreatment for t Cart Bay Length (m) 1.00 1.00 1.00 1.00 1.00 | upstream GPT ie Enviro With for Cart Bay Width (m) N/A 0.76 1.39 1.83 2.80 | Pod. Use 75%-90% sy ebay Cart Bay Area (m ²) N/A 0.76 1.39 1.83 2.80 | stem efficiency Max Number Carts N/A 2 3 3 5 7 | |
| Length Cartridge Bay (Lbay) Width Cartridge Bay (Wbay) Area Cartridge Bay (Abay) Total area of Cartridges (Acarts) Area Lower Volume (Alow) Volume Lower Volume (Vlow) Volume Upper Volume (Vupp) Live storage volume at internal bypass (Vstor) 3.2 Catchment Sediment Loading Estimated TSS Concentration Impervious Roof (TSSroof) | 1.000 m 2.440 m 2.440 m ² 0.905 m ² 1.535 m ² 1059 L 2.440 m ² 244 L 1303 L | 3.0 Estimate sediment mass 3.1 Use tables 2 & 3 below to 3.2 Use table 3 below to f 3.3 Use 0% pretreatment for 1 Std Manhole Dimensions 1050 1200 1500 1800 2050 | loading (Refer sheet '2. 1 fill in StormFilter Dimen fill in estimated TSS c vaults/manholes with no Cart Bay Length (m) 1.00 1.00 1.00 1.00 1.00 | Aass Load Cales' for mor sions based on number of oncentration. For road forebay. Use 10-15% pre Without Cart Bay Width (m) 0.77 1.03 1.67 2.44 3.20 | e details) of cartridges as calculati ds with ≥25,000 ypd, u treatmnet for vaults/mar forebay Cart Bay Area (m ²) 0.77 1.03 1.67 2.44 3.20 | ed in cl 2.2 above see minimum 600kg/h holes with forebays. U Max Number Carts 1 3 4 7 9 | talyr se 50% pretreatment for i Cart Bay Length (m) 1.00 1.00 1.00 1.00 | Upstream GPT ie Enviro With for Cart Bay Width (m) N/A 0.76 1.39 1.83 2.80 Table 2: Standard St | Pod. Use 75%-90% sy ebay Cart Bay Area (m²) N/A 0.76 1.39 1.83 2.80 comwater360 Manh | stem efficiency Max Number Carts N/A 2 3 5 7 ole Dimensions | |
| Length Cartridge Bay (Lbay) Width Cartridge Bay (Wbay) Area Cartridge Bay (Mbay) Total area of Cartridges (Acarts) Area Lower Volume (Alow) Volume Lower Volume (Mupp) Volume Upper Volume (Mupp) User Upper Volume (Vupp) Clive storage volume at internal bypass (Vstor) 3.2 Catchment Sediment Loading Estimated TSS Concentration Impervious Roof (TSSroof) Estimated TSS Concentration Impervious Rood (TSSroof) | 1.000 m 2.440 m 2.440 m ² 0.905 m ² 1.535 m ² 1059 L 2.440 m ² 2.44 L 1303 L | 3.0 Estimate sediment mass 3.1 Use tables 2 & 3 below to 3.2 Use table 3 below to f 3.3 Use 0% pretreatment for v Std Manhole Dimensions 1050 1200 1500 1500 2050 Std Yault Dimensions | loading (Refer sheet '2. 1 fill in StormFilter Dimen fill in estimated TSS c vaults/manholes with no Cart Bay Length (m) 1.00 1.00 1.00 1.00 | Aass Load Cales' for mor sions based on number of oncentration. For road forebay. Use 10-15% pre Without Cart Bay Width (m) 0.77 1.03 1.67 2.44 3.20 Without | e details) of cartridges as calculat ds with ≥25,000ypd, u treatment for vaults/mar forebay Cart Bay Area (m ²) 0.77 1.03 1.67 2.44 3.20 forebay | ed in cl 2.2 above ise minimum 600kg/h aboles with forebays. U Max Number Carts 1 3 4 7 9 | halyr Se 50% pretreatment for 1 Cart Bay Length (m) 1.00 1.00 1.00 1.00 1.00 | upstream GPT ie Enviro With for Cart Bay Width (m) N/A 0.76 1.39 1.83 2.80 Table 2: Standard St With for | Pod. Use 75%-90% sy cebay Cart Bay Area (m²) N/A 0.76 1.39 1.83 2.80 cornwater360 Manh rebay | stem efficiency Max Number Carts N/A 2 3 5 7 ole Dimensions | |
| Length Cartridge Bay (Lbay) Width Cartridge Bay (Wbay) Area Cartridge Bay (Mbay) Total area of Cartridges (Acarts) Area Lower Volume (Alow) Volume Lower Volume (Vlow) Area Upper Volume (Vlow) Outime Upper Volume (Vupp) Volume Upper Volume (Vupp) Live storage volume at internal bypass (Vstor) <u>3.2 Catchment Sediment Loading</u> Estimated TSS Concentration Impervious Roof (TSSroad) Estimated TSS Concentration Impervious Road (TSSroad) Estimated TSS Concentration Pervious Area (TSSper) | 1.000 m 2.440 m 2.440 m ² 0.905 m ² 1.535 m ² 1059 L 2.440 m ² 244 L 1303 L 100 kg/ha/year 200 kg/ha/year 200 kg/ha/year | 3.0 Estimate sediment mass 3.1 Use tables 2 & 3 below to 3.2 Use table 3 below to f 3.3 Use 0% pretreatment for v Std Manhole Dimensions 1050 1200 1500 1800 2050 Std Vault Dimensions | loading (Refer sheet '2. I fill in StormFilter Dimen iill in estimated TSS c vaults/manholes with no Cart Bay Length (m) 1.00 1.00 1.00 1.00 Cart Bay Length (m) | Aass Load Cales' for mor sions based on number oncentration. For road forebay. Use 10-15% pre- Without Cart Bay Width (m) 0.77 1.03 1.67 2.44 3.20 Without Cart Bay Width (m) | e details) of cartridges as calculat for a second second second second second for a second second second second second 0.77 0.03 0.77 1.03 1.67 2.44 3.20 for ebay Cart Bay Area (m ²) | ed in cl 2.2 above ise minimum 600kg/h sholes with forebays. U Max Number Carts 1 3 4 7 9 Max Number Carts | la/yr se 50% pretreatment for r Cart Bay Length (m) 1.00 1.00 1.00 1.00 Cart Bay Length (m) | Upstream GPT is Enviro With for Cart Bay Width (m) N/A 0.76 1.39 1.83 2.80 Table 2: Standard St With for Cart Bay Width (m) | Pod. Use 75%-90% sy rebay Cart Bay Area (m ²) N/A 0.76 1.39 1.83 2.80 comwater360 Manh ebay Cart Bay Area (m ²) | stem efficiency Max Number Carts N/A 2 3 5 7 ole Dimensions Max Number Carts | |
| Length Cartridge Bay (Lbay) Width Cartridge Bay (Wbay) Area Cartridge Bay (Mbay) Total area of Cartridges (Acarts) Area Lower Volume (Alow) Volume Lower Volume (Vlow) Volume Lower Volume (Vlow) Volume Upper Volume (Vlop) Live storage volume at internal bypass (Vstor) 3.2 Catchment Sediment Loading Estimated TSS Concentration Impervious Road (TSSroof) Estimated TSS Concentration Impervious Road (TSSroof) Estimated TSS Concentration Impervious Area (TSSper) Estimated TSS Concentration Impervious Area (TSSper) Estimated TSS Concentration Provide Area (TSSper) | 1.000 m 2.440 m 2.440 m ² 0.905 m ² 1.535 m ² 1059 L 2.440 m ² 244 L 1303 L 100 kg/ha/year 200 kg/ha/year 90 kg/year | 3.0 Estimate sediment mass 3.1 Use tables 2 & 3 below to 3.2 Use table 3 below to f 3.3 Use 0% pretreatment for v Std Manhole Dimensions 1050 1500 1500 2050 Std Vault Dimensions 3.4 L x 1.5 W x 1.8 D | loading (Refer sheet '2. 1 fill in StormFilter Dimen fill in estimated TSS c vaults/manholes with no Cart Bay Length (m) 1.00 1.00 1.00 2.85 | Aass Load Cales' for mor sions based on number of oncentration. For roar forebay. Use 10-15% pre Without Cart Bay Width (m) 1.67 2.44 3.20 Without Cart Bay Width (m) 1.50 | e details) of cartridges as calculati ds with ≥25,000vpd, u treatmnet for vaults/mar forebay 0.77 1.03 1.67 2.44 3.20 forebay Cart Bay Area (m ²) 4.28 | ed in cl 2.2 above use minimum 600kg/h holes with forebays. U Max Number Carts 1 3 4 7 9 9 Max Number Carts 11 | ta/yr Se 50% pretreatment for i Cart Bay Length (m) 1.00 1.00 1.00 1.00 Cart Bay Length (m) 2.30 | Upstream GPT is Enviro With for Cart Bay Width (m) N/A 0.76 1.39 1.83 2.80 Table 2: Standard St With for Cart Bay Width (m) 1.80 | Pod. Use 75%-90% sy ebay Cart Bay Area (m²) N/A 0.76 1.39 1.83 2.80 ornwater360 Manh ebay Cart Bay Area (m²) 4.14 | stem efficiency Max Number Carts N/A 2 3 5 7 ole Dimensions Max Number Carts 8 | |
| Length Cartridge Bay (Lbay) Width Cartridge Bay (Wbay) Area Cartridge Bay (Mbay) Total area of Cartridges (Acarts) Area Lower Volume (Alow) Volume Lower Volume (Now) Area Upper Volume (Vop) Uolume Upper Volume (Vop) Live storage volume at internal bypass (Vstor) 3.2 Catchment Sediment Loading Estimated TSS Concentration Impervious Roof (TSSrood) Estimated TSS Concentration Impervious Roof (TSSrood) Estimated TSS Concentration Pervious Area (TSSper) Estimated TSS Concentration Pervious Area (TSSper) | 1.000 m 2.440 m 2.440 m ² 0.905 m ² 1.535 m ² 1059 L 2.440 m ² 2.44 L 1303 L 100 kg/ha/year 200 kg/ha/year 90 kg/year | 3.0 Estimate sediment mass 3.1 Use tables 2 & 3 below to 3.2 Use table 3 below to f 3.3 Use 0% pretreatment for 1 Std Manhole Dimensions 1050 1200 1500 1800 2050 Std Vault Dimensions 3.4 L x 1.5 W x 1.8 D 4.5 L x 1.5 W x 1.8 D | loading (Refer sheet '2. 1 fill in StormFilter Dimen fill in estimated TSS c vaults/manholes with no Cart Bay Length (m) 1.00 1.00 1.00 1.00 2.00 Cart Bay Length (m) 2.85 3.95 | Aass Load Cales' for mor sions based on number of oncentration. For road forebay, Use 10-15% pre Without Cart Bay Width (m) 0.77 1.03 1.67 2.44 3.20 Without Cart Bay Width (m) 1.50 1.50 | re details) of cartridges as calculat ds with ≥25,000 ypd, u treatment for vaults/mar forebay Cart Bay Area (m ²) 0.77 1.03 1.67 2.44 3.20 forebay Cart Bay Area (m ²) 4.28 5.93 | ed in cl 2.2 above rese minimum 600kg/h tholes with forebays. U Max Number Carts 1 3 4 7 9 Max Number Carts 11 17 | na/yr se 50% pretreatment for 1 Cart Bay Length (m) 1.00 1.00 1.00 1.00 1.00 Cart Bay Length (m) 2.30 3.40 | Upstream GPT ie Enviro With for Cart Bay Width (m) N/A 0.76 1.39 1.83 2.80 Table 2: Standard St With for Cart Bay Width (m) 1.80 1.50 | Pod. Use 75%-90% sy ebay Cart Bay Area (m ²) N/A 0.76 1.39 1.33 2.80 comwater360 Manh ebay Cart Bay Area (m ²) 4.14 5.10 | stem efficiency Max Number Carts N/A 2 3 5 7 ole Dimensions Max Number Carts 8 14 | |
| Length Cartridge Bay (Lbay) Width Cartridge Bay (Wbay) Area Cartridge Bay (Mbay) Total area of Cartridges (Acarts) Area Lower Volume (Alow) Volume Lower Volume (Vlow) Area Upper Volume (Vupp) Volume Upper Volume (Vupp) Live storage volume at internal bypass (Vstor) 3.2 Catchment Sediment Loading Estimated TSS Concentration Impervious Roof (TSSroof) Estimated TSS Concentration Impervious Roof (TSSroof) Estimated TSS Concentration Impervious Area (TSSper) Estimated TSS Concentration Pervious Area (TSSper) Estimated Total TSS Load (TSSload) 3.3 Treatment Efficiencies | 1.000 m 2.440 m 2.440 m ² 0.905 m ² 1.535 m ² 1059 L 2.440 m ² 244 L 1303 L 100 kg/ha/year 300 kg/ha/year 90 kg/year | 3.0 Estimate sediment mass 3.1 Use tables 2 & 3 below to 3.2 Use table 3 below to f 3.3 Use 0% pretreatment for v Std Manhole Dimensions 1050 1200 1500 1500 2050 Std Vault Dimensions 3.4 L x 1.5 W x 1.8 D 4.5 L x 1.5 W x 1.8 D 4.2 L x 2.0 W x 1.8 D | loading (Refer sheet '2. I fill in StormFilter Dimen iill in estimated TSS c vaults/manholes with no Cart Bay Length (m) 1.00 1.00 1.00 2.85 3.95 3.95 | Aass Load Cales' for mor sions based on number oncentration. For road forebay, Use 10-15% pre- Without Cart Bay Width (m) 0.77 1.03 1.67 2.44 3.20 Without Cart Bay Width (m) 1.50 1.50 1.55 | e details) of cartridges as calculat ds with ≥25,000ypd, u treatment for vaults/mar forebay Cart Bay Area (m ²) 0.77 1.03 1.67 2.44 3.20 forebay Cart Bay Area (m ²) 4.28 5.93 7.70 | ed in cl 2.2 above ise minimum 600kg/t hholes with forebays. U Max Number Carts 1 3 4 7 9 Max Number Carts 11 17 23 | halyr Se 50% pretreatment for r Cart Bay Length (m) 1.00 1.00 1.00 1.00 2.00 Cart Bay Length (m) 2.30 3.40 3.40 | upstream GPT ie Enviro With for Cart Bay Width (m) N/A 0.76 1.39 1.83 2.80 Table 2: Standard St With for Cart Bay Width (m) 1.80 1.50 2.10 | Pod. Use 75%-90% sy rebay Cart Bay Area (m ²) N/A 0.76 1.39 1.83 2.80 comwater360 Manh rebay Cart Bay Area (m ²) 4.14 5.10 7.14 | stem efficiency Max Number Carts N/A 2 3 5 7 ole Dimensions Max Number Carts 8 14 14 | |
| Length Cartridge Bay (Lbay) Width Cartridge Bay (Wbay) Area Cartridge Bay (Mbay) Total area of Cartridges (Acarts) Area Cartridges (Acarts) Area Lower Volume (Alow) Volume Lower Volume (Vlow) Volume Upper Volume (Vlow) Live storage volume at internal bypass (Vstor) 3.2 Catchment Sediment Loading Estimated TSS Concentration Impervious Roof (TSSroof) Estimated TSS Concentration Impervious Roaf (TSSroof) Estimated TSS Concentration Impervious Area (TSSper) Estimated TSS Concentration Previous Area (TSSper) Estimated TSS Concentration Previous Area (TSSper) Estimated TSS Concentration Previous Area (TSSper) Association (TSS) Bay (TSS) (TSS) Bay (TSS) (TSS) Bay (TSS) (TSS) (TSS) Bay (TSS) (TS | 1.000 m 2.440 m 2.440 m ² 0.905 m ² 1.535 m ² 1059 L 2.440 m ² 244 L 1303 L 100 kg/ha/year 200 kg/ha/year 90 kg/year | 3.0 Estimate sediment mass 3.1 Use tables 2 & 3 below to 3.2 Use table 3 below to f 3.3 Use 0% pretreatment for v Std Manhole Dimensions 1050 1500 1500 1500 Std Vault Dimensions 3.4 L x 1.5 W x 1.8 D 4.2 L x 2.0 W x 1.8 D 5.6 L x 2.0 W x 1.8 D | loading (Refer sheet '2. I fill in StormFilter Dimen iil in estimated TSS c vaults/manholes with no Cart Bay Length (m) 1.00 1.00 1.00 2.85 3.95 3.95 5.05 | Aass Load Cales' for mor sions based on number of oncentration. For roar forebay. Use 10-15% pre Without Cart Bay Width (m) 1.67 2.44 3.20 Without Cart Bay Width (m) 1.50 1.55 1.95 | e details) of cartridges as calculati ds with ≥25,000ypd, u treatmnet for vaults/mar forebay Cart Bay Area (m ²) 0.77 1.03 1.67 2.244 3.20 forebay Cart Bay Area (m ²) 4.28 5.33 7.70 9.85 | ed in cl 2.2 above see minimum 600kg/h holes with forebays. U Max Number Carts 1 3 4 7 9 Max Number Carts 11 17 23 31 | ta/yr Se 50% pretreatment for i Cart Bay Length (m) 1.00 1.00 1.00 1.00 Cart Bay Length (m) 2.30 3.40 4.50 | Upstream GPT is Enviro With for Cart Bay Width (m) N/A 0.76 1.39 1.83 2.80 Table 2: Standard St With for Cart Bay Width (m) 1.80 1.50 2.10 2.10 | Pod. Use 75%-90% sy ebay Cart Bay Area (m²) N/A 0.76 1.39 1.83 2.80 comwater360 Manh rebay Cart Bay Area (m²) 4.14 5.10 7.14 9.45 | stem efficiency Max Number Carts N/A 2 3 5 7 0le Dimensions Max Number Carts 8 14 18 26 | |
| Length Cartridge Bay (Lbay) Width Cartridge Bay (Wbay) Area Cartridge Bay (Mbay) Total area of Cartridges (Acarts) Area Lower Volume (Alow) Volume Lower Volume (Now) Area Upper Volume (Vop) Usume Upper Volume (Vupp) Live storage volume at internal bypass (Vstor) 3.2 Catchment Sediment Loading Estimated TSS Concentration Impervious Road (TSSroad) Estimated TSS Concentration Impervious Road (TSSroad) Estimated TSS Concentration Impervious Road (TSSroad) Estimated TSS Concentration Pervious Area (TSSer) Estimated TSS Concentration Pervious Area (TSSer) Stimated TSS Concentration Erroris Cond (TSSIcad) Estimated TSS Cond Erroris Cond (TSSIcad) Estimated TSS Cond (TSSIcad) | 1.000 m 2.440 m 2.440 m ² 0.905 m ² 1.535 m ² 1059 L 2.440 m ² 2.44 L 1303 L 100 kg/ha/year 300 kg/ha/year 90 kg/year | 3.0 Estimate sediment mass 3.1 Use tables 2 & 3 below to 3.2 Use table 3 below to f 3.3 Use 0% pretreatment for v Std Manhole Dimensions 1050 1200 1500 2050 Std Vault Dimensions 3.4 L x 1.5 W x 1.8 D 4.5 L x 1.5 W x 1.8 D 4.5 L x 1.5 W x 1.8 D 5.6 L x 2.0 W x 1.8 D 5.6 L x 2.4 W x 1.8 D | loading (Refer sheet '2, 1 fill in StormFilter Dimen fill in estimated TSS c vaults/manholes with no Cart Bay Length (m) 1.00 1.00 1.00 2.85 3.95 3.95 5.05 | Aass Load Cales' for mor sions based on number of oncentration. For road forebay. Use 10-15% pre Without Cart Bay Width (m) 0.77 1.03 1.67 2.44 3.20 Without Cart Bay Width (m) 1.50 1.50 1.55 1.95 2.40 | re details) of cartridges as calculati ds with ≥25,000 ypd, u treatmet for vaults/mar forebay Cart Bay Area (m ²) 0.77 1.03 1.67 2.44 3.20 cart Bay Area (m ²) 4.28 5.93 7.70 9.85 12.12 | ed in cl 2.2 above use minimum 600kg/h holes with forebays. U Max Number Carts 1 3 4 7 9 Max Number Carts 11 17 23 31 39 | 18/yr se 50% pretreatment for r Cart Bay Length (m) 1.00 1.00 1.00 1.00 1.00 2.30 3.40 3.40 4.50 | Upstream GPT is Enviro With for Cart Bay Width (m) N/A 0.76 1.39 1.83 2.80 Table 2: Standard St With for Cart Bay Width mo 1.80 1.50 2.10 2.10 | Pod. Use 75%-90% sy ebay Cart Bay Area (m ²) N/A 0.76 1.39 1.83 2.80 cort water 360 Manh ebay Cart Bay Area (m ²) 4.14 5.10 7.14 9.45 9.45 | stem efficiency Max Number Carts N/A 2 3 5 7 Ole Dimensions Max Number Carts 8 14 18 26 27 | |
| Length Cartridge Bay (Lbay) Width Cartridge Bay (Wbay) Area Cartridge Bay (Mbay) Total area of Cartridges (Acarts) Area Lower Volume (Alow) Volume Lower Volume (Vlow) Area Upper Volume (Vlow) Area Upper Volume (Vupp) Use Upper Volume (Vupp) Settimated TSS Concentration Impervious Road (TSSroof) Estimated TSS Concentration Impervious Road (TSSroof) Estimated TSS Concentration Impervious Road (TSSroof) Estimated TSS Concentration Pervious Area (TSSper) Estimated TSS Concentration Pervious Area (TSSper) Estimated Total TSS Load (TSSload) Sa Treatment Efficiencies Pre-treatment Efficiency (EFFpre) System Efficiency (EFFsps) | 1.000 m 2.440 m 2.440 m ² 0.905 m ² 1.535 m ² 1059 L 2.440 m ² 2.44 L 1303 L 100 kg/ha/year 300 kg/ha/year 90 kg/year 50 % 75 % | 3.0 Estimate sediment mass 3.1 Use tables 2 & 3 below to 3.2 Use table 3 below to f 3.3 Use 0% pretreatment for v Std Manhole Dimensions 1050 1200 1500 1500 2050 Std Vault Dimensions 3.4 L x 1.5 W x 1.8 D 4.5 L x 1.5 W x 1.8 D 4.5 L x 2.0 W x 1.8 D 5.6 L x 2.0 W x 1.8 D 5.6 L x 2.4 W x 1.8 D | loading (Refer sheet '2. I fill in StormFilter Dimen iill in estimated TSS c vaults/manholes with no Cart Bay Length (m) 1.00 1.00 1.00 2.85 3.95 3.95 3.95 5.05 5.60 | Aass Load Cales' for mor sions based on number of oncentration. For road forebay, Use 10-15% pre Without Cart Bay Width (m) 0.77 1.03 1.67 2.44 3.20 Without Cart Bay Width (m) 1.50 1.50 1.95 1.95 2.40 | e details) of cartridges as calculat ds with ≥25,000ypd, u treatmet for vaults/mar forebay Cart Bay Area (m ²) 0.77 1.03 1.67 2.44 3.20 forebay Cart Bay Area (m ²) 4.28 5.93 7.70 9.85 12.12 13.44 | ed in cl 2.2 above ise minimum 600kg/h tholes with forebays. U Max Number Carts 1 3 4 7 9 Max Number Carts 11 17 23 31 39 44 | halyr se 50% pretreatment for 1 Cart Bay Length (m) 1.00 1.00 1.00 1.00 Cart Bay Length (m) 2.30 3.40 3.40 4.50 4.50 | upstream GPT ie Enviro With for Cart Bay Width (m) N/A 0.76 1.39 1.83 2.80 Table 2: Standard St With for Cart Bay Width (m) 1.80 1.50 2.10 2.10 2.10 2.40 | Pod. Use 75%-90% sy cebay Cart Bay Area (m²) N/A 0.76 1.39 1.33 2.80 cornwater/360 Manh rebay Cart Bay Area (m²) 4.14 5.10 7.14 9.45 9.45 10.80 | stem efficiency Max Number Carts N/A 2 3 5 7 ole Dimensions Max Number Carts 8 14 18 26 27 33 | |
| Length Cartridge Bay (Lbay) Width Cartridge Bay (Wbay) Area Cartridge Bay (Mbay) Total area of Cartridges (Acarts) Area Lower Volume (Alow) Volume Lower Volume (Vlow) Area Upper Volume (Vupp) Volume Upper Volume (Vupp) Live storage volume at internal bypass (Vstor) 3.2 Catchment Sediment Loading Estimated TSS Concentration Impervious Roof (TSSroaf) Estimated TSS Concentration Impervious Roof (TSSroaf) Estimated TSS Concentration Impervious Area (TSSper) Estimated TSS Concentration Pervious Area (TSSper) Estimated TSS Concentration Pervious Area (TSSoad) Estimated TSS Concentration Pervious Area (TSSper) Estimated Total TSS Load (TSSIcad) 3.3 Treatment Efficiencies Pre-treatment Efficiency (EFFsys) System Efficiency (EFFsys) 3.4 Maintenance Requirements | 1.000 m 2.440 m 2.440 m ² 0.905 m ² 1.535 m ² 1059 L 2.440 m ² 244 L 1303 L 100 kg/ha/year 300 kg/ha/year 90 kg/year | 3.0 Estimate sediment mass 3.1 Use tables 2 & 3 below to 3.2 Use table 3 below to f 3.3 Use 0% pretreatment for v Std Manhole Dimensions 1050 1200 1500 1500 1500 Std Vault Dimensions 3.4 L x 1.5 W x 1.8 D 4.5 L x 1.5 W x 1.8 D 5.6 L x 2.0 W x 1.8 D 5.6 L x 2.4 W x 1.8 D 6.2 L x 2.4 W x 1.8 D | loading (Refer sheet '2. I fill in StormFilter Dimen iil in estimated TSS c vaults/manholes with no Cart Bay Length (m) 1.00 1.00 1.00 2.85 3.95 5.05 5.05 5.60 | Aass Load Cales' for mor sions based on number of oncentration. For roar forebay. Use 10-15% pre Without Cart Bay Width (m) 0.77 1.03 1.67 2.44 3.20 Without Cart Bay Width (m) 1.50 1.55 1.95 1.95 2.40 2.40 | e details) of cartridges as calculati ds with ≥25,000vpd, u treatmet for vaults/mar forebay Cart Bay Area (m ²) 0.77 1.03 1.67 2.244 3.20 forebay Cart Bay Area (m ²) 4.28 5.93 7.70 9.85 12.12 13.44 | A constraints of the second se | ta/yr se 50% pretreatment for i Cart Bay Length (m) 1.00 1.00 1.00 1.00 Cart Bay Length (m) 2.30 3.40 3.40 4.50 4.50 4.50 | Upstream GPT is Enviro With for Cart Bay Width (m) N/A 0.76 1.33 1.83 2.80 Table 2: Standard St With for Cart Bay Width (m) 1.80 1.50 2.10 2.10 2.10 2.10 2.10 2.10 2.10 2.1 | Pod. Use 75%-90% sy ebay Cart Bay Area (m²) N/A 0.76 1.39 1.83 2.80 comwater360 Manh rebay Cart Bay Area (m²) 4.14 5.10 7.14 9.45 9.45 10.80 d Stormwater360 V | stem efficiency Max Number Carts N/A 2 3 5 7 ole Dimensions Max Number Carts 8 14 18 26 27 33 ault Dimensions | |
| Length Cartridge Bay (Lbay) Width Cartridge Bay (Wbay) Area Cartridge Bay (Mbay) Total area of Cartridges (Acarts) Area Lower Volume (Alow) Volume Lower Volume (Vlow) Volume Upper Volume (Vlop) Live storage volume at internal bypass (Vstor) 3.2 Catchment Sediment Loading Estimated TSS Concentration Impervious Road (TSSroad) Estimated TSS Concentration Ervice Regulations (CSS) Bay State (SSS) 3.3 Treatment Efficiency (EFFpre) System Efficiency (EFFpre) System Efficiency (EFFpre) Source Stimated number of cleans per annum (nCleans) | 1.000 m 2.440 m 2.440 m ² 0.905 m ² 1.535 m ² 1059 L 2.440 m ² 2.44 L 1303 L 100 kg/ha/year 300 kg/ha/year 90 kg/year 90 kg/year | 3.0 Estimate sediment mass 3.1 Use tables 2 & 3 below to 5 3.2 Use table 3 below to 6 3.3 Use 0% pretreatment for 1 Std Manhole Dimensions 1050 1500 1500 2050 Std Vault Dimensions 3.4 L x 1.5 W x 1.8 D 4.5 L x 1.5 W x 1.8 D 4.5 L x 1.5 W x 1.8 D 5.6 L x 2.0 W x 1.8 D 5.6 L x 2.0 W x 1.8 D 5.6 L x 2.4 W x 1.8 D | loading (Refer sheet '2. 1 fill in StormFilter Dimen fill in estimated TSS c vaults/manholes with no Cart Bay Length (m) 1.00 1.00 1.00 2.85 3.95 3.95 3.95 5.05 5.05 5.60 TSS (kg/ha/vr) | Aass Load Cales' for mor sions based on number of oncentration. For roar forebay. Use 10-15% pre Without Cart Bay Width (m) 1.67 2.44 3.20 Without Cart Bay Width (m) 1.50 1.50 1.95 1.95 2.40 2.40 | re details) of cartridges as calculati ds with ≥25,000 ypd, u treatmet for vaults/mar forebay Cart Bay Area (m ²) 0.77 1.03 1.67 2.44 3.20 forebay Cart Bay Area (m ²) 4.28 5.93 7.70 9.85 12.12 13.44 | ed in cl 2.2 above see minimum 600kg/h holes with forebays. U Max Number Carts 1 3 4 7 9 Max Number Carts 11 17 23 31 39 44 | talyr se 50% pretreatment for i Cart Bay Length (m) 1.00 1.00 1.00 1.00 1.00 Cart Bay Length (m) 2.30 3.40 3. | Upstream GPT is Enviro With for Cart Bay Width (m) N/A 0.76 1.39 1.83 2.80 Table 2: Standard St With for Cart Bay Width (m) 1.50 2.10 2.10 2.10 2.10 2.40 Table 3: Standar | Pod. Use 75%-90% sy ebay Cart Bay Area (m²) N/A 0.76 1.39 1.83 2.80 cornwater360 Manh ebay Cart Bay Area (m²) 4.14 5.10 7.14 9.45 9.45 9.45 10.80 rd Stornwater360 V | stem efficiency Max Number Carts N/A 2 3 5 7 ole Dimensions Max Number Carts 8 14 18 26 27 33 ault Dimensions | |
| Length Cartridge Bay (Lbay) Width Cartridge Bay (Wbay) Area Cartridge Bay (Mbay) Total area of Cartridges (Acarts) Area Lower Volume (Alow) Volume Lower Volume (Now) Area Upper Volume (Vow) Area Upper Volume (Vupp) Usume Upper Volume (Vupp) Live storage volume at internal bypass (Vstor) Sac Catchment Sediment Loading Estimated TSS Concentration Impervious Road (TSSroof) Estimated TSS Concentration Impervious Road (TSSroof) Estimated TSS Concentration Pervious Area (TSSper) Estimated TSS Concentration Pervious Area (TSSper) Estimated TSS Concentration Pervious Area (TSSper) Estimated TSS Concentration Pervious Area (TSSper) System Efficiencies Pre-treatment Efficiency (EFFpre) System Efficiency (EFFpre) 3.4 Maintenance Requirements Estimated number of cleans per annum (nCleans) | 1.000 m 2.440 m 2.440 m ² 0.905 m ² 1.535 m ² 1059 L 2.440 m ² 2.44 L 1303 L 100 kg/ha/year 300 kg/ha/year 200 kg/ha/year 90 kg/year 50 % 75 % | 3.0 Estimate sediment mass 3.1 Use tables 2 & 3 below to 3.2 Use table 3 below to f 3.3 Use 0% pretreatment for v Std Manhole Dimensions 1050 1200 1500 1500 2050 Std Vault Dimensions 3.4 L x 1.5 W x 1.8 D 4.5 L x 1.5 W x 1.8 D 5.6 L x 2.0 W x 1.8 D 5.6 L x 2.4 W x 1.8 D 5.6 L x 2.4 W x 1.8 D 1.6 L x 2.4 | loading (Refer sheet '2. I fill in StormFilter Dimen iill in estimated TSS c vaults/manholes with no Cart Bay Length (m) 1.00 1.00 1.00 2.85 3.95 3.95 3.95 5.05 5.60 TSS (kg/halyr) 281 - 723 | Ass Load Cales' for more sions based on number of oncentration. For road forebay, Use 10-15% pre Without Cart Bay Width (M) 0.77 1.03 1.67 2.44 3.20 Without Cart Bay Width (M) 1.50 1.50 1.55 1.95 1.95 2.40 2.40 | re details) of cartridges as calculat ds with ≥25,000 ypd, u treatmet for vaults/mar forebay Cart Bay Area (m ²) 0.77 1.03 1.67 2.44 3.20 forebay Cart Bay Area (m ²) 4.28 5.93 7.70 9.85 12.12 13.44 | ed in cl 2.2 above rese minimum 600kg/h tholes with forebays. U Max Number Carts 1 3 4 7 9 9 Max Number Carts 11 17 23 31 39 44 | na/yr se 50% pretreatment for (1.00 1.00 1.00 1.00 1.00 2.30 3.40 4.50 4.50 4.50 | With for With for Cart Bay Width (m) N/A 0.76 1.39 1.33 2.80 Table 2: Standard St With for Cart Bay Width (m) 1.80 1.50 2.10 2.10 2.10 2.40 Table 3: Standard References 1.Denved from Steel | Pod. Use 75%-90% sy Cart Bay Area (m²) N/A 0.76 1.39 1.33 2.80 cornwater/360 Manh chat Bay Area (m²) 4.14 5.10 7.14 9.45 10.80 rd Stormwater/360 V | stem efficiency Max Number Carts N/A 2 3 5 7 ole Dimensions Max Number Carts 8 14 18 26 27 33 aut Dimensions | ntice PD 04 |
| Length Cartridge Bay (Lbay) Width Cartridge Bay (Wbay) Area Cartridge Bay (Mbay) Total area of Cartridges (Acarts) Area Lower Volume (Alow) Volume Lower Volume (Vlow) Area Upper Volume (Vupp) Use Upper Volume (Vupp) Settimated TSS Concentration Impervious Roof (TSSroof) Estimated TSS Concentration Impervious Roof (TSSroof) Estimated TSS Concentration Pervious Area (TSSper) Estimated TSS Concentration Pervious Area (TSSper) System Efficiencies Pre-treatment Efficiency (EFFsys) 3.4 Maintenance Requirements Estimated number of cleans per annum (nCleans) Estimated Maintenance Frequency (Mfreq) | 1.000 m 2.440 m 2.440 m ² 0.905 m ² 1.535 m ² 1059 L 2.440 m ² 244 L 1303 L 100 kg/ha/year 300 kg/ha/year 300 kg/ha/year 30 kg/year 50 % 75 % | 3.0 Estimate sediment mass 3.1 Use tables 2 & 3 below to 3.2 Use table 3 below to f 3.3 Use 0% pretreatment for v Std Manhole Dimensions 1050 1200 1500 1500 1500 1500 1500 1500 1500 1500 14.5 L x 1.5 W x 1.8 D 5.6 L x 2.0 W x 1.8 D 5.6 L x 2.0 W x 1.8 D 5.6 L x 2.0 W x 1.8 D 5.6 L x 2.4 W x 1.8 D 5.6 L x 2.4 W x 1.8 D 16.2 L x 2.4 W x 1.8 D 17.2 L x 2.4 W x 1.8 D 1 | loading (Refer sheet '2. I fill in StormFilter Dimen iil in estimated TSS c vaults/manholes with no Cart Bay Length (m) 1.00 1.00 1.00 2.85 3.95 5.05 5.05 5.60 TSS (kg/halyr) 2.81 - 723 2.49 - 1369 | Aass Load Cales' for mor sions based on number o oncentration. For road forebay, Use 10-15% (pro- Without Cart Bay Width (m) 0.77 1.03 1.67 2.44 3.20 Without Cart Bay Width (m) 1.50 1.55 1.95 2.40 2.40 | e details) of cartridges as calculati ds with ≥25,000vpd, u treatmet for vaults/mar forebay 0.77 1.03 1.67 2.44 3.20 forebay Cart Bay Area (m ²) 4.28 5.93 7.70 9.85 12.12 13.44 | A constraints of the second se | ta/yr se 50% pretreatment for i Cart Bay Length (m) 1.00 1.00 1.00 2.30 3.40 4.50 4.50 4.50 4.50 | Upstream GPT is Enviro With for Cart Bay Width MA 0.76 1.39 1.83 2.80 Table 2: Standard St With for Cart Bay Width (m) 1.80 1.50 2.10 2.10 2.10 2.10 2.10 2.10 2.10 2.1 | Pod. Use 75%-90% sy ebay Cart Bay Area (m²) NA 0.76 1.39 1.83 2.80 Cart Bay Area (m²) 4.14 5.10 7.14 9.45 9.45 10.80 d Stormwater360 V mwater Managemen | stem efficiency Max Number Carts N/A 2 3 5 7 ole Dimensions Max Number Carts 8 14 18 26 27 33 fault Dimensions | cation PD-04- |
| Length Cartridge Bay (Lbay) Width Cartridge Bay (Wbay) Area Cartridge Bay (Mbay) Total area of Cartridges (Acarts) Area Cartridges (Acarts) Area Lower Volume (Alow) Volume Lower Volume (Vlopy) Volume Upper Volume (Vupp) Live storage volume at internal bypass (Vstor) 3.2 Catchment Sediment Loading Estimated TSS Concentration Impervious Road (TSSroad) Estimated TSS Concentration Ervices Area (TSSper) 3.3 Treatment Efficiency (EFFpre) System Efficiency (EFFpre) System Efficiency (EFFsys) Estimated number of cleans per annum (nCleans) Estimated Maintenance Frequency (Mfreq) (A Design Summary | 1.000 m 2.440 m 2.440 m ² 0.905 m ² 1.535 m ² 1059 L 2.440 m ² 2.44 L 1303 L 100 kg/ha/year 300 kg/ha/year 300 kg/ha/year 90 kg/year 90 kg/year 90 kg/year | 3.0 Estimate sediment mass 3.1 Use tables 2 & 3 below to 3.2 Use table 3 below to f 3.3 Use 0% pretreatment for v Std Manhole Dimensions 1050 1500 1500 1500 2050 Std Vault Dimensions 3.4 L x 1.5 W x 1.8 D 4.2 L x 2.0 W x 1.8 D 5.6 L x 2.4 W x 1.8 D 10.2 L x 1.4 W x 1.8 D 10.2 L x 1. | loading (Refer sheet '2, 1 fill in StormFilter Dimen fill in estimated TSS c vaults/manholes with no Cart Bay Length (m) 1.00 1.00 1.00 2.85 3.95 3.95 5.05 5.05 5.05 5.05 5.05 5.05 5.05 5 | Aass Load Cales' for mor sions based on number of oncentration. For roar forebay, Use 10-15% pre Without Cart Bay Width (m) 1.67 2.44 3.20 Without Cart Bay Width (m) 1.50 1.50 1.95 1.95 2.40 2.40 | e details) of cartridges as calculati ds with ≥25,000γpd, u treatmet for vaults/mar forebay Cart Bay Area (m ²) 0.77 1.03 1.67 2.44 3.20 | ed in cl 2.2 above use minimum 600kg/h holes with forebays. U Max Number Carts 1 3 4 7 9 Max Number Carts 11 17 23 31 39 44 | na/yr se 50% pretreatment for u Cart Bay Length (m) 1.00 1.00 1.00 1.00 1.00 2.30 3.40 3.40 3.40 3.40 4.50 4.50 4.50 | Upstream GPT is Enviro With for Cart Bay Width MA 0.76 1.39 1.83 2.80 Table 2: Standard St With for Cart Bay Width (m) 1.80 1.50 2.10 2.10 2.10 2.10 2.10 2.10 2.10 2.1 | Pod. Use 75%-90% sy ebay Cart Bay Area (m²) N/A 0.76 1.39 1.83 2.80 cornwater360 Manh ebay Cart Bay Area (m²) 4.14 5.10 7.14 9.45 9.45 10.80 rd Stornwater360 V mwater Managementer Solutions. Storn | stem efficiency Max Number Carts N/A 2 3 5 7 ole Dimensions Max Number Carts 8 14 18 26 27 33 rault Dimensions tt Inc., Technical Public Filter Product Design I | cation PD-04- Manual |
| Length Cartridge Bay (Lbay) Width Cartridge Bay (Wbay) Area Cartridge Bay (Mbay) Total area of Cartridges (Acarts) Area Cartridges (Acarts) Area Lower Volume (Alow) Volume Lower Volume (Mow) Area Upper Volume (Vupp) Usume Upper Volume (Vupp) Live storage volume at internal bypass (Vstor) 3.2 Catchment Sediment Loading Estimated TSS Concentration Impervious Road (TSSroof) Estimated TSS Concentration Impervious Road (TSSroof) Estimated TSS Concentration Pervious Area (TSSper) Estimated TSS Concentration Pervious Area (TSSper) Estimated TSS Concentration Pervious Area (TSSper) Estimated TSS Concentration Pervious Area (TSSper) System Efficiency (EFFsys) 3.4 Maintenance Requirements Estimated number of cleans per annum (nCleans) Estimated Maintenance Frequency (Mfreq) AD Design Numary | 1.000 m 2.440 m 2.440 m ² 0.905 m ² 1.535 m ² 1059 L 2.440 m ² 2.44 L 1303 L 100 kg/ha/year 300 kg/ha/year 200 kg/ha/year 90 kg/year 50 % 75 % | 3.0 Estimate sediment mass 3.1 Use tables 2 & 3 below to 3.2 Use table 3 below to f 3.3 Use 0% pretreatment for v Std Manhole Dimensions 1050 1200 1500 1500 2050 Std Vault Dimensions 3.4 L x 1.5 W x 1.8 D 4.5 L x 1.5 W x 1.8 D 4.5 L x 2.0 W x 1.8 D 5.6 L x 2.0 W x 1.8 D 5.6 L x 2.4 W x 1.8 D 1.6 L x 2.4 W x 1.8 D Call L x 1.5 W x 1.8 D 1.6 L x 2.4 W x 2 | loading (Refer sheet '2, 1 fill in StormFilter Dimen fill in estimated TSS c vaults/manholes with no Cart Bay Length (m) 1.00 1.00 1.00 1.00 2.85 3.95 5.05 5.05 5.05 5.05 5.05 5.05 5.05 5 | Aass Load Cales' for mor sions based on number of oncentration. For road forebay. Use 10-15% pre Without Cart Bay Width (m) 0.77 1.03 1.67 2.44 3.20 Without Cart Bay Width (m) 1.50 1.50 1.50 1.55 1.95 2.40 2.40 | re details) of cartridges as calculation ds with ≥25,000 ypd, u treatmet for vaults/mar forebay Cart Bay Area (m ²) 0.77 1.03 1.67 2.44 3.20 Cart Bay Area (m ²) 4.28 5.93 7.70 9.85 12.12 13.44 | ed in cl 2.2 above use minimum 600kg/h tholes with forebays. U Max Number Carts 1 3 4 7 9 Max Number Carts 11 17 23 31 39 44 | 18/yr se 50% pretreatment for r Cart Bay Length (m) 1.00 1.00 1.00 1.00 1.00 Cart Bay Length (m) 2.30 3.40 3.40 4.50 4.50 4.50 1.50 1.50 | upstream GPT is Enviro With for Cart Bay Width (m) NA 0.76 1.39 1.83 2.80 Table 2: Standard St With for 0.76 Cart Bay Width (m) 1.80 1.50 2.10 2.10 2.10 2.10 1.30 Table 3: Standard References 1. Derived from Stor 1. Derived from Stor 002.0 2. Contech Stormwa 3. Table 4.4. Techni | Pod. Use 75%-90% sy cart Bay Area (m²) N/A 0.76 1.39 1.83 2.80 corrtwater360 Manh bebay Cart Bay Area (m²) 4.14 5.10 7.14 9.45 10.80 rd Stornwater360 V mwater Management ter Solutions, Storm ale Publication 10.2 | stem efficiency Max Number Carts N/A 2 3 5 7 Ole Dimensions Max Number Carts 8 14 18 26 27 33 ault Dimensions tt Inc., Technical Publik Filter Product Design 1 de Edition, May 2003. | cation PD-04- Manual. Auckland |
| Length Cartridge Bay (Lbay) Width Cartridge Bay (Wbay) Area Cartridge Bay (Mbay) Total area of Cartridges RACarts) Area Lower Volume (Alow) Volume Lower Volume (Now) Area Upper Volume (Vow) Area Upper Volume (Vupp) Use the storage volume at internal bypass (Vstor) Live storage volume at internal bypass (Vstor) Estimated TSS Concentration Impervious Road (TSSroof) Estimated TSS Concentration Impervious Road (TSSroof) Estimated TSS Concentration Pervious Area (TSSper) Estimated TSS Concentration Pervious Area (TSSper) Estimated Total TSS Load (TSSioad) System Efficiency (EFFpre) System Efficiency (EFFpre) System Efficiency (EFFpre) System Efficiency (EFFsys) At Maintenance Requirements Estimated number of cleans per annum (nCleans) Estimated number of cleans per annum (nCleans) | 1.000 m 2.440 m 2.440 m ² 0.905 m ² 1.535 m ² 1059 L 2.440 m ² 2.44 L 1303 L 100 kg/ha/year 300 kg/ha/year 300 kg/ha/year 300 kg/ha/year 50 % 75 % 0.28 42.857 months | 3.0 Estimate sediment mass 3.1 Use tables 2 & 3 below to 3.2 Use table 3 below to f 3.3 Use 0% pretreatment for v Std Manhole Dimensions 1050 1200 1500 1500 2050 Std Vault Dimensions 3.4 L x 1.5 W x 1.8 D 4.5 L x 1.5 W x 1.8 D 5.6 L x 2.0 W x 1.8 D 5.6 L x 2.0 W x 1.8 D 5.6 L x 2.4 W x 1.8 D 5.6 L x 2.4 W x 1.8 D Commercial Residential (low) Residential (low) Residential (low) | loading (Refer sheet '2. I. fill in StormFilter Dimen iill in estimated TSS c vaults/manholes with no Cart Bay Length (m) 1.00 1.00 1.00 2.85 3.95 5.05 5.05 5.05 5.60 TSS (kghalyr) 281 - 723 242 - 1369 60 - 340 97 - 547 133 - 755 | Aass Load Cales' for mori sions based on number o oncentration. For road forebay, Use 10-15% (pro- Without Cart Bay Width (m) 0.77 1.03 1.67 2.44 3.20 Without Cart Bay Width (m) 1.50 1.55 1.95 2.40 2.40 | e details) of cartridges as calculat ds with ≥25,000vpd, u treatmnet for vaults/mar forebay Cart Bay Area (m ²) 0.77 1.03 1.67 2.244 3.20 forebay Cart Bay Area (m ²) 4.28 5.93 7.70 9.85 12.12 13.44 | ed in cl 2.2 above ise minimum 600kg/h tholes with forebays. U Max Number Carts 1 3 4 7 9 Max Number Carts 11 17 23 31 39 44 | na/yr se 50% pretreatment for 1 1.00 1.00 1.00 1.00 2.30 3.40 4.50 4.50 4.50 | With for With for Cart Bay Width (m) N/A 0.76 1.39 1.33 2.80 Table 2: Standard St With for Cart Bay Width (m) 1.80 1.50 2.10 2.10 2.10 2.10 Zable 3: Standard References 1. Derived from Stor 3. Table 4.4, Techni Regional Council | Pod. Use 75%-90% sy Cart Bay Area (m²) N/A 0.76 1.39 1.33 2.80 cart Bay Area (m²) 4.14 5.10 7.14 9.45 10.80 rd Stormwater 360 V mwater Management ater Solutions, Storm cal Publication 10, 2 | stem efficiency Max Number Carts N/A 2 3 5 7 ole Dimensions Max Number Carts 8 14 18 26 27 33 'ault Dimensions t Inc., Technical Public Filter Product Design I nd Edition, May 2003, | cation PD-04- Manual Auckland |
| Length Cartridge Bay (Lbay) Width Cartridge Bay (Wbay) Area Cartridge Bay (Mbay) Total area of Cartridges (Acarts) Area Lower Volume (Alow) Volume Lower Volume (Alow) Volume Lower Volume (Vlup) Live storage volume at internal bypass (Vstor) 3.2 Catchment Sediment Loading Estimated TSS Concentration Impervious Road (TSSroof) Estimated TSS Concentration Previous Area (TSSper) Estimated TSS Concentration Previous Area (TSScod) Estimated TSS Concentration Previous Area (TSScod) Estimated TSS Concentration Previous Area (TSSper) System Efficiency (EFFsps) System Efficiency (EFFsps) System Efficiency (EFFsps) Sustem Efficiency (Mfreq) Estimated number of cleans per annum (nCleans) Estimated Maintenance Frequency (Mfreq) Besign Water Quality Treatment Flowrate (Qwq) StormFilter Design WQ Treatment flowrate (QSP) | 1.000 m 2.440 m 2.440 m ² 0.905 m ² 1.535 m ² 1059 L 2.440 m ² 244 L 1303 L 100 kg/ha/year 300 kg/ha/year 200 kg/ha/year 90 kg/year 50 % 75 % 0.28 42.857 months 7.083 L/s 7.100 L/s 7.610 L/s | 3.0 Estimate sediment mass 3.1 Use tables 2 & 3 below to 3.2 Use table 3 below to f 3.3 Use 0% pretreatment for v Std Manhole Dimensions 1050 1200 1500 1500 1500 3.4 L x1.5 W x1.8 D 4.5 L x1.5 W x1.8 D 5.6 L x2.0 W x1.8 D 5.6 L x2.0 W x1.8 D 5.6 L x2.0 W x1.8 D 5.6 L x2.4 W x1.8 D 6.2 L x2.4 W x1.8 D Commercial Residential (low) Residential (low) | loading (Refer sheet '2, 1 fill in StormFilter Dimen iil in estimated TSS c vaults/manholes with no Cart Bay Length (m) 1.00 1.00 1.00 2.85 3.95 5.05 5.05 5.05 5.05 5.05 5.05 5.05 5 | Aass Load Cales' for more isons based on number of oncentration. For roar forebay. Use 10-15% pre- Without Cart Bay Width (m) 0.77 1.03 1.67 2.44 3.20 Without Cart Bay Width (m) 1.50 1.55 1.95 | e details) of cartridges as calculati ds with ≥25,000ypd, u treatment for vaults/mar forebay Cart Bay Area (m ²) 0.77 1.03 1.67 2.244 3.20 forebay Cart Bay Area (m ²) 4.28 5.93 7.70 9.85 12.12 13.44 | A standard s | na/yr se 50% pretreatment for of 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.0 | Upstream GPT is Enviro With for Cart Bay Width (n) 1.33 2.80 Table 2: Standard St With for Cart Bay Width (n) 1.80 1.50 2.10 2.10 2.10 2.40 Table 3: Standard St 1. Derived from Stor 0.2.0 2. Contech Stormwa 3. Table 4.4, Techni Regional Council | Pod. Use 75%-90% sy ebay Cart Bay Area (m²) N/A 0.76 1.39 1.83 2.80 ornwater360 Manh ebay Cart Bay Area (m²) 4.14 5.10 7.14 9.45 9.45 10.80 rd Stornwater360 V mwater Management ater Solutions, Storm cal Publication 10, 2 | stem efficiency Max Number Carts N/A 2 3 5 7 ole Dimensions Max Number Carts 8 14 18 26 27 33 aut Dimensions ti Inc., Technical Public Filter Product Design I nd Edition, May 2003, | cation PD-04- Manual. Auckland |
| Length Cartridge Bay (Lbay) Width Cartridge Bay (Wbay) Area Cartridge Bay (Mbay) Total area of Cartridges (Acarts) Area Lower Volume (Alow) Volume Lower Volume (Vlow) Area Upper Volume (Vlow) Area Upper Volume (Vupp) Use to the point of the terminal bypass (Vstor) <u>3.2 Catchment Sediment Loading</u> Estimated TSS Concentration Impervious Road (TSSroad) Estimated TSS Concentration Impervious Road (TSSroad) Estimated TSS Concentration Impervious Road (TSSroad) Estimated TSS Concentration Pervious Road (TSSroad) Estimated TSS Concentration Pervious Area (TSSper) <u>Estimated TSS Concentration Streat (TSSIcad)</u> Sistemet Efficiency (EFFse) System Efficiency (EFFse) <u>3.4 Maintenance Requirements</u> Estimated number of cleans per annum (nCleans) <u>Estimated Maintenance Frequency (Mfreq)</u> Design Water Quality Treatment Flowrate (Qwq) StormFilter Design flowrate at internal bypass (Qbypass) | 1.000 m 2.440 m 2.440 m ² 0.905 m ² 1.535 m ² 1059 L 2.440 m ² 2.440 m ² 2.44 L 1303 L 100 kg/ha/year 300 kg/ha/year 200 kg/ha/year 300 kg/h | 3.0 Estimate sediment mass 3.1 Use tables 2 & 3 below to 3.2 Use table 3 below to f 3.3 Use 0% pretreatment for v Std Manhole Dimensions 1050 1200 1500 2050 Std Vault Dimensions 3.4 L x 1.5 W x 1.8 D 4.5 L x 1.5 W x 1.8 D 4.2 L x 2.0 W x 1.8 D 5.6 L x 2.4 W x 1.8 D 5.6 L x 2.4 W x 1.8 D Land Use Road Commercial Residential (low) Residential (low) Terraced Bush | loading (Refer sheet '2. 1 fill in StormFilter Dimen fill in estimated TSS c vaults/manholes with no Cart Bay Length (m) 1.00 1.00 1.00 1.00 2.85 3.95 3.95 5.05 5.05 5.05 5.05 5.05 5.05 5.05 5 | Aass Load Cales' for more soins based on number of oncentration. For roar forebay. Use 10-15% pre Without Cart Bay Width (m) 1.57 2.44 3.20 Without Cart Bay Width (m) 1.55 1.95 2.40 2.40 | e details) of cartridges as calculati ds with 225,000ypd, u treatmet for vaults/mar forebay Cart Bay Area (m ²) 1.03 1.67 2.44 3.20 Cart Bay Area (m ²) 4.28 5.93 7.70 9.85 12.12 13.44 | ed in cl 2.2 above isse minimum 600kg/h holes with forebays. U Max Number Carts 1 3 4 7 9 Max Number Carts 11 17 23 31 39 44 | nalyr se 50% pretreatment for u Cart Bay Length (m) 1.00 1.00 1.00 1.00 2.30 3.40 3.40 3.40 4.50 4.50 4.50 | Upstream GPT is Enviro With for Cart Bay Width (m) 0.76 1.39 1.83 2.80 Table 2: Standard St With for Cart Bay Width (m) 1.83 2.80 Table 2: Standard St With for Cart Bay Width (m) 1.50 2.10 2.10 2.10 2.10 2.10 2.10 3.1 Sable 3: Standar References 1. Derived from Stor 02.0 2. Contech Stormwa 3. Table 4.4, Techni Regional Council | Pod. Use 75%-90% sy cart Bay Area (m²) N/A 0.76 1.39 1.83 2.80 corrmwater360 Manh ebay Cart Bay Area (m²) 4.14 5.10 7.14 9.45 10.80 rd Stornwater360 V mwater Managementater Solutions, Storm cal Publication 10, 2 | stem efficiency Max Number Carts N/A 2 3 5 7 ole Dimensions Max Number Carts 8 14 18 26 27 33 fault Dimensions tt Inc., Technical Public Filter Product Design I nd Edition, May 2003, | cation PD-04- Vanual. Auckland |
| Length Cartridge Bay (Lbay) Width Cartridge Bay (Wbay) Area Cartridge Bay (Mbay) Total area of Cartridges RACarts) Area Lower Volume (Alow) Volume Lower Volume (Vlow) Area Upper Volume (Vlow) Area Upper Volume (Vupp) Use storage volume at internal bypass (Vstor) <u>3.2 Catchment Sediment Loading</u> Estimated TSS Concentration Impervious Roof (TSSroof) Estimated TSS Concentration Impervious Roof (TSSroof) Estimated TSS Concentration Pervious Area (TSSper) <u>Estimated TSS Concentration Pervious Area (TSSper)</u> <u>Estimated TSS Concentration Pervious Area (TSSper)</u> System Efficiencies Pre-treatment Efficiency (EFFpre) System Efficiency (EFFpre) System Efficiency (EFFpre) <u>3.4 Maintenance Requirements</u> Estimated number of cleans per annum (nCleans) <u>Estimated number of cleans per annum (nCleans)</u> Estimated number of cleans per annum (nCleans) Estimated number of cleans | 1.000 m 2.440 m 2.440 m ² 0.905 m ² 1.535 m ² 1059 L 2.440 m ² 2.44 L 1303 L 100 kg/ha/year 300 kg/ha/year 200 kg/ha/year 300 kg/ha/year 90 kg/year 90 kg/year 90 kg/sear 90 | 3.0 Estimate sediment mass 3.1 Use tables 2 & 3 below to 3.2 Use table 3 below to f 3.3 Use 0% pretreatment for v Std Manhole Dimensions 1050 1200 1500 1500 2050 Std Vault Dimensions 3.4 L x 1.5 W x 1.8 D 4.5 L x 1.5 W x 1.8 D 5.6 L x 2.0 W x 1.8 D 5.6 L x 2.4 W x 1.8 D 5.6 L x 2.4 W x 1.8 D 5.6 L x 2.4 W x 1.8 D Commercial Residential (low) Residential | Ioading (Refer sheet '2. I. fill in StormFilter Dimen iill in estimated TSS c vaults/manholes with no Cart Bay Length (m) 1.00 1.00 1.00 2.85 3.95 5.05 5.05 5.05 5.60 TSS (kg/ha/yr) 281 - 723 242 - 1369 60 - 340 97 - 547 133 - 755 2.6 - 146 80 - 588 | Aass Load Cales' for mori sions based on number o oncentration. For road forebay, Use 10-15% pro- Without Cart Bay Width (m) 0.77 1.03 1.67 2.44 3.20 Without Cart Bay Width (m) 1.50 1.50 1.95 2.40 2.40 | e details) of cartridges as calculat ds with ±25,000vpd, u treatment for vaults/mar forebay 0.77 1.03 1.67 2.244 3.20 forebay Cart Bay Area (m ²) 4.28 5.93 7.70 9.85 12.12 13.44 | ed in cl 2.2 above rese minimum 600kg/h tholes with forebays. U Max Number Carts 1 3 4 7 9 Max Number Carts 11 17 23 31 39 44 | na/yr se 50% pretreatment for (1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.0 | Upstream GPT is Enviro With for Cart Bay Width (m) NA 0.76 1.39 1.83 2.80 Table 2: Standard St With for Cart Bay Width (m) 1.80 1.50 2.10 2.10 2.10 2.10 2.10 2.10 1.Derived from Stor 002.0 2. Contech Stormwa 3. Table 4.4. Techni Regional Council | Pod. Use 75%-90% sy cebay Cart Bay Area (m²) N/A 0.76 1.39 1.33 2.80 comwater/360 Manh cebay Cart Bay Area (m²) 4.14 5.10 7.14 9.45 10.80 rd Stormwater/360 V mwater Management ater Solutions, Storm cal Publication 10, 2 | stem efficiency Max Number Carts N/A 2 3 5 7 ole Dimensions Max Number Carts 8 14 18 26 27 33 'ault Dimensions ti Inc., Technical Publie Filter Product Design I nd Edition, May 2003, | cation PD-04- Manual. Auckland |
| Length Cartridge Bay (Lbay) Width Cartridge Bay (Wbay) Area Cartridge Bay (Mbay) Total area of Cartridges (Acarts) Area Cower Volume (Alow) Volume Lower Volume (Vlow) Volume Lower Volume (Vlow) Volume Upper Volume (Vupp) Live storage volume at internal bypass (Vstor) 3.2 Catchment Sediment Loading Estimated TSS Concentration Impervious Road (TSSroof) Estimated TSS Concentration Impervious Road (TSSroof) Estimated TSS Concentration Impervious Road (TSSroof) Estimated TSS Concentration Impervious Area (TSSper) Estimated TSS Concentration Pervious Area (TSSper) Estimated TSS Concentration Pervious Area (TSSper) Estimated TSS Concentration Pervious Area (TSSper) System Efficiency (EFFsps) System Efficiency (EFFsps) System Efficiency (EFFsps) StormFilter Design Wa Treatment Flowrate (Qwq) StormFilter Design Wa Treatment flowrate (Qwq) StormFilter Design Wa Treatment flowrate (Qsp) StormFilter Design Was requised (nTOTAL) Treatment Flux per cartridge (FLUX) | 1.000 m 2.440 m 2.440 m ² 0.905 m ² 1.535 m ² 1059 L 2.440 m ² 2.44 L 1303 L 100 kg/ha/year 300 kg/ha/year 200 kg/ha/year 200 kg/ha/year 300 | 3.0 Estimate sediment mass 3.1 Use tables 2 & 3 below to 3.2 Use table 3 below to f 3.3 Use 0% pretreatment for v Std Manhole Dimensions 1050 1200 1500 1500 1500 1500 1500 1500 1500 1500 1500 1500 1500 1500 1800 2050 Std Vault Dimensions 3.4 L x 1.5 W x 1.8 D 5.6 L x 2.0 W x 1.8 D 5.6 L x 2.0 W x 1.8 D 5.6 L x 2.0 W x 1.8 D 5.6 L x 2.4 W x 1.8 D 6.2 L x 2.4 W x 1.8 D Land Use Road Commercial Residential (low) Residential (low) Residential (low) Crass Roof Deci | loading (Refer sheet '2, I fill in StormFilter Dimen iil in estimated TSS c vaults/manholes with no Cart Bay Length (m) 1.00 1.00 1.00 1.00 2.85 3.95 5.05 5.05 5.05 5.60 TSS (kg/halyr) 2.81 - 723 2.42 - 1369 6.0 - 340 9.7 - 547 1.33 - 755 2.6 - 146 8.0 - 588 5.0-110 (1) | Aass Load Cales' for more Signs based on number of Signs based on number of Signs based on number of Signs based on number of Signs based on number of Without Cart Bay Width (m) 1.67 2.44 3.20 Without Cart Bay Width (m) 1.50 1.55 1.95 1.95 1.95 2.40 2.50 2 | e details) of cartridges as calculati ds with ≥25,000/vpd, u treatment for vaults/mar forebay Cart Bay Area (m ²) 0.77 1.03 1.67 2.244 3.20 forebay Cart Bay Area (m ²) 4.28 5.93 7.70 7.70 9.85 12.12 13.44 | A standard s | ta/yr se 50% pretreatment for i Cart Bay Length (m) 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.0 | upstream GPT is Enviro With for Cart Bay Width (m) 1.33 2.80 Table 2: Standard St With for Cart Bay Width (m) 1.80 1.50 2.10 2.20 Contech Stornwa 3. Table 4-4, Techni Regional Council | Pod. Use 75%-90% sy ebay Cart Bay Area (m²) NA 0.76 1.39 1.83 2.80 comwater360 Manh rebay Cart Bay Area (m²) 4.14 5.10 7.14 9.45 9.45 10.80 rd Stormwater360 V mwater Managemen ater Solutions, Storm cal Publication 10, 2 | stem efficiency Max Number Carts N/A 2 3 5 7 ole Dimensions Max Number Carts 8 14 18 26 27 33 autt Dimensions tt Inc., Technical Public Filter Product Design I nd Edition, May 2003, | cation PD-04- Wanual. Auckland |
| Length Cartridge Bay (Lbay) Width Cartridge Bay (Wbay) Area Cartridge Bay (Mbay) Total area of Cartridges Re(Acarts) Area Lower Volume (Alow) Volume Lower Volume (Vlow) Volume Upper Volume (Vlop) Live storage volume at internal bypass (Vstor) 3.2 Catchment Sediment Loading Estimated TSS Concentration Impervious Road (TSSroad) Estimated TSS Concentration Ervice Response Bay (Sterresponse) System Efficiency (EFFpre) System Efficiency (EFFpre) System Efficiency (EFFsys) Estimated number of cleans per annum (nCleans) Estimated number of cleans per annum (nCleans) Estimated Maintenance Frequency (Mfreq) StormFilter Design Water Quality Treatment Flowrate (Qwg) StormFilter Design Howrate at internal bypass (Qbypass) Number of StormFilter Cartridges required (nTOTAL) Treatment Flux per cartridge (FLUX) Restrictor Disc Size (d) | 1.000 m 2.440 m 2.440 m ² 0.905 m ² 1.535 m ² 1059 L 2.440 m ² 2.44 L 1303 L 100 kg/ha/year 300 kg/ha/year 300 kg/ha/year 300 kg/ha/year 90 kg/ha/ha/ha/ha/ha/ha/ha/ha/ha/ha/ha/ha/ha/ | 3.0 Estimate sediment mass 3.1 Use tables 2 & 3 below to 3.2 Use table 3 below to f 3.3 Use 0% pretreatment for v Std Manhole Dimensions 1050 1200 1500 1500 2050 Std Vault Dimensions 3.4 L x 1.5 W x 1.8 D 4.5 L x 1.5 W x 1.8 D 4.5 L x 1.5 W x 1.8 D 5.6 L x 2.0 W x 1.8 D 5.6 L x 2.0 W x 1.8 D 5.6 L x 2.0 W x 1.8 D 5.6 L x 2.4 W x 1.8 D Commercial Residential (low) Residential (ligh) Terraced Bush Grass Roof Pasture | loading (Refer sheet '2, 1 fill in StormFilter Dimen iil in estimated TSS c vaults/manholes with no Cart Bay Length (m) 1.00 1.00 1.00 1.00 2.85 3.95 5.05 5.05 5.05 5.05 5.05 5.05 5.05 5 | Aass Load Cales' for more Sions based on number of oncentration. For roar forebay. Use 10-15% pre- Without Cart Bay Width (m) 1.67 2.44 3.20 Without Cart Bay Width (m) 1.50 1.55 1.95 1.95 1.95 2.40 2.40 2.40 2.40 2.40 | e details) of cartridges as calculation ds with 225,000ypd, u treatment for vaults/mar forebay Cart Bay Area (m ²) 0.77 1.03 1.67 2.44 3.20 | Max Number Carts 1 3 4 7 9 Max Number Carts 1 3 4 7 9 Max Number Carts 11 17 23 31 39 44 | na/yr se 50% pretreatment for u 1.00 1.00 1.00 1.00 1.00 1.00 1.00 Cart Bay Length (m) 2.30 3.40 3.40 3.40 3.40 3.40 3.40 3.40 3 | Upstream GPT is Enviro With for Cart Bay Width (m) N/A 0.76 1.39 1.83 2.80 Table 2: Standard St With for Cart Bay Width (m) 1.80 1.50 2.10 2.0 2.0 2.0 2.0 2.0 2.0 2.0 2. | Pod. Use 75%-90% sy ebay Cart Bay Area (m²) N/A 0.76 1.39 1.83 2.80 cornwater360 Manh ebay Cart Bay Area (m²) 4.14 5.10 7.14 9.45 9.45 9.45 10.80 rd Stornwater360 V mwater Managemer ater Solutions, Storm cal Publication 10, 2 | stem efficiency Max Number Carts N/A 2 3 5 7 ole Dimensions Max Number Carts 8 14 18 26 27 33 rault Dimensions tt Inc., Technical Public Filter Product Design In d Edition, May 2003, | cation PD-04- Manual Auckland |
| Length Cartridge Bay (Lbay) Width Cartridge Bay (Wbay) Area Cartridge Bay (Mbay) Total area of Cartridges RACarts) Area Lower Volume (Alow) Volume Lower Volume (Vlow) Area Upper Volume (Vlow) Area Upper Volume (Vupp) Use storage volume at internal bypass (Vstor) <u>3.2 Catchment Sediment Loading</u> Estimated TSS Concentration Impervious Road (TSSroof) Estimated TSS Concentration Impervious Road (TSSroof) Estimated TSS Concentration Pervious Road (TSSroof) Estimated TSS Concentration Pervious Road (TSSroof) Estimated TSS Concentration Pervious Area (TSSper) <u>8.3 Treatment Efficiencies</u> Pre-treatment Efficiency (EFFpre) System Efficiency (EFFpre) System Efficiency (EFFpre) Sustem Efficiency (Mfreq) <u>8.0 Design Summary</u> Design Water Quality Treatment Flowrate (QsP) StormFilter Design flowrate at internal bypass (Qbypass) Number of StormFilter Cartridges required (nTOTAL) Treatment Flic apprecipation (nTOTAL) Restrictor Disc Size (d) Maximum Hydraulic Effect (Imax) | 1.000 m 2.440 m 2.440 m ² 0.905 m ² 1.535 m ² 1059 L 2.440 m ² 2.44 L 1303 L 100 kg/ha/year 200 kg/ha/year 200 kg/ha/year 200 kg/ha/year 90 kg/year 90 kg/ba/year 90 kg/year 90 kg/ba/year 90 kg/ba/ba/ba/ba/ba/ba/ba/ba/ba/ba/ba/ba/ba/ | 3.0 Estimate sediment mass 3.1 Use tables 2 & 3 below to 3.2 Use table 3 below to f 3.3 Use 0% pretreatment for v Std Manhole Dimensions 1050 1200 1500 1500 2050 Std Vault Dimensions 3.4 L x 1.5 W x 1.8 D 4.5 L x 1.5 W x 1.8 D 4.5 L x 2.0 W x 1.8 D 5.6 L x 2.0 W x 1.8 D 5.6 L x 2.0 W x 1.8 D 5.6 L x 2.4 W x 1.8 D 4.2 L x 2.0 W x 1.8 D 5.6 L x 2.4 W x 1.8 D 4.2 L x 2.0 W x 1.8 D 5.6 L x 2.4 W x 1.8 D 4.2 L x 2.0 W x 1.8 D 5.6 L x 2.4 W x 1.8 D 4.2 L x 2.0 W x 1.8 D 5.6 L x 2.4 W x 1.8 D 4.2 L x 2.0 W x 1.8 D 5.6 L x 2.4 | loading (Refer sheet '2. I fill in StormFilter Dimen iill in estimated TSS c vaults/manholes with no 1.00 1.00 1.00 1.00 2.85 3.95 3.95 5.05 5.60 TSS (kg/halyr) 2.81 · 723 2.42 · 1369 6.0 · 340 9.7 · 547 1.33 · 755 2.66 · 146 8.0 · 588 5.0-110 (7) 1.03 · 583 1.045 [3] | Aass Load Cales' for more soins based on number of oncentration. For road forebay. Use 10-15% pre Without Cart Bay Width (m) 1.03 1.67 2.44 3.20 Without Cart Bay Width (m) 1.50 1.50 1.50 1.50 1.55 2.40 2.40 | e details) of cartridges as calculation ds with 225,000 ypd, u treatmet for vaults/mar forebay Cart Bay Area (m ²) 1.03 1.67 2.44 3.20 Cart Bay Area (m ²) 4.28 5.93 7.70 9.85 12.12 13.44 | ed in cl 2.2 above use minimum 600kg/h holes with forebays. U Max Number Carts 1 3 4 7 9 Max Number Carts 11 17 23 31 39 44 | na/yr se 50% pretreatment for u 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.0 | Upstream GPT is Enviro With for Cart Bay Width (m) N/A 0.76 1.39 1.83 2.80 Table 2: Standard St With for Cart Bay Width (m) 1.80 1.50 2.10 2.10 2.10 2.10 3. Table 3: Standard St References 3. Table 4.4, Techni Regional Council | Pod. Use 75%-90% sy ebay (art Bay Area (m ²) N/A 0.76 1.39 1.83 2.80 corruwater360 Manh ebay Cart Bay Area (m ²) 4.14 5.10 7.14 9.45 9.45 10.80 rt Solutions, Storm cal Publication 10, 2 | stem efficiency Max Number Carts N/A 2 3 5 7 Ole Dimensions Max Number Carts 8 14 18 26 27 33 fault Dimensions It Inc., Technical Public Filter Product Design I nd Edition, May 2003, | cation PD-04- Manual. Auckland |

Maximum Hydraulic Effect (hmax) Estimated Maintenance Frequency (Mfreq) 43 months

CSF 0054; StormFilter Flow-Based Sizing - Other - Revision 2.2 - Updated 10th December 2018



| Project | Ecological Memo regarding proposed Oruaiti School Car Park Redevelopment |
|-------------------|---|
| Matters addressed | Ecology |
| Date issued | 05/06/2024 |
| Attention of | Riaan De Swardt |

BACKGROUND

Wild Ecology were engaged by Land Development and Engineering (LDE) to provide ecological input relating to the proposed Oruaiti School Car Park Redevelopment. It is understood that the required car park redevelopment work will take place within a 100m setback from a natural inland wetland (as defined under National Policy Statement for Freshwater Management (NPS-FM 2020) and therefore the works require consideration under National Environmental Standards for Freshwater (NES-FW (2020).

A desktop study was completed which included a review of relevant information relating to the ecological features contained within the development footprint and immediate surrounds, including a review of ecological considerations under operative Far North District Plan (oFNDP), Northland Regional Council (NRC) ecological overlays, National Policy Statement for Freshwater Management (NPS-FM 2020), National Environmental Standards for Freshwater (NES-FW 2020) followed by a review of google aerial imagery, and LIDAR data (NRC 2020).

SITE DESCRIPTION

The proposed car park redevelopment works are to take place at Ouraiti School as shown under Figure 1. The ecological baseline of the site and wider surrounds is that formed of Ouraiti River and its riparian margins. Under NRC Flood Hazard Maps the area has been mapped as being subject to 10-Year, 50-Year and 100-Year River Flood Hazard Zones. A small natural inland wetland area is located approximately 30m south of the proposed new car park area separated by State Highway 10.

It is understood that the existing flow direction is largely from north to south via overland flow paths and through a series of existing culverts. LDE Stormwater Neutrality Report states that it is assumed that a Ø900mm concrete pipe culvert crosses State Highway 10 and discharges into a downstream wetland, which continues to flow into the tidal zone of the Oruaiti River.



Figure 1: Showing the location of Oruaiti School, the location for the proposed new car park footprint, natural inland wetland area extent and NRC River Flood Hazard Zones – Regionwide Models

NES-FW (2020) CONSIDERATIONS

It is understood that the proposed car park construction will involve stormwater discharges to the stormwater catchpit which has hydrological connectivity to the mapped natural inland wetland area. According to LDE reporting the proposal would result in 15L/s increase is discharge which has been assessed as negligible compared to the expected overall flows from the river catchment. The proposal will not require for the existing culvert structure to be replaced as it has sufficient capacity to accommodate for the proposed increased discharges. It is understood that the proposal will not require for any earthworks or vegetation clearance to take place within a 10m setback of the identified natural inland wetland area.

Earthworks will occur a minimum 30m north of the mapped natural inland wetland area and stormwater discharged will occur within 100m setback of the wetland. A basic assessment in relation to consenting obligations under NES-FW (2020) is briefly discussed under Table 1 below.

| NES-FW (2020) Regulation | Comment/assessment |
|--|--|
| Drainage of natural wetlands | Consent under condition 52(1) is not required/applicable. While earthworks associated with the car park re-development will take place within a |
| 52 Non-complying activities | 100m setback of the identified natural inland wetland area located approximately 30m south of the works |
| (1) Earthworks outside, but within a | footprint, it will not result or is not likely to result in |
| 100 m setback from, a natural inland | complete or partial drainage of the natural inland |
| wetland is a non-complying activity if it— | wetland feature should appropriate erosion and sediment control measures are constructed and |
| (a) results, or is likely to result, in the complete or partial drainage of all or part of a natural inland wetland; and | maintained in accordance with the principles outlined in relevant expert reporting prepared for the proposal and best practice. |
| (b) does not have another status under any of regulations 38 to 51. | Site earthworks should be undertaken during suitable weather conditions and dry periods when the wetland area and watercourses are likely to be naturally dry to avoid any potential residual risk to aquatic organisms during the construction of the accessway. All sediment and erosion controls to be installed as per GD05 and associated technical reporting prepared for the site development. |
| 52(2) The taking, use, damming, diversion, or discharge of water outside, but within a 100 m setback from, a natural wetland is a non- complying activity if it— | Consent under Regulation 52(2) is not required/applicable. The diversion of water and discharge of water outside but within a 100m setback from the natural inland wetland area associated with stormwater discharges and diversions as part of site development will not result or are not likely to result in complete or partial drainage of the natural inland wetland features identified. |

Table 1: Assessment of NES-FW consenting obligations

| (a) results, or is likely to result, in the complete or partial drainage of all or part of a natural wetland; and (b) does not have another status under any of regulations 38 to 51. | Due to the incremental increase in impervious surface associated with the site's development, the overall volume of water entering the aquatic features is not expected to increase to a detectable level. These discharges are not likely to change the water level range or hydrological function of the wetland area. |
|--|--|
| Other activities Regulation 54 amended (Non- complying activities) 54(a) vegetation clearance within, or within a 10 m setback from, a natural | Consent under Regulation 54(a) is not required/applicable as the proposal will not require for any vegetation clearance to take place within a 10m setback from the identified natural inland wetland area. |
| inland wetland: | Consent under Regulation 54(b) is not |
| 10 m setback from, a natural inland wetland | required/applicable as the proposal will not require for any earthworks to take place within a 10m setback from the identified natural inland wetland area. |
| 54(c) the taking, use, damming, or diversion of water within, or within a 100 m setback from, a natural inland wetland if— (i) there is a hydrological connection between the taking, use, damming, or diversion and the wetland; and | Consent under Reg 54(c) is not required/applicable. While the stormwater diversions associated with the site development will occur within a 100m setback from the identified wetland area and may have a hydrological connection with these areas, they will not change or are unlikely to change the water level range or hydrological function of the wetland. |
| (ii) the taking, use, damming, or diversion will change, or is likely to change, the water level range or hydrological function of the wetland. | |
| 54(d) the discharge of water into water within, or within a 100 m setback from, a natural inland wetland if— (i) there is a hydrological connection between the discharge and the wetland; and | Consent under Reg 54(d) is not required/applicable. While stormwater will be discharged to water and may have a hydrological connection to the wetland and will likely enter the wetland, the incremental increase in impervious surfaces and discharge (15L/s) associated with the car park development, the overall volume of water entering the aquatic features is not expected to increase to any detectable level. These discharges are not likely to change the water level range or hydrological function of the wetland areas. |

| (ii) the discharge will enter the wetland; and | |
|---|--|
| (iii) the discharge will change, or is likely to change, the water level range or hydrological function of the wetland | |

PRPN (FEBRUARY 2024) CONSIDERATIONS

It is considered that Proposed Regional Plan for Northland (PRPN) February 2024)) C.4.1.9 Land drainage and flood control general conditions are applicable. Table 2 below outlines the relevant conditions with a brief ecological comment provided, where applicable. In respect to stormwater discharges, the discharge is in or from the same catchment in which the water would naturally flow, and the discharge is not within the catchment of an Outstanding Lake or a dune lake with outstanding or high ecological value.

| Condition | Comment |
|--|--|
| 1) There is no adverse flooding, erosion or over-drainage effects on other property | None anticipated – this has been addressed under LDE Stormwater Neutrality Report. |
| 2) The activity does not alter the course of a lake or continually or intermittently flowing river. | None anticipated – no works within a river or wetland proposed. |
| 3) New land drainage does not occur within 50 metres of any natural wetland. | No new land drainage proposed – upgrade to an existing stormwater system. |
| 4) Drainage does not cause any change to the seasonal or annual range in water level of a natural wetland to an extent that may adversely affect the wetland's natural ecosystem | No drainage has been proposed – stormwater discharges from the upgraded car park are unlikely to change the water level range or hydrological function of the wetland area, not adversely affect the wetland's natural ecosystem. |
| 5) No vegetation, soil or other debris generated from the activity is placed in a position where it may be carried into a river or natural wetland, lake or the coastal marine area. | To be managed through standard erosion and sediment best practice during construction. All earthworks are to take place a minimum 30m from any identified waterbody (including rivers and wetlands). |
| 6) There is no damage to a flood defence or any other authorised structure. | Not an ecological matter -to be managed through standard erosion and sediment best practice during construction. |
| 7) Fish passage is maintained, unless an existing authorisation provides otherwise, or temporary works to enable repair and replacement works are being carried out. | Fish passage shall not be altered from the baseline (current) setting. |

Table 2: Assessment of C.4.1.9 Land drainage and flood control general conditions

| 8) Eels, fish (other than pest fish), kōura (freshwater crayfish) and kākahi (freshwater mussels) unintentionally removed during mechanical clearing of drainage channels are returned to the drainage channel as soon as practicable, but no later than one hour after their removal. | No mechanical clearing of any waterbodies is required. |
|--|---|
| 9) Refuelling of machinery does not take place in the bed of a river or lake. | None anticipated. All earthworks are to take place a minimum 30m from any identified waterbody (including rivers and wetlands). |
| 10) Any discharge of drainage water does not contain concentrations of contaminants which have or are likely to have significant adverse effects on aquatic life in any river, wetland, or the coastal marine area. | LDE Stormwater Neutrality Report states that a Stormwater 360 Stormfilter is a cartridge filtration system that will be installed upstream of the connection point to the existing outlet structure. This passive filtration system consists of a precast concrete manhole with cartridges installed to provide treatment of the 'first flush' through filtration media to effectively remove contaminants generated by the movement of vehicles through the carpark. |
| 11) The discharge to the water body or coastal marine area does not, beyond the zone of reasonable mixing: a) result in any conspicuous oil or grease films, scums or foams, or floatable or suspended material except where caused by natural events in the receiving water, and | Not anticipated. |
| b) cause the pH of the receiving water to fall outside the range of 6.5 to 9.0 (except where caused by natural events, or when natural background levels fall outside that range), and | Not anticipated. |
| c) cause any emission of objectionable odour in the receiving water, and | Not anticipated. |
| d) cause any conspicuous change in colour or visual clarity of the receiving water, and | Not anticipated. |
| e) cause the natural temperature of the receiving water body to be changed by more than three degrees Celsius, except in an Outstanding Freshwater Body where it must not be changed by more than one degree Celsius, and | Not anticipated. |
| f) cause contamination which may render freshwater taken from a mapped priority drinking water abstraction point (refer I Maps | Not anticipated. |

| Ngā mahere matawhenua) unsuitable for human consumption after existing treatment. | |
|---|--|
| 12) Any discharge of sediment associated with repair and maintenance activities does not occur for more than five consecutive days and must not occur for more than 12 hours on any one day. | Not anticipated. |
| 13) Where in-river works involve bed disturbance from mechanical vegetation clearance or sediment removal: | No in-river works proposed. |
| a) if undertaken between 1 August and 31 December, a visual inspection of the works area must be undertaken, immediately prior to in-river work starting. If a shoal of whitebait is present, no inriver works shall be undertaken until the shoal passes; and | No in-river works proposed. |
| b) the works shall not occur more than once in any area between 1 August and 31 December of any year | Noted. |
| 14) River bank disturbance is limited to one side of the waterway, at any one time. | No river bank disturbance proposed. |
| 15) When mechanically clearing aquatic vegetation, a weed bucket shall be used with a curved flat base and a slatted back. | No mechanical clearance of aquatic vegetation proposed. |
| 16) The activity does not take place in an īnanga spawning site between 1 March and 30 September. | Activity will not take place within an īnanga spawning site. |

CONCLUSION

Based on desktop analysis of relevant ecological data and information provided by LDE, it is considered that proposed car park redevelopment works at Oruaiti School would result in negligible potential adverse ecological effect on the identified aquatic habitats. No adverse effect on the identified natural inland wetland area or Oruaiti Stream is anticipated – the nearest natural inland wetland area is located approximately 30m south of the works footprint. Any potential adverse ecological effects on natural inland wetland features associated with the proposal can be avoided, minimised or mitigated through applying appropriate construction and sediment/erosion controls. Provided that they are implemented successfully, adverse effects on the environment would be negligible.

It is considered that the construction of the car park associated with the proposal is not likely to change the water level range or hydrological function of the identified natural inland wetland area and will not result, or is not likely to result, in the complete or partial drainage of all or part of a natural inland wetland. Therefore, it is considered that the proposal does not require a consent under NES-FW (2020). Having reviewed the reporting prepared by LDE for the proposal, it is deemed that it complies with C.4.1.9 Land drainage and flood control general conditions under PRPN (February 2024).



Madara Vilde

Principal Ecologist

Wild Ecology

C 027 579 0624

maddy@wildecology.co.nz

wildecology.co.nz

Milde

05.06.2024



24 June 2024

Barker & Associates PO Box 37 Whangarei, 0112

Outline Plan and Resource Consent Application – Oruaiti School

Tēnā koe,

The Kahukuraariki Trust is the post settlement governance entity for the lwi of Ngatikāhu ki Whangaroa. Our rohe encompasses the Oruaiti River running south to Otangaroa and Waihapa, over to Waitaruke, inclusive of the western arm of the Whangaroa Harbour and the coastline back around to Taipa – including Taupo Bay.in Te Taitokerau.

Please accept this letter in support of the school's endeavours to upgrade, enhance and/or expand Oruaiti School.

We wish the project well and if there is anything further, we can do to assist, please let us know.

Noho ora mai,

Geraldine Baker General Manager



NZ Transport Agency Waka Kotahi Reference: Application-2024-0690

4 March 2025

Oruaiti School Board of Trustees c/o Avail Pacific / B&A Urban & Environmental 2554 State Highway 10 Mangonui Northland 0494 New Zealand

Sent via: MelissaM@barker.co.nz & r.deswardt@lde.co.nz

Dear Riaan and Melissa,

Proposed upgrade to Oruaiti School entrance/car park – 2554 State Highway 10, Mangonui – Board of Trustees c/o Avail Pacific

Thank you for your request dated 30 May 2024 for the written consent of the NZ Transport Agency Waka Kotahi (NZTA) pursuant to s176(1)(b) of the Resource Management Act 1991 (RMA) to undertake work within the following NZTA designation.

| District plan: | Operative Far North District Plan | Proposed Far North District Plan |
|-------------------------|---|---|
| Designation reference: | 5262201 | NZTA-2 |
| Designation purpose: | To give public notice of the existence of the work and of the nature and extent of activities permitted pursuant to the designation | To construct, operate, maintain, and improve a state highway, cycleway and/or shared path and associated infrastructure |

Your proposal has been considered as follows:

Proposal

The following activity is proposed:

- To upgrade the existing vehicle crossing/entrance, creating a one-way entry/exit system for all vehicles and establish formalised sealed parking and one-way manoeuvring areas at Oruaiti School in Mangonui.
- The proposal includes internal pedestrian pathways, stormwater infrastructure to serve the proposed impervious area and associated works.
- Stormwater runoff will be directed to a culvert within the state highway corridor that discharge south of the state highway near a stream (on the opposite side of the road from the school). The proposal includes stormwater treatment.

These activities are described in detail in:

• The plans titled "Oruaiti School Board of Trustees c/o Avail Pacific Proposed Carpark Upgrades 2554 State Highway 10, Mangonui" dated 1 May 2024 and referenced as' project number 21102', Revision A, prepared by Land Development & Engineering.



- The "STORMWATER NEUTRALITY REPORT" dated 2 May 2024, and referenced as 'project reference 21102', Version A, prepared by Land Development & Engineering.
- The memorandum titled 'Flooding Volume Assessment in Relation to the Oruaiti School Carpark Upgrades' dated 5 November 2024, and referenced as project ref. 21102, Revision 01, prepared by Land Development & Engineering.

Assessment

In assessing the proposed activity, NZTA notes the following:

- The proposal will improve safety outcomes for the site by:
 - formalising use of the metalled area adjacent to the formed state highway as parking, separated from the carriageway by a traffic island
 - o providing a left-in "slip bay" where vehicles may slow as they approach the entrance to the site
 - increasing sight distances to and from right-turning traffic exiting the site by establishing a one-way system with an exit located ~70m further west than the existing crossing, away from a bend on the east side, where a raised and vegetated berm currently reduces sight distances.
 - o providing a separated pick up / drop off area for buses separated from light vehicle movements
 - o enabling forward entry and exit to and from the site
- Northland Regional Council maps indicate this section of state highway is subject to flooding, however specialist
 assessment notes this is the result of downstream flooding and future sea level rise scenarios. A variety of
 options have been considered for the potential mitigation of stormwater discharge; however they would
 ultimately have little effect on the predicted flood scenario and associated effects. The increase in flood levels is
 considered no different from the effects of modelled predevelopment flood scenarios. The relevant interested
 parties within NZTA concur that the additional discharge to the state highway can be accepted in this particular
 case.

Conditions

NZTA considers that Oruaiti School Board of Trustees c/o Avail Pacific can undertake the proposed access and parking upgrades and associated stormwater discharge within 5262201 without preventing or hindering the work to which the designation relates subject to these conditions: The legal name of NZTA is the **New Zealand Transport Agency**; therefore our full legal name is referred to in the conditions and approval.

Conditions:

Pre-construction Agreements

- 1. Prior to the commencement of any works on the vehicle crossing or within the state highway corridor, the consent holder must provide evidence of the following as approved by the New Zealand Transport Agency, or written confirmation from New Zealand Transport Agency that the approval is not required:
 - a. A 'Corridor Access Request' (CAR)
 - b. An 'Agreement as to Work on State Highway'
 - c. A 'Work access permit' (WAP)

The application must include detailed construction drawings showing full details of the works, methodology and a Traffic Management Plan.



2. A formal licence to occupy / network licence (as advised by NZ Transport Agency) to occupy NZTA managed Crown land for the purposes of completing the proposed works shall be agreed to between the applicant and the NZ Transport Agency prior to commencement of any construction associated with this approval.

General Conditions

- 3. That the vehicle crossing, parking, manoeuvring and associated works be undertaken in general accordance with the plans referenced below and to the satisfaction of the New Zealand Transport Agency Network Manager:
 - a. 'Parking Area & Accessway Earthworks Layout' dated 1 May 2024 and referenced as 'C200', revision A, prepared by Land Development & Engineering.
 - b. 'Parking Area & Accessway Overall Site Plan' dated 1 May 2024 and referenced as 'C300', revision A, prepared by Land Development & Engineering.
 - c. 'Parking Area & Accessway Design Plan Sheet 1 of 2' dated 1 May 2024 and referenced as 'C301', revision A, prepared by Land Development & Engineering.
 - d. 'Parking Area & Accessway Design Plan Sheet 2 of 2' dated 1 May 2024 and referenced as 'C302', revision A, prepared by Land Development & Engineering.
 - e. 'Parking Area & Accessway Longitudinal Sections' dated 1 May 2024 and referenced as 'C310', revision A, prepared by Land Development & Engineering.
 - f. 'Parking Area & Accessway Stormwater Management Plan' dated 1 May 2024 and referenced as 'C400', revision A, prepared by Land Development & Engineering.
 - g. 'Parking Area & Accessway Stormwater Longitudinal Sections Sheet 1 of 2' dated 1 May 2024 and referenced as 'C410', revision A, prepared by Land Development & Engineering.
 - h. 'Parking Area & Accessway Stormwater Longitudinal Sections Sheet 1 of 2' dated 1 May 2024 and referenced as 'C411', revision A, prepared by Land Development & Engineering.
- 2. Consultation with the New Zealand Transport Agency shall be undertaken in the event changes are made from the plans initially submitted to the New Zealand Transport Agency (and referenced above). This consultation will confirm if the changes are in general accordance with the approved plans or confirm if additional approvals are required.

After construction

3. Prior to the use of the upgraded crossing and vehicle manoeuvring areas, the consent holder shall provide to Council, correspondence from New Zealand Transport Agency confirming that works to the state highway, including vehicle crossings, have been constructed to New Zealand Transport Agency standards or in general accordance with this approval.

Advice Notes:

- The maintenance of all structures and infrastructure associated with the proposal is the responsibility of the applicant. These must be maintained in good order and condition, and otherwise to the satisfaction of NZTA, and subject to any directions and conditions stipulated by NZTA. This advice note may be superseded by a condition on a licence to occupy / network licence granted by the NZ Transport Agency.
- The applicant should provide a copy of any resource consent application associated with the proposal to NZTA once lodged for s95E approval.

Determination

On the basis of the above assessment of the proposed activity, and the conditions volunteered by the applicant, the New Zealand Transport Agency grants Oruaiti School Board of Trustees c/o Avail Pacific written approval under section 176(1)(b) of the RMA for the proposed access and parking upgrades and associated stormwater discharge to occur.

In addition, before you undertake any physical work on the state highway, including the formation of any vehicle crossing, you are required to apply to NZTA for a Corridor Access Request (CAR) and for that request to be approved.



Before you undertake any physical work on the state highway, including the formation or change to any vehicle crossing, you are legally required to apply to the New Zealand Transport Agency for a Corridor Access Request (CAR) and for that request to be approved. A CAR is submitted online via <u>www.submitica.com</u> a minimum of fourteen working days prior to the commencement of any works on the state highway; longer is advised for complex works.

Please notify NZTA if changes to the application or volunteered conditions occur.

Expiry of this approval

Unless resource consent has been obtained this approval will expire two years from the date of this approval letter. This approval will lapse at that date unless prior agreement has been obtained from The New Zealand Transport Agency.

If you have any queries regarding the above or wish to discuss matters further, please feel free to contact the Environmental Planning team at <u>environmentalplanning@nzta.govt.nz</u>.

Yours sincerely,

Jan.

Jordan Pauw Planner Poutiaki Taiao / Environmental Planning, System Design, on behalf of NZ Transport Agency Waka Kotahi. cc: <u>aran.arietta@nzta.govt.nz</u>

Enclosed:

Attachment 1: Proposed Site Plan



Project Number:

Project Office: Project Manager: Issue Date:

Drawing Status: For Consent

Oruaiti School Board of Trustees c/o Avail Pacific 2554 State Highway 10, Mangonui Proposed Carpark Upgrades

| Drawing Index | | | | |
|---------------|--|-------------|---------------------|--|
| Sheet | Description | Status | Revision / Issue | |
| C001 | Existing Site Plan | For Consent | А | |
| C200 | Earthworks Layout | For Consent | А | |
| C300 | Overall Site Layout | For Consent | А | |
| C301 | Parking Area & Accessway Design Plan Sheet 1 of 2 | For Consent | А | |
| C302 | Parking Area & Accessway Design Plan Sheet 2 of 2 | For Consent | А | |
| C310 | Parking Area & Accessway Longitudinal Sections | For Consent | А | |
| C311 | Parking Area & Accessway Typical Cross Sections | For Consent | А | |
| C312 | Parking Area & Accessway 5m Interval Cross Sections Sheet 1 of 3 | For Consent | А | |
| C313 | Parking Area & Accessway 5m Interval Cross Sections Sheet 2 of 3 | For Consent | А | |
| C314 | Parking Area & Accessway 5m Interval Cross Sections Sheet 3 of 3 | For Consent | А | |
| C320 | Parking Area & Accessway Typical Details | For Consent | А | |
| C400 | Parking Area & Accessway Stormwater Management Plan | For Consent | А | |
| C410 | Parking Area & Accessway Stormwater Longitudinal Sections Sheet 1 of 2 | For Consent | А | |
| C411 | Parking Area & Accessway Stormwater Longitudinal Sections Sheet 2 of 2 | For Consent | А | |
| C420 | Parking Area & Accessway Typical Stormwater Device Details | For Consent | А | |
| C421 | Parking Area & Accessway Typical Stormwater Details | For Consent | A | |



21102

Warkworth Riaan de Swardt 1 May 2024



General Notes:

4

Library

Pool

- All dimensions in metres unless noted otherwise.
- Coordinates are in terms of NZGD Mt Eden 2000. 2. All levels are in metres to NZVD2016. Origin of 3
 - levels RCM XIII SO 50961 RL 11.81m.
- Survey undertaken by Donaldsons; 25 July 2022. Existing titles Pt Allot 18A PSH OF Mangonui East
- 5. Boundaries and are approximate only and are subject to change. Final boundaries will be in 6. accordance with city plan and LINZ requirements.
- Boundaries not for building design purposes. 7 Existing building, roading, and services information 8. may have been obtained from third party records and should be verified on site prior to the commencement of any works. LDE does not in any way guarantee the accuracy of any underground service shown on this plan.
- 9. Contractor to locate / mark / confirm and protect if necessary all existing services prior to commencement of work on site. Any discrepancies to be reported to the Engineer.
- 10. Flood extent extracted from NRC Natural Hazards and shown indicatively for information only.

General Legend: ____

 Boundary (Surveyed) - 1m Major Contour 5m Minor Contour

NRC River Flood Hazard Zone - Regionwide Models

| 10 Year Exent |
|------------------------|
| 50 Year Extent |
| 100 Year CC Extent |

Existing Services Legend:

| | | | | _ | — |
|---|---------------|---------------|---------------|---------------|--------------|
| 8 | **** | *** | **** | *** | \bigotimes |
| | SW | | | sw — | _ |
| | SW | \langle | \$ | | |
| | | | | | |
| ⇒ | \rightarrow | \rightarrow | \rightarrow | \rightarrow | _ |
| | -ww | | | ww— | |
| | — он | (| | он — | |
| | | | Γ | | |
| _ | | | | | _ |
| | | - | | - | |

Edge of Road Asphalt Surface Stormwater Pipe Stormwater Manhole, Wingwall, Scruffy Dome, Catchpit Open Drain Wastewater Pipe Power Pole & Over Head Lines Transformer Fence **Retaining Wall**

| 2/ | | | | ° | 5 | 10 | 15 | 5 2 | 20 | 25 |
|--------|-------|------------|-----------|-------|----------|------|-------------------|----------------------------|------------------------|-------------------|
| \geq | | | | Scale | 1:500 | | (m) | (| Original siz | e = A3 |
| | | | Design: | R. d | e Swardt | Proj | ect status: Fo | r Con | sent | |
| | | | Drawn: | R. d | e Swardt | Proj | ect | | 00111 | |
| | | | Approved: | J. \$ | Simson | | | 2110 | 2 | |
| | J. S | 1 May 2024 | Scale A3: | 1 | 1:500 | Dra | | 2001 | | A |
| | Aprvd | Date | | | | | Drive | Location C., Saved by Rise | nDeSwardt, Document ID | 362062, Vension 6 |



General Notes:

- . All dimensions in metres unless noted otherwise.
- Coordinates are in terms of NZGD Mt Eden 2000.
 All levels are in metres to NZVD2016. Origin of
- levels RCM XIII SO 50961 RL 11.81m. Survey undertaken by Donaldsons: 25
- Survey undertaken by Donaldsons; 25 July 2022.
 Existing titles Pt Allot 18A PSH OF Mangonui East
- Existing trues Pt Allot 18A PSH OF Mangonul Ear
 Boundaries and are approximate only and are subject to change. Final boundaries will be in coordenee with city plan and UNZ
- accordance with city plan and LINZ requirements.7. Boundaries not for building design purposes.9. Eviding building and bui
- Existing building, roading, and services information may have been obtained from third party records and should be verified on site prior to the commencement of any works. LDE does not in any way guarantee the accuracy of any underground service shown on this plan.
- Contractor to locate / mark / confirm and protect if necessary all existing services prior to commencement of work on site. Any discrepancies to be reported to the Engineer.
- Flood extent extracted from NRC Natural Hazards and shown indicatively for information only.

Earthworks Notes:

- All works to be undertaken in accordance with FNDC Engineering Standards, GD05 Guidelines and the Health and Safety at Work Act 2015 (HSWA).
- All disturbed public areas to be made good and restored to previous condition.
- All earthworks to be undertaken during periods of fine weather.
- Earthworks are to be undertaken and completed in within the minimum possible time frame.
- The area is to be progressively finished as areas are brought to grade.
- The earthworks volumes shown is a solid measure.
- The earthworks volumes has been calculated in relation to the existing and final ground levels.
- Topsoil removal volumes is excluded in the total earthworks volumes.
- Additional box cut volumes has not been included in the earthworks volumes.

Erosion and Sediment Control Notes:

- The contractor is solely responsible for ensuring the the site has effective erosion control and sediment retention measures operating at all times.
- All erosion and sediment control measures must be operational prior to any other works commencing on site.
- All sediment control measure are to comply with GD05 and FNDC requirements.

| Earthworks Cut / Fill Volumes | | | | | | | | |
|-------------------------------|----------------------|--|--|--|--|--|--|--|
| Site Area | 41,210m ² | | | | | | | |
| Earthworks Area | 2,525m² | | | | | | | |
| Cut Volume | 18m³ | | | | | | | |
| Fill Volume | 230m³ | | | | | | | |
| Net Fill Volume | 212m³ | | | | | | | |

| | / | | | 0 5 | 10 | 15 | 20 | 25 |
|------|---------------|--------------------|-----------|--------------|-------------|-------|-------------|------------|
| / | | | | Scale 1:500 | (m | ı) | Original si | ze = A3 |
| | | | Design: | R. de Swardt | Project sta | For C | Consent | |
| | | | Drawn: | R. de Swardt | Project: | 1010 | | |
| | | | Approved: | H. Dubinko | Drawing N | 21 | 102 | Icoue/Rour |
| sion | H. D Aprvd | 1 May 2024 Date | Scale A3: | 1:500 | Drawing iv | C20 | 00 | A |



Proposed Services Notes:

- All works to be undertaken in accordance with FNDC Engineering Standards and the Health and Safety at Work Act 2015 (HSWA).
- All dimensions in metres unless noted otherwise. 2.
- Contractor to locate / ml dimensionark / confirm and 3. protect if necessary all existing services prior to commencement of work on site.
- 4 All disturbed public areas to be made good and restored to existing condition.
- Manholes to be reinforced concrete chambers with a 5. precast base. Min Ø1050mm manhole unless otherwise specified.
- 6. Catchpits to be 675 x 450mm reinforced concrete chambers, with trafficable frame & grid inlet lid.
- All PVC stormwater pipes to be uPVC SN16.
- All concrete stormwater pipes to be RCRRJ Class 4. 8 Pipe bedding and installation to comply with relevant 9 council details.
- 10. All lane and parking markings as per MOTSAM.
- 11. Parking bays as per Appendix 3D of the Far North District Plan (FNDP).

Reference Notes:

- Refer to C301 & C302 for Parking Area Layouts.
- Refer to C310 for Parking Area and Accessway Longitudinal Sections Layout.
- Refer to C311 to C314 for Cross Section Layouts.
- Refer to C320 for Roading details.
- Refer to C400 for Stormwater Layout.
- Refer to C410 to C411 Stormwater Longitudinal Section Layouts.
- Refer to C420 & C421 for Stormwater details.

Existing Services Legend:

| | Boundary (Surveyed) |
|---|-------------------------------|
| | Edge of Road |
| sw sw | Stormwater Pipe |
| 👓 🤇 🏶 🛛 | Stormwater Manhole, Wingwall, |
| | Scruffy Dome, Catchpit |
| $\rightarrow \rightarrow \rightarrow \rightarrow \rightarrow -$ | Open Drain |
| — он — ф он — | Power Pole & Over Head Lines |
| T | Transformer |
| | Fence |
| -0-0-0-0-0-00 | Retaining Wall |
| | |

Proposed (Prop.) Services Legend:





& ENGINEERING

- 0.5m Major Contour 0.1m Major Contour Kerb Block - Kerb & Channel Flush Edge Beam Asphalt Surface Proposed Concrete Surface Proposed Gravel Surface Rock Filled Traffic Island Stormwater Pipe Stormwater Perforated Pipe Stormwater Manhole SW Manhole with Scruffy Dome Stormwater Catchpit Stormwater Rodding Point Stormwater Swale Fence

| / | | | 0 5 | 10 | 15 | 20 | 25 |
|----|--------------|-----------|--------------|----------------|------------------------|-----------------------------------|-----------------|
| / | | | Scale 1:500 | (m) | | Original siz | e = A3 |
| | _ | Design: | R. de Swardt | Project status | Eor Co | onsent | |
| | | Drawn: | R. de Swardt | Project: | | | |
| | | Approved: | H. Dubinko | Drawing No: | 21' | 102 | losuo/Pour |
| H. | D 1 May 2024 | Scale A3: | 1:500 | Drawing No. | C300 | 0 | A |
| П | Vu Date | | | | Drive Location C: Save | of her Disso CarSecord, Company D | accord Gambra G |





| | | Pro | posed S | ervic | es Leger | nd | : | | | |
|----------------|-----|---------------------|-----------------------------|----------------|--------------|-----|-----------------|---------|----------|-----------|
| ed) | | | | | Asphalt S | Sur | face | | | |
| | | | | | Proposed | d C | Concrete | Surfa | се | |
| | | | | | Proposed | 9 6 | Gravel S | urface | | |
| ole, Wingwall, | | 62 | 9493 | <u> 9</u> 2 | Rock Fille | ed | Traffic I | sland | | |
| chpit | | | | | Stormwat | ter | Perfora | ted Pi | pe | |
| rlloodlingo | | | (ev) (A) | | Stormwai | ter | Manho | e | Domo | |
| r Head Lines | | | ₩ ₩ | | Stormwat | tor | Catchn | it | Dome | |
| | | | R | | Stormwat | ter | Roddin | a Poin | t | |
| | | → - | $\rightarrow \rightarrow -$ | → | Stormwat | ter | Swale | 5 | | |
| ır | | | | | Fence | | | | | |
| ur | | | | | | | | | | |
| | | | | | | | | | | |
| nnel | | | | | | | | | | |
| Beam | | | | | | | | | | |
| | | Roa | d Marki | nala | aand. | | | | | |
| | | Nua | | | egenu. | | | | | |
| sing | | | • •••• • | | 1650mm I | Ru | bber Wi | ieel St | top | |
| Wide, 0.6m G | iap | | Ö . | | Yellow Di | sa | bled Pa | rking S | Sign | |
| | | $\overline{\nabla}$ | \overline{III} | \overline{Z} | Yellow No | o-F | Parking | sland | | |
| | | | | | | | | | | |
| | | | | | | | | | | |
| | | | | | | | | | | |
| | | | | | | | | | | |
| W | | | | | | | | | | |
| | | | | | | | | | | |
| row | | | | | | | | | | |
| ineation Line | | | | | | | | | | |
| | | | | | | | | | | |
| nes Gap | | | | | | | | | | |
| | | | | | | | | | | |
| | | | | | | | | | | |
| | | | | | | | | | | |
| | | | | | | | | | | |
| | | | | | | | | | | |
| | | | | | | | | | | |
| | | | | | | | | | | |
| | | | | | | | | | | |
| | | | | | | | | | | |
| | | | | | | | | | | |
| | | | | | | | | | | |
| | | | | | | | | | | |
| | | | | | | | | | | |
| | | | | | | | | | | |
| | | | | | | | | | | |
| | | | | | | | | | | |
| | | | | | | | | | | |
| | | | | | | | | | | |
| | | | | | | | | | | |
| | | | | | | | | | | |
| | | | | | | | | | | |
| | | | | | | | | | | |
| | | | | | | | | | | |
| | | | | | | | | | | |
| | | | | | | | | | | |
| | | | | | | | | | | |
| | | | | | | | | | | |
| | | | | 0 | 2.5 | ļ | 5 7 | .5 | 10 | 12.5 |
| | | | | Scal | e 1:250 | | (m) | | 0 | size - ** |
| | | | Design: | | t de Swardt | | Project status: | | Uriginal | size = A3 |
| | | | D D D | | | - | F | or Co | nsen | t |
| | | | Drawn: | F | k. de Swardt | _ | Project: | 211 | 02 | |
| | | | | | LL Durkislas | - 1 | | | | |

H. Dubinko

1:250

Scale A3

Α

C302

| Vortical Exagonation: 2.5 | | | | | | | | |
|----------------------------|----|----------|--------|--------|-----|-----|--|--|
| Vertical Exaggeration. 2.5 | | | | | | | | |
| Datum: 0.0 | | | | | | | | |
| | | | | | | | | |
| | | | | | | | | |
| | | - | | | - | - | | |
| Design Levels | | 00 | 34 | 04 | 36 | 20 | 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0 | |
| Design Levels | | 5. | 5 | D | 5. | 5 | 2 | |
| | | | | | | | | |
| | | | | | | | | |
| | 9 | 2 | ດ | - - | 0 | 4 | <u>o</u> | |
| Existing Levels | 6. | <u>.</u> | N N | 4 | 2 | N N | 0 | |
| - | 4 | Ω. | ى ى | L) L) | 2 | Ω. | വ | |
| | | | | | | | | |
| | | - | | | | | | |
| Cut/Fill Depth | | 6 | 05 | 80 | 10 | 05 | 6 | |
| | | ö | ö | i õ | | ö | o. | |
| | | - | - | - | - | | | |
| | | | | | | | | |
| | | õ | | | 2 | Q | | |
| Chainage | | 0.0 | | 0.0 | 0.0 | 0.0 | | |
| - | | 5 | - ÷ | ы М | 5 | ж |) ř | |
| | l | | | | | | | |

Long Section (ACCESSWAY CL)

| | _^ | | + | | | | | | | | | | | | | | | |
|----------------------------|-----|-----|-----|-----|------|-----|-----|-----|----------|----------|------|-----|-------|-----|-----|-----|------|-----|
| | | | | | | | | | + | | | | | | | | | |
| | | | | | | | | | | | | | | | | | | |
| | | | | | | | | | | | | | | | | | | |
| | | | | | | | | | | | | | | | | | | |
| | | | | | | | | | | | | | | | | | | |
| | | | | | | | | | | | | | | | | | | |
| | | | | | | | | | | | | | | | | | | |
| | | | | | | | | | | | | | | | | | | |
| | | | | | | | | | | | | | | | | | | |
| | | | | | | | | | | | | | | | | | | |
| | | | | | | | | | | | | | | | | | | |
| Vertical Exaggeration: 2.5 | | | | | | | | | | | | | | | | | | |
| Datum: 0.0 | | | | | | | | | | | | | | | | | | |
| | | | | | | | | | | | | | | | | | | |
| | • | | | 10 | - | • | | 10 | + | | 10 | ~ | 0 | | | 10 | ~ | ~ |
| Design Levels | 5 | 30 | 5 | 5 | Ņ | ÷. | | 4 | ő | .97 | 0 | ö | Ő | | 5 | .36 | .56 | 12 |
| _ | 4 | 4 | 4 | 4 | 4 | 4 | 4 | 4 | 4 | с С | 4 | 4 | 4 | 4 | 4 | 4 | 4 | 4 |
| | | | | | | | | | | | | | | | | | | |
| Existing Levels | 29 | 30 | 27 | 52 | 5 | 19 | 17 | 15 | 2 | 61 | 02 | 08 | 60 | 12 | 17 | 35 | 58 | 77 |
| | 4 | 4 | 4 | 4 | 4 | 4 | 4 | 4 | 4 | τi C | 4 | 4 | 4 | 4 | 4 | 4 | 4 | 4 |
| | | | | | | | | | | | | | | | | | | |
| Cut/Fill Depth | 8 | 0 | | 8 | 8 | 8 | 0 | 8 | 8 | 8 | 8 | 8 | 0 | | 8 | 00 | 00 | 8 |
| | o.o | 0.0 | i o | Ö.Ö | 0.0 | 0.0 | 0.0 | 0.0 | Ö.Ö | 0.0 | 0.0 | 0.0 | 0.0 | ö | 0.0 | 0.0 | 0.0 | Ö.Ö |
| | | | | | | | | | | | | | | | | | | |
| | 0 | E E | | 0 | 8 | 0 | g | 8 | 8 | 8 | 8 | g | 0 | | 0 | 0 | 0 | g |
| Chainage | 2.0 | | 2.0 | 0.0 | 25.0 | 0.0 | 5.0 | 0.0 | 5.0 | 0.0 | 55.0 | 0.0 | 52 (C | 0.0 | 5.0 | 0.0 | 35.0 | 0.0 |
| | | | | N | N | ო | ო | 4 | 4 | LO LO | cu | Ű | Ű | | 2 | ω | ω | 0 |

Long Section (PARKING AREA CL)

Copyright: LDE All rights reserved / Do not scale off drawings / Confirm all dimensions on site prior to work Client

Oruaiti School Board of Trustees c/o Avail Pacific

2554 State Highway 10, Mangonui Proposed Carpark Upgrades

Project

Drawing Title Parking Area & Accessway Longitudinal Sections



| | | | | Oliginal Size - Ac |
|------|------------|------------|----------------|----------------------|
| | | Design: | R. de Swardt | For Consent |
| | | Drawni | D de Swardt | |
| | | Diawii. | R. de Swardt | Project: |
| | | Approved: | H. Dubinko | 21102 |
| | | rippiorod. | The Bradelinko | Drawing No: Issue/Re |
| H. D | 1 May 2024 | Scale A3: | 1:250 | C310 A |



| | | | | | ě |
|--------|-------|------------|-----------|--------------|--------------------------------|
| | | | Design: | R. de Swardt | Project status: For Consent |
| | | | _ | | |
| | | | Drawn: | R. de Swardt | Project: |
| | | | | | 01100 |
| | | | Approved: | H Dubinko | 21102 |
| | | | Appioreu. | TI: Dubinito | Drawing No: Issue/Rev |
| | | | | | |
| | H. D | 1 May 2024 | Scale A3: | 1:75 | I C311 IA |
| vision | Aprvd | Date | | | |











Copyright: LDE All rights reserved / Do not scale off drawings / Confirm all dimensions on site prior to work Client

Oruaiti School Board of Trustees c/o Avail Pacific

Project 2554 State Highway 10, Mangonui Proposed Carpark Upgrades

Drawing Title Parking Area & Accessway 5m Inteval Cross Sections Sheet 1 of 3



A

C312

H. D 1 Scale A3: May 2024

Approved:

1:250



Oruaiti School Board of Trustees c/o Avail Pacific

2554 State Highway 10, Mangonui Proposed Carpark Upgrades

Drawing Title Parking Area & Accessway 5m Inteval Cross Sections Sheet 2 of 3





с |RL:-4.52 0-







| | | | Design: | R. de Swardt | Project status: For Consent |
|--------|-------|------------|-----------------------|--------------|--|
| | | | | | |
| | | | Drawn: | R. de Swardt | Project: |
| | | | | | 21102 |
| | | | Approved [.] | H Dubinko | 21102 |
| | | | rippiorou. | | Drawing No: Issue/Rev: |
| | | | | | |
| | H. D | 1 May 2024 | Scale A3: | 1:250 | I 6313 IA |
| rision | Aprvd | Date | | | |
| | | | | | Drive Location C., Saved by RisanDeSvaardt, Document ID 455509, Version 14 |











Copyright: LDE All rights reserved / Do not scale off drawings / Confirm all dimensions on site prior to work Client

Oruaiti School Board of Trustees c/o Avail Pacific

2554 State Highway 10, Mangonui Proposed Carpark Upgrades

Project

Drawing Title Parking Area & Accessway 5m Inteval Cross Sections Sheet 3 of 3







| | | | | | Original size = A3 |
|--------|---------------|--------------------|-----------|--------------|--------------------------------|
| | | | Design: | R. de Swardt | Project status: For Consent |
| | | | Drawn: | R. de Swardt | Project |
| | | | Approved: | H. Dubinko | 21102 |
| rision | H. D Aprvd | 1 May 2024 Date | Scale A3: | As Shown | C320 A |

- Safety at Work Act 2015 (HSWA).
- the FNDC Engineering Standards (ES).
- Standards as stated in the FNDC ES.
- Contractor to locate / ml dimensionark / confirm and protect if necessary all existing services prior to commencement of work on site.
- 6. All disturbed public areas to be made good and restored to existing condition.
- with a precast base. Min Ø1050mm manhole unless otherwise specified.
- chambers, with trafficable frame & grid inlet lid.

- Trap", installed as per supplier specification.
- is to be re-laid to SWMH6 and connected to

- Scruffy Dome, Catchpit
 - Transformer

- 0.5m Major Contour 0.1m Major Contour ---- Prop. Kerb Block







Long Section (SWCP1 TO SWMH3)





Long Section (SWCP7 TO SWMH3)

Copyright: LDE All rights reserved / Do not scale off drawings / Confirm all dimensions on site prior to work Client

Project 2554 State Highway 10, Mangonui Proposed Carpark Upgrades Drawing Title Parking Area & Accessway Stormwater Longitudinal Sections Sheet 1 of 2



Original size = A3

| | | | Design: | R. de Swardt | Project status: For Consent |
|--------|---------------|--------------------|-----------|--------------|--------------------------------|
| | | | Drawn: | R. de Swardt | Project: |
| | | | Approved: | H. Dubinko | 21102 |
| vision | H. D Aprvd | 1 May 2024 Date | Scale A3: | 1:250 | C410 A |



Long Section (SPORTS FIELD CATCHPIT TO TANK)



Long Section (DRAINAGE TRENCH)

Copyright: LDE All rights reserved / Do not scale off drawings / Confirm all dimensions on site prior to work Client

Oruaiti School Board of Trustees c/o Avail Pacific

Project 2554 State Highway 10, Mangonui Proposed Carpark Upgrades

Drawing Title Parking Area & Accessway Stormwater Longitudinal Sections Sheet 2 of 2



| | | | | | Oliginal size – Ab |
|--------|-------|------------|-----------|--------------|--------------------------------|
| | | | Design: | R. de Swardt | Project status: For Consent |
| | | | Drawn: | R. de Swardt | Project: |
| | | | Approved: | H. Dubinko | 21102 |
| | _ | | | | Drawing No: Issue/Rev |
| islan | H. D | 1 May 2024 | Scale A3: | 1:250 | C411 A |
| rision | Aprvd | Date | | | 1 1 |





| | B B B B B B B B B B B B B B B B B B B | 00 dia. |
|---|---|--|
| Opening to be over for sanitary sewer only. | 600 for pipes over 30 | 0 dia Conci 25mr |
| ME | B SECTION CC | uP.V.C. r Fran |
| <u>NOTES:</u> 1. This de | stail is applicable for pipe diameters | riser bedo |
| up to 6 2. 150mm iron fro & bern iron fro | 600mm & for manhole depths up to 5.0m. In thick reinforced concrete lids with heavy duty du ames & covers to be used in driveways, carriagew ms. 100mm thick concrete lids with light duty cas ames & covers may be used elsewhere. | Mort ctile arou rays t |
| Precast with mi No add | t manhole bases shall be used in all instances inimum sized holes cut for pipe entry. ditional thin plastering of benching or benching of | 20 |
| inverts 5. All con | is permitted. Increte to be 20MPa. | co |
| manhol bond b should 8. All mar rungs. The | les, then a gritted starter pipe shall be installed to between manhole and pipe material. Also, a 3flexibl be specified as part of gritted starter pipe (as si nholes >1.2m in depth shall be provided with man be abalt follow the requirements on about 40 | o allow le joint tandard). hole step |
| | ese shun funow the requirements on sheet 40 | |
| | | |
| | 200mm Tax London junction | |
| | 200mm London junction Epoxy mortar around pipe | |
| | 200mm London junction Epoxy mortar around pipe Short - no longer than 1000mm | andard step at 300m |
| | 200mm London junction Epoxy mortar around pipe Short – no longer than 1000mm Dropper pipe for sewer only (single piece) | andard step at 300m Bolt h mortar se Sheet 40 |
| | 200mm Trax Epoxy mortar around pipe Short – no longer than 1000mm Dropper pipe for sewer only (single piece) Where the height of drop exceeds 1.5m the dropper pipe shall be rigidly attached to the wall of the manhole with approved Grade 316 SS or plastic supports at 1.5m centres m | andard step at 300m Bolt h mortar see Sheet 40 rtar around nax. |
| | 200mm Trax London junction Epoxy mortar around pipe Short - no longer than 1000mm Dropper pipe for sewer only (single piece) Where the height of drop exceeds 1.5m the dropper pipe shall be rigidly attached to the wall of the manhole with approved Grade 316 SS or plastic supports at 1.5m centres m 88' bend set in benching -Holf pipe channel (ceramic or uPVC) TION A-A INTERNAL DROP | andard step at 300m Bolt h mortar sea Sheet 40 rtar around nax. nm layer o IPa concre |
| SECT Note: 1. This det manhole 2. External | 200mm Trax London junction Epoxy mortar around pipe Short - no longer than 1000mm Dropper pipe for sewer only (single piece) Where the height of drop exceeds 1.5m the dropper pipe shall be rigidly attached to the wall of the manhole with approved Grade 316 SS or plastic supports at 1.5m centres m 88' bend set in benching Half pipe channel (ceramic or uPVC) TION A-A INTERNAL DROP tail is applicable for pipe diameters up to 250mm e depth up to 5.0m & for manhole diameters > 1 drops shall not be used | andard step at 300m Bolt h mortar see Sheet 40 rtar around nax. nm layer o IPa concre & for 200mm. |

ENGINEERING STANDARDS

Issued for Consent Issue/Rev

Copyright: LDE All rights reserved / Do not scale off drawings / Confirm all dimensions on site prior to work

Oruaiti School Board of Trustees c/o Avail Pacific

2554 State Highway 10, Mangonui Proposed Carpark Upgrades

Project

Parking Area & Accessway Typical Stormwater Details



| | | | Design: | R. de Swardt | For Consent | |
|--------|-------|------------|-----------|--------------|---------------------|-----|
| | | | Drawn: | R. de Swardt | Project: | |
| | | | Approved: | H. Dubinko | 21102 | |
| | H. D | 1 May 2024 | Scale A3: | As Shown | Drawing No: C421 | A A |
| vision | Aprvd | Date | | | ••• | |